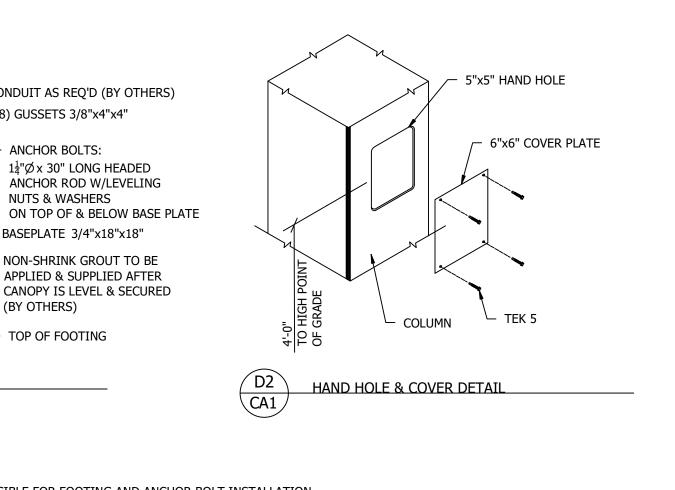


FOUNDATION PLAN SCALE 1/4"=1'-0"



FOOTING NOTES

COLUMN -

BASEPLATE DETAIL

1. OWNER / GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR FOOTING AND ANCHOR BOLT INSTALLATION. 2. ALL FOOTINGS SHALL BE CAST ON LEVEL UNDISTURBED SOIL, ROCK OR PROPERLY COMPACTED STRUCTURAL FILL PER GEOTECH REPORT. BOTTOM OF FOOTING TO BE ABOVE WATER TABLE. FOOTING SIZE BASED ON MINIMUM 1500 PSF SOIL BEARING CAPACITY AT BASE AND 150 PSF PER FOOT OF DEPTH LATERAL BEARING CAPACITY. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING ALL SOIL PARAMETERS.

CONDUIT AS REQ'D (BY OTHERS)

 $1\frac{1}{4}$ "Øx 30" LONG HEADED ANCHOR ROD W/LEVELING

- (8) GUSSETS 3/8"x4"x4"

ANCHOR BOLTS:

NUTS & WASHERS

(BY OTHERS)

- TOP OF FOOTING

- BASEPLATE 3/4"x18"x18"

NON-SHRINK GROUT TO BE APPLIED & SUPPLIED AFTER

CANOPY IS LEVEL & SECURED

3. ALLOWABLE SOIL BEARING INDICATED IN NOTE 2 ABOVE BASED ON FOUNDATIONS BEARING IN UNDISTURBED SOIL OR PROPERLY PREPARED / COMPACTED STRUCTURAL FILL.

4. FOOTING CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI. 5. TOPS OF ALL FOOTINGS ARE ASSUMED TO BE AT SAME ELEVATION. OWNER / GENERAL CONTRACTOR SHALL PROVIDE BURIAL DEPTH FROM HIGH GRADE UNDER CANOPY. WHERE TOPS OF FOOTINGS ARE AT DIFFERENT ELEVATIONS, THE OWNER / GENERAL CONTRACTOR SHALL PROVIDE THE CANOPY MANUFACTURER WITH ALL FOOTING AND GRADE ELEVATIONS PRIOR TO CANOPY FABRICATION. VARIATIONS FROM DESIGN ELEVATIONS MAY RESULT IN INADEQUATE CLEARANCE AND UNDER SIZED FOOTINGS.

6. OWNER / GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR PLACING NON-SHRINK GROUT UNDER ALL

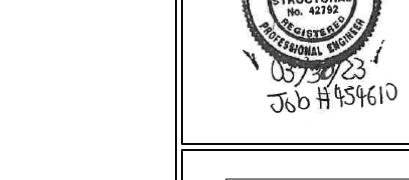
COLUMN BASES AFTER CANOPY IS LEVELED AND SECURED. 7. FOOTING REINFORCING STEEL SHALL BE ASTM A615 GRADE 60 DEFORMED BILLET STEEL BARS WITH SPACING AS SHOWN ON DRAWING.

8. FOOTINGS ARE ASSUMED TO BE CONSTRAINED BY DRIVE MAT CONCRETE. WHERE THIS CONDITION DOES NOT EXIST, THE OWNER SHALL NOTIFY CANOPY MANUFACTURER. 9. ANCHOR RODS SHALL BE PLACED IN ACCORDANCE WITH THIS DRAWING. TEMPLATES SHALL BE USED TO

ENSURE PROPER PLACEMENT OF ANCHOR RODS. ANCHOR RODS ARE TO BE INSTALLED SUCH THAT A MINIMUM OF 7" OF THREAD IS EXPOSED ABOVE TOP OF FOOTING. BOTTOM OF THREADS SHALL NOT END MORE THAN 3/4" ABOVE TOP OF FOOTER.

PLATE 3/4"x18"x18" WITH GUSSETS (DETAIL D1/CA1) SECTION A-A HIGH POINT OF GRADE -/-LOW POINT OF GRADE ALL TOP OF FOOTINGS AT SAME ELEVATIONS 1'-10" \sim (4) #4 HORZ. BARS AT 16" CENTERS **EMBEDMENT** EACH WAY, (4) FOUR LAYERS ─#4 VERT. BARS AT CORNERS AND CENTER OF EACH SIDE 6" LOCAL FROST 3" CLEAR (TYP) LINE (MIN.) 4'-6" SQUARE **FOUNDATION DETAIL**

SCALE 3/8" = 1'-0"



DAVID E. SCHATTE 10609 99TH STREET OVERLAND PARK, KS. 66214 LICENSE #42792

0 3/29/23 TJH PERMIT / APPROVAL REV DATE BY DESCRIPTION

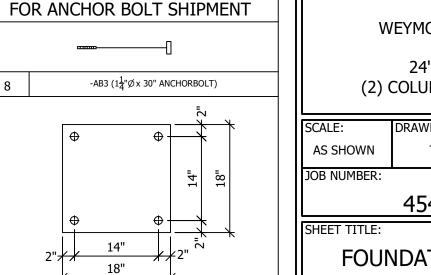
REVISION HISTORY



P.O. Box 90 Clayville, New York 13322 Phone: 315.867.9799

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UNBRANDED CANOPY



2'-6"

1 1/4"Ø x 30" LONG HEADED ANCHOR \neg

ROD WITH LEVELING NUTS & WASHERS

(ASTM F1554 GRADE 55) ON TOP OF &

BELOW BASE PLATE

ABT1 (TEMPLATE)

WOOD TEMPLATE

ANCHOR BOLTS

(4) REQ'D. PER FOOTING

₁8" THREADED

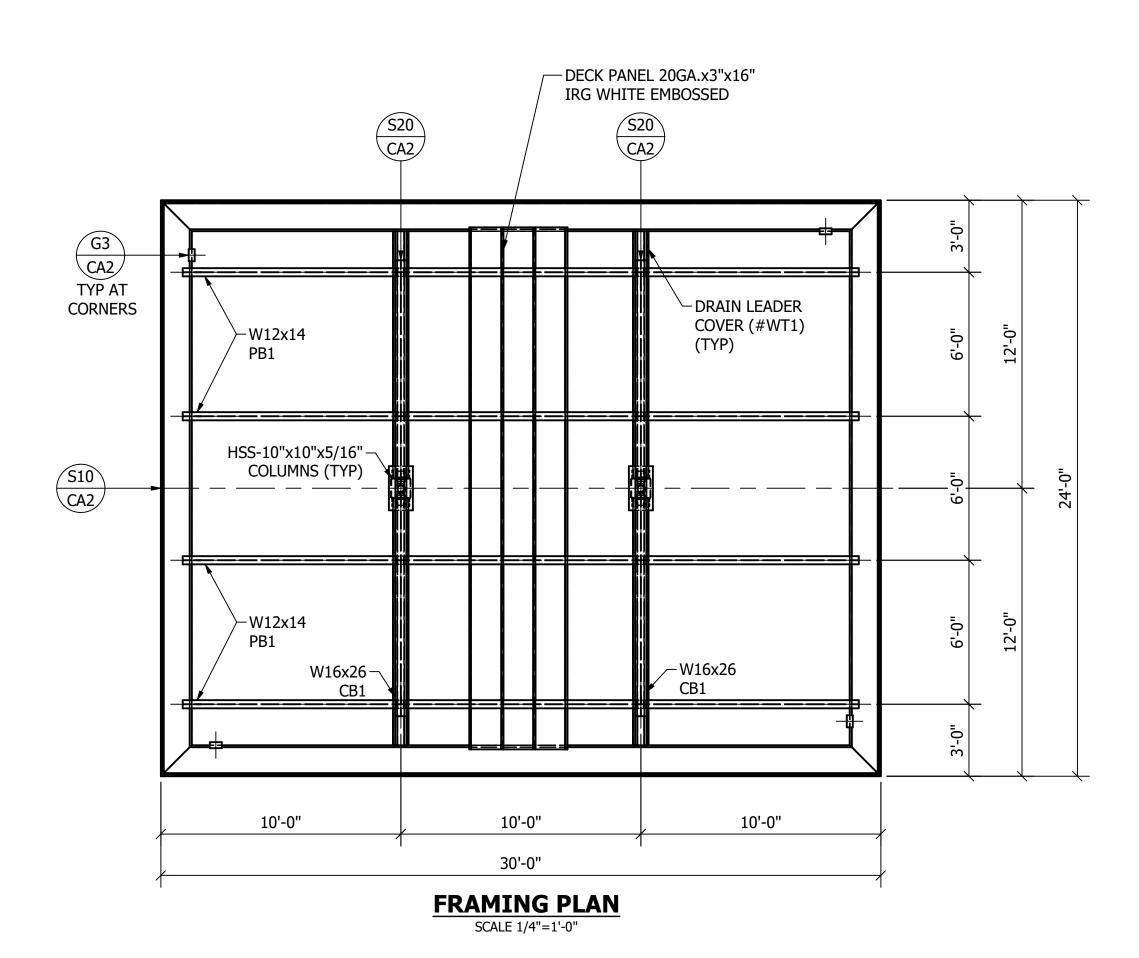
WEYMOUTH, MA

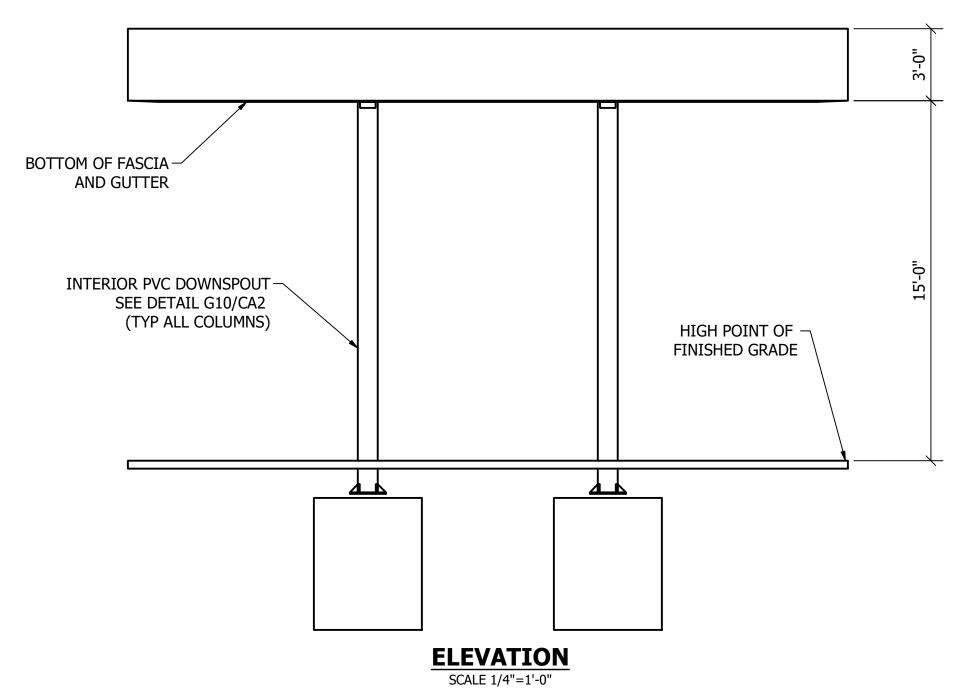
24' x 30' (2) COLUMN CANOPY DRAWN BY: CHECKED BY:

454610

FOUNDATION PLAN SHEET NUMBER:

CA1 of 2





- 1/4" STIFFENER

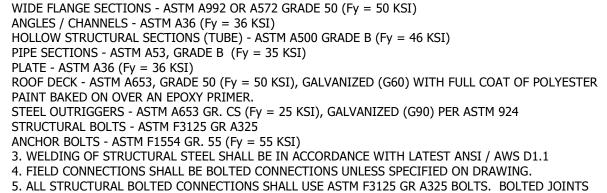
싉" GUSSET

- PL-3/4"x12"x22"

CAP PLATE

3" A-325 BOLTS & NUTS (8 PER CONNECTION)

(NS / FS)



1. DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE LATEST AISC

SPECIFICATIONS. DESIGN, FABRICATION AND ERECTION OF COLD FORMED STEEL SECTIONS SHALL

SHALL BE TIGHTENED TO SNUG TIGHT PER LATEST RCSC SPECIFICATION. 6. STRUCTURAL STEEL SHALL BE SHOP COATED WITH A RED-OXIDE RUST INHIBITIVE PRIMER. FIELD TOUCH-UP, FINISH PAINTING, AND MAINTENANCE SHALL BE THE RESPONSIBILITY OF THE OWNER (UNLESS OTHERWISE SPECIFIED).

7. DESIGN LOADS PER ASCE7-10 AND 2012 IBC:

CONFORM TO THE LATEST AISI SPECIFICATIONS.

2. STRUCTURAL MATERIALS:

RISK CATEGORY II

ROOF LIVE LOAD = 20 PSF (ERECTION & MAINTENANCE ONLY - NO PUBLIC ACCESS

FLAT ROOF SNOW LOAD = 35 PSF BASED ON GROUND SNOW LOAD= 35 PSF Is = 1.0; Ct = 1.2; Ce = 1.0

LATERAL = +/-35 PSF (MWFRS)(USING 0.6 W FOR ASD) UPLIFT = \pm +/- 20 PSF (MWFRS)(USING 0.6 W FOR ASD) BASED ON 132 MPH ULTIMATE WIND SPEED PER ASCE 7 EXPOSURE "C" IMPORTANCE FACTOR = 1.0; OPEN STRUCTURE, NO INTERNAL PRESSURE

SEISMIC RISK CATEGORY II, SITE CLADD "D", SEISMIC DESIGN CATEGORY "B" Sds = 0.22G (Ss = 0.20G, Fa = 1.6), Sd1 = 0.11G (S1 = 0.066G, Fv = 2.4) SEISMIC FORCE RESISTING SYSTEM IS INVERTED PENDULUM CANTERLEVERED COLUMN, R = 2.0 Cs = 0.11 DESIGN BASE SHEAR, $Cs \times W = 0.0.56$ K/COLUMN USING EQUIVILENT LATERAL FORCE PROCEDURE

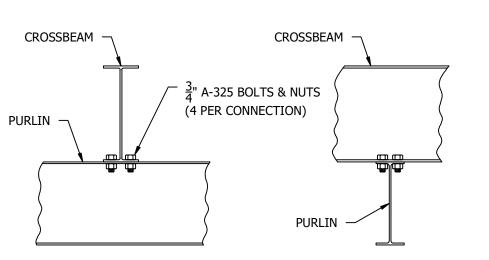
DEAD LOADS: DECK / GUTTER / LIGHTS - 5 PSF FASCIA - 10 PLF STRUCTURAL STEEL - SELF WT CONCRETE - 145 PCF

8. CANOPY USE GROUP "M" / CONSTRUCTION TYPE II-B

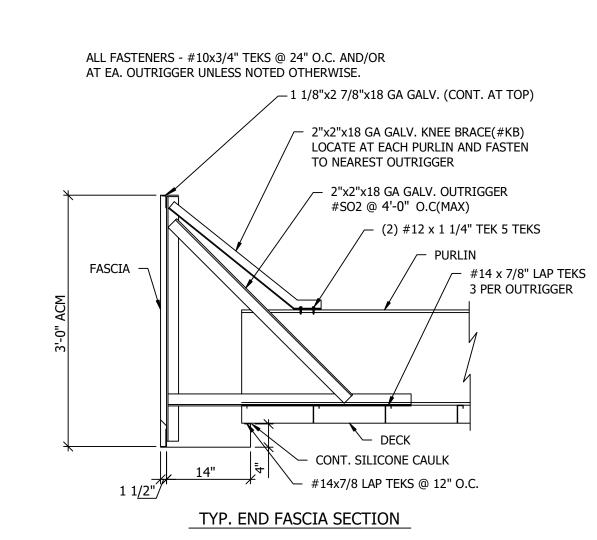
CROSSBEAM TO COLUMN CONNECTION

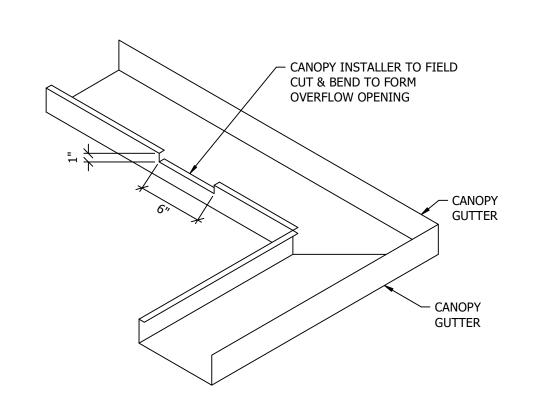
CROSSBEAM

CROSSBEAM

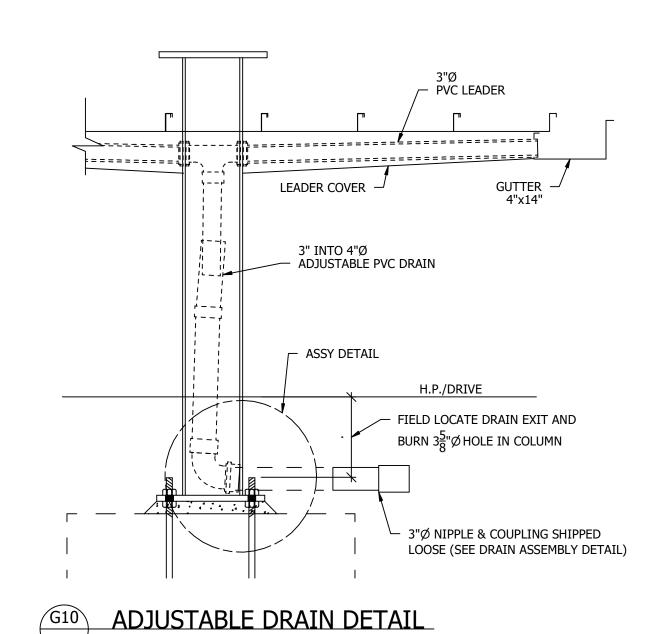


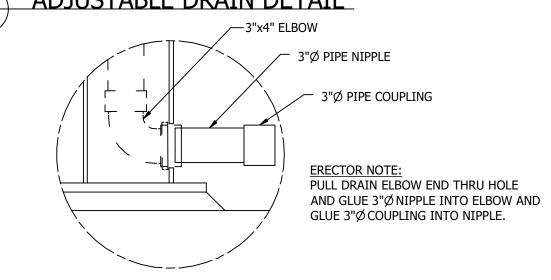
CROSSBEAM TO PURLIN CONNECTION





FIELD CUT GUTTER OVERFLOW DETAIL





DRAIN ASSEMBLY DETAIL





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UNBRANDED CANOPY

WEYMOUTH, MA

24' x 30'

(2) COLUMN CANOPY

454610

FRAMING PLAN

CA2 of 2

AS SHOWN

JOB NUMBER:

DRAWN BY: CHECKED BY:

0 3/29/23 TJH PERMIT / APPROVAL REV DATE BY DESCRIPTION REVISION HISTORY