## MEMORANDUM

TO: $\quad$ McDonald Keohane Funeral Home c/o Mr. Dennis Keohane 809 Main Street<br>Weymouth, MA 02190<br>DATE: January 19, 2022<br>RE: 8135<br>\(\begin{array}{ll}SUBJECT: \& Transportation Impact Assessment<br>\& Proposed Funeral Home Expansion - 809 Main Street<br>\& Weymouth, Massachusetts\end{array}\)

Vanasse \& Associates, Inc. (VAI) has conducted a Transportation Impact Assessment (TIA) in order to determine the potential impacts on the transportation infrastructure associated with the proposed expansion of the existing McDonald-Keohane Funeral Home, a licensed Funeral Establishment under 239 Code of Massachusetts Regulations (CMR) 3.00, located at 809 Main Street (Route 18) in Weymouth, Massachusetts (hereafter referred to as the "Project"). The proposed expansion is intended to update and modernize the mortuary operation to accommodate the increasing trend toward cremation and more intimate services at a funeral home vs. a traditional church service and burial. The funeral home currently accommodates both options (cremation and burial); however, as more families are requesting services in one day at a single location, the current capacity of the large gathering room (currently 50 persons) is not sufficient to accommodate the number of attendees (typically 60 persons on average), which requires attendees to wait in the hall or elsewhere within the building and not being able to participate in the service.

The Project is not intended to result in an increase in the frequency of services, the number of attendees (which can vary) or the number of clients served. As such, other than a positive benefit derived from eliminating or reducing the frequency of off-site parking, the Project will not result in an increase in traffic to the area. As such, our assessment focuses on the following areas: i) a review of the existing conditions of the transportation infrastructure serving the Project site; and ii) an evaluation of lines of sight at the Project site driveway intersections.

This assessment was conducted in general accordance with the Massachusetts Department of Transportation (MassDOT) Transportation Impact Assessment (TIA) Guidelines and the standards of the Traffic Engineering and Transportation Planning professions for the preparation of such reports. Based on this assessment, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner with the implementation of the recommendations defined herein.

The following details our findings with respect to the Project.

## PROJECT DESCRIPTION

The Project will entail the modernization and expansion of the McDonald-Keohane Funeral Home located at 809 Main Street (Route 18) in Weymouth, Massachusetts. Specifically, the Project includes the following elements: i) construction of a $4,300 \pm$ square foot (sf) addition to the existing $3,411 \pm$ sf funeral home; ii) construction of an enclosed attached garage for regulated removal, preparation and transportation under 239 CMR 3.10 ; iii) construction of a $2,516 \pm$ sf detached garage in western portion of the site to accommodate enclosed parking for a hearse and two (2) limousines and that are currently parked at the site; and iv) associated building access, parking, and circulation improvements in the western portion of the Project site. The Project site encompasses approximately 2.81 acres of land bounded by residential and commercial properties to the north and south; Route 18 to the east; and Cypress Street to the west. The western portion of the Project site currently contains a single-family home and associated appurtenances that will be removed to accommodate the Project.


Access to the Project site will continue to be provided by way of the two (2) existing driveways that intersect the west side of Route 18 approximately 375 feet and 160 feet north of Columbian Street, respectively, with the north driveway serving as a one-way entrance and the south driveway serving as a one-way, right-turn only exit. On-site parking will be provided for 117 vehicles, which exceeds the requirements of Section 120-74, Minimum required spaces, of the Town of Weymouth Zoning Ordinance. ${ }^{1}$

## EXISTING CONDITIONS CONTEXT

In order to establish the existing conditions context of the Project with respect to the transportation infrastructure, a comprehensive field inventory of existing conditions within the study area was conducted in January 2019. The field investigation consisted of an inventory of existing roadway geometrics and

[^0]pedestrian and bicycle facilities, as well as posted speed limits and land use information along Route 18 and Columbian Street in the vicinity of the Project site. The following provides a description of the transportation infrastructure serving the Project site.

## Roadways

## Route 18

Route 18 is a four-lane urban principal arterial under MassDOT jurisdiction that traverses the study area in a general north-south alignment providing a full access interchange with Route 3 (Exit 38) to the north of the Project. In the vicinity of the Project site, Route 18 provides four 12 -foot wide travel lanes separated by a painted median or a double-yellow centerline, with 4 -foot wide marked shoulders and additional turning lanes provided at major intersections. The posted speed limit along Route 18 within the study area is 35 miles per hour ( mph ). Prevailing travel speeds measured in January 2019 were found to be 30 mph in the northbound direction and 35 mph southbound. ${ }^{2}$ Illumination is provided by way of streetlights mounted on wood poles. Land use in the vicinity of the Project site consists of the Project site, residential and commercial properties, the South Shore Hospital and the Dana-Farber/Brigham and Women's Cancer Center.

## Columbian Street

Columbian Street is a two-lane urban minor arterial that is under Town jurisdiction and traverses the study area in a general northwest-southeast alignment between Grove Street and Pleasant Street. In the vicinity of the Project site, Columbian Street provides two 10 to 12 -foot wide travel lanes separated by a doubleyellow centerline with 3 to 4 -foot wide marked shoulders and additional turning lanes provided at major intersections. The posted speed limit along Columbian Street is 30 mph in the eastbound direction and 25 mph westbound. Prevailing travel speeds measured in January 2019 were found to be 35 mph in the eastbound direction and 38 mph westbound. Illumination is provided by way of streetlights mounted on wood poles. Land use in the vicinity of the Project site consists of residential and commercial properties, the South Shore Hospital and the Dana-Farber/Brigham and Women's Cancer Center.

## Pedestrian and Bicycle Facilities

Sidewalks are provided along both sides of Route 18 and Columbian Street, with marked crosswalks, pedestrian traffic signal equipment and phasing provided as a part of the traffic signal system at the Route 18/Columbian Street intersection. Formal bicycle facilities are not provided within the study area; however, Route 18 and Columbian Street provide sufficient width to accommodate bicycle travel in a shared traveled-way configuration (i.e., bicyclists and motor vehicles sharing the traveled-way). ${ }^{3}$

## Motor Vehicle Crash Data

A review of the MassDOT statewide High Crash Location List indicated that the Route 18/Columbian Street intersection is included on MassDOT's Highway Safety Improvement Program (HSIP) listing as a top 200 high crash location for 2015-2017. A Road Safety Audit (RSA) was completed in June 2011 along the Route 18 corridor that included this intersection. ${ }^{4}$ The RSA suggested a number of

[^1]
safety-related improvements at the Route 18/Columbian Street intersection that included the retiming of the traffic signal, many of which appear to have been completed.

The Project will include an expansion of the parking lot within the Project site to accommodate up to 132 vehicles. The expanded parking supply is intended to eliminate or reduce the frequency of occurrence of off-site parking for larger events and, as a result, will reduce traffic volumes and pedestrian activity at and in the vicinity of the Route 18/Columbian Street intersection.

## PROJECT-GENERATED TRAFFIC

As described herein, the Project will entail the expansion of the existing McDonald-Keohane Funeral Home to update and modernize the mortuary operation to accommodate the increasing trend toward cremation and one day, single location services at a funeral home vs. a traditional church service and burial. The Project is not intended to result in an increase in the frequency of services, the number of attendees (which can vary) or the number of clients served. As such, the Project will not result in an increase in traffic to the area.

In order to provide context with regard to traffic volumes and parking demands associated with the current funeral home operation, traffic volumes have been developed for the following conditions: i) typical operations without a service; ii) operations with a funeral/cremation/service; and iii) operations with two (2) concurrent visitations/wakes. Visitations/wakes are not scheduled to occur when a funeral or cremation service is scheduled.

On a typical day, there are eight (8) full-time employees on a weekday and three (3) full-time employees during the weekend, generally working between 7:00 AM and 5:00 PM. When a funeral or cremation is scheduled, there are four (4) additional part-time employees to assist with the service. Typically, a funeral or cremation service begins at 10:30 AM and has an approximate duration of 60 minutes, with the part-time employees generally arriving approximately 45 minutes prior to the service (typically 9:00 AM). The funeral home can also host two (2) concurrent visitations/wakes which are typically scheduled to begin at 4:00 PM on weekdays and at 2:00 PM on weekends.

A typical funeral or cremation service has approximately 60 guests that arrive in approximately 25 vehicles (or a vehicle occupancy ratio (VOR) of 2.4 persons per vehicle). When two (2) concurrent visitations/wakes are scheduled each event has approximately 30 family and friends that stay for the duration of the event, with up to 98 visitors (per visitation/wake) arriving and departing over the duration of the event, with a similar VOR to that associated with the funeral or cremation service ( 2.4 persons per vehicle).

Using these parameters, traffic volumes were derived for the three operational scenarios (typical day, funeral/cremation service and two (2) concurrent visitations/wakes) in order to describe the current and future operations at the funeral home with the planned improvements. In all cases, the resulting traffic volumes were increased by 10 percent in order to account for miscellaneous trips, such as deliveries, etc., and normal variations. Table 1 summarizes the resulting trip estimates with the detailed trip-generation calculations attached.

Table 1
TRIP-GENERATION SUMMARY
$\left.\begin{array}{lccc}\hline & & & \text { Vehicle Trips }\end{array}\right]$

## Traffic Volume Summary

Typical Day - Under typical daily operations, the funeral home generates approximately 40 vehicle trips on an average weekday and 14 vehicle trips on an average Saturday (both two-way volumes over the operational day of the Project), with 9 vehicle trips ( 1 vehicle entering and 8 exiting) during the weekday morning peak hour, 9 vehicle trips ( 8 vehicles entering and 1 exiting) during the weekday evening peak hour and 7 vehicle trips ( 4 vehicles entering and 3 exiting) during the Saturday midday peak hour.

Funeral/Cremation Service - When a funeral or cremation service is scheduled, the funeral home generates approximately 104 vehicle trips on an average weekday and 78 vehicle trips on an average Saturday (both two-way volumes over the operational day of the Project), with 9 vehicle trips ( 1 vehicle entering and 8 exiting) during the weekday morning peak hour, 9 vehicle trips ( 8 vehicles entering and 1 exiting) during the weekday evening peak hour and 39 vehicle trips ( 4 vehicles entering and 35 exiting) during the Saturday midday peak hour.

Two (2) Concurrent Visitations/Wakes - When two (2) concurrent visitations/wakes are scheduled, the funeral home generates approximately 258 vehicle trips on an average weekday and 232 vehicle trips on an average Saturday (both two-way volumes over the operational day of the Project), with 9 vehicle trips ( 1 vehicle entering and 8 exiting) during the weekday morning peak hour, 100 vehicle trips ( 71 vehicles entering and 29 exiting) during the weekday evening peak hour and 58 vehicle trips ( 55 vehicles entering and 3 exiting) expected during the Saturday midday peak hour.

As stated previously, the Project is not intended to result in an increase in the frequency of services, the number of attendees (which can vary) or the number of clients served, and, as such, will not result in an increase in traffic to the area.

The increased parking that will be provided within the site as a result of the Project will allow for visitors to park on-site and will eliminate or significantly reduce the frequency that off-site parking will occur. This is an improvement over current conditions where visitors park at adjacent properties for larger events.

## SIGHT DISTANCE ASSESSMENT

Sight distance measurements were performed at the Project site driveway intersections with Route 18 in accordance with MassDOT and American Association of State Highway and Transportation Officials (AASHTO) ${ }^{5}$ requirements. Both stopping sight distance (SSD) and intersection sight distance (ISD) measurements were performed. In brief, SSD is the distance required by a vehicle traveling at the design speed of a roadway, on wet pavement, to stop prior to striking an object in its travel path. ISD or corner sight distance (CSD) is the sight distance required by a driver entering or crossing an intersecting roadway to perceive an on-coming vehicle and safely complete a turning or crossing maneuver with on-coming traffic. In accordance with AASHTO standards, if the measured ISD is at least equal to the required SSD value for the appropriate design speed, the intersection can operate in a safe manner. Table 2 presents the measured SSD and ISD at the subject intersections.

[^2]Table 2
SIGHT DISTANCE MEASUREMENTS ${ }^{\text {a }}$

| Intersection/Sight Distance Measurement | Feet |  |  |
| :---: | :---: | :---: | :---: |
|  | Required <br> Minimum (SSD) | Desirable (ISD) ${ }^{\text {b }}$ | Measured |
| Route 18 at the North Project Driveway (Entrance) |  |  |  |
| Stopping Sight Distance: |  |  |  |
| Route 18 approaching from the north | 250 | -- | 500+ |
| Route 18 approaching from the south | 250 | -- | $500+$ |
| Route 18 at the South Project Driveway (Right-Turn Exit) |  |  |  |
| Stopping Sight Distance: |  |  |  |
| Route 18 approaching from the north | 250 | -- | 500+ |
| Route 18 approaching from the south | 250 | -- | $500+$ |
| Intersection Sight Distance: |  |  |  |
| ${ }^{\text {a Recommended minimum values obtained from A Policy on Geometric Design of Highways and Streets }}$ $7^{\text {th }}$ Edition; American Association of State Highway and Transportation Officials (AASHTO); 2018; and based on a 35 mph approach speed along Route 18 . |  |  |  |
| ${ }^{\mathrm{b}}$ Values shown are the intersection sight distance for a vehicle turning right or left exiting a roadway under STO control such that motorists approaching the intersection on the major street should not need to adjust their trave speed to less than 70 percent of their initial approach speed. |  |  |  |

As can be seen in Table 2, the available lines of sight to and from the Project site driveway intersections with Route 18 were found to exceed the recommended minimum sight distance to function in a safe (SSD) and efficient (ISD) manner based on a 35 mph approach speed, which is consistent with the posted speed limit along Route $18(35 \mathrm{mph})$ and up to 5 mph above the measured $85^{\text {th }}$ percentile vehicle travel speed approaching the driveways ( $30 / 35 \mathrm{mph}$ ).

## SUMMARY

VAI has completed a detailed assessment of the potential impacts on the transportation infrastructure associated with the proposed expansion of the existing McDonald-Keohane Funeral Home located at 809 Main Street (Route 18) in Weymouth, Massachusetts. As described herein, the Project is intended to update and modernize the mortuary operation to accommodate the increasing trend toward cremation and one day, single location services at a funeral home vs. a traditional church service and burial, and is not intended to result in an increase in the frequency of services, the number of attendees (which can vary) or the number of clients served. As such, other than a positive benefit derived from eliminating or reducing the frequency of off-site parking, the Project will not result in an increase in traffic to the area. Accordingly and with consideration that the lines of sight at the Project site driveway intersections exceed the required minimum distances for safe and efficient operation, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner with the implementation of the recommendations that follow.

## RECOMMENDATIONS

A detailed transportation improvement program has been developed that is designed to maintain safe and efficient access to the Project site. The following improvements have been recommended as a part of this evaluation and, where applicable, will be completed in conjunction with the Project subject to receipt of all necessary rights, permits, and approvals.

## Project Access

Access to the Project site will continue to be provided by way of the two (2) existing driveways that intersect the west side of Route 18 approximately 375 feet and 160 feet north of Columbian Street, respectively, with the north driveway serving as a one-way entrance and the south driveway serving as a one-way, right-turn only exit. The following recommendations are offered with respect to the design and operation of the Project site access and internal circulation, many of which are reflected on the Site Plans:

- The Project site driveways should continue to be a minimum of 20 feet in width and accommodate one-way travel, with the north driveway serving as a one-way entrance and the south driveway serving as a one-way exit this is restricted to right turn exiting movements only.
- Where perpendicular parking is proposed, the drive aisle behind the parking should be a minimum of 23 feet in order to facilitate parking maneuvers.
- Vehicles exiting the Project site should be placed under STOP-sign control with a marked STOPline provided. "DO NOT ENTER" and "ONE WAY" signs should be installed to regulate the oneway operation of the driveways.
- A "RIGHT TURN ONLY" sign should be installed facing exiting traffic for the south Project site driveway, with a "NO LEFT TURN" sign installed along the east side of Route 18 facing the driveway.
- All signs and pavement markings to be installed within the Project site should conform to the applicable standards of the Manual on Uniform Traffic Control Devices (MUTCD). ${ }^{6}$
- A sidewalk should be provided that links the funeral home building to the sidewalk along Route 18 , with marked crosswalks and Americans with Disabilities Act (ADA) wheelchair ramps provided at pedestrian crossings within the Project site.
- Signs and landscaping to be installed as a part of the Project within the intersection sight triangle areas of the Project site driveways should be designed and maintained so as not to restrict lines of sight.
- Snow accumulations (windrows) within sight triangle areas of the Project site driveways should be promptly removed where such accumulations would impede sight lines.

With the implementation of the above recommendations, safe and efficient access can be provided to the Project site and the Project can be accommodated within the confines of the existing transportation infrastructure.
cc: File

[^3]
## ATTACHMENTS

## PROJECT SITE PLAN

VEHICLE TRAVEL SPEED DATA
MASSDOT HIGH CRASH LOCATION MAPPING AND ROAD SAFETY AUDIT
TRIP GENERATION CALCULATIONS

PROJECT SITE PLAN


VEHICLE TRAVEL SPEED DATA

Location : Route 18
Location : North of Columbian Street
City/State: Weymouth, MA
8135SPD1


Daily

| 15th Percentile : | 14 MPH |
| ---: | ---: |
| 50th Percentile : | 20 MPH |
| 85th Percentile : | 30 MPH |
| 95th Percentile : | 34 MPH |
| Mean Speed(Average) : | 22 MPH |
| 10 MPH Pace Speed : | $15-24 \mathrm{MPH}$ |
| Number in Pace : | 6375 |
| Percent in Pace : | $51.7 \%$ |
| Number of Vehicles $>30 \mathrm{MPH}:$ | 1960 |
| Percent of Vehicles $>30 \mathrm{MPH}:$ | $15.9 \%$ |

Location : Route 18
Location : North of Columbian Street
City/State: Weymouth, MA
8135SPD1


Daily
50th Percentile 85th Percentile : 95th Percentile :

Mean Speed(Average)
0 MPH Pace Speed:
Number in Pace :
Percent in Pace :
14 MPH
20 MPH
29 MPH
34 MPH
22 MPH
13-22 MPH
6651
52.2\%

Number of Vehicles > 30 MPH
Percent of Vehicles > 30 MPH

Location : Route 18
Location : North of Columbian Street
City/State: Weymouth, MA
8135SPD1


Daily

| 15th Percentile : | 14 MPH |
| ---: | ---: |
| 50th Percentile : | 21 MPH |
| 85th Percentile : | 31 MPH |
| 95th Percentile : | 35 MPH |
| Mean Speed(Average) : | 23 MPH |
| 10 MPH Pace Speed : | $15-24 \mathrm{MPH}$ |
| Number in Pace : | 5695 |
| Percent in Pace : | $48.2 \%$ |
| Number of Vehicles $>30 \mathrm{MPH}:$ | 2407 |
| Percent of Vehicles $>30 \mathrm{MPH}:$ | $20.4 \%$ |


| Grand Total | 0 | 0 | 403 | 1549 | 4598 | 6546 | 5920 | 4704 | 3509 | 3299 | 3085 | 1883 | 975 | 395 | 36866 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Overall |  |  | 15th Percentile : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 50th Percentile : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 85th Percentile : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 95th Percentile : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Mean S | Speed(Average) : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 10 MP | MPH Pace Speed : | 15-2 |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | Number in Pace : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | Percent in Pace : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Number of Vehi | hicles > 30 MPH : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Percent of Vehi | hicles > 30 MPH : |  |  |  |  |  |  |  |  |  |  |  |  |

Location : Route 18
Location : North of Columbian Street
City/State: Weymouth, MA
8135SPD1
SB

| Start | 1 | 4 | 7 | 10 | 13 | 16 | 19 | 22 | 25 | 28 | 31 | 34 | 37 | 40 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | 3 | 6 | 9 | 12 | 15 | 18 | 21 | 24 | 27 | 30 | 33 | 36 | 39 | 999 | Total |
| 01/10/19 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 4 | 8 | 22 | 41 | 28 | 51 | 155 |
| 01:00 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 2 | 1 | 7 | 14 | 16 | 20 | 20 | 81 |
| 02:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 8 | 18 | 16 | 13 | 57 |
| 03:00 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 1 | 0 | 2 | 8 | 7 | 8 | 17 | 45 |
| 04:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 7 | 5 | 14 | 17 | 18 | 62 |
| 05:00 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 6 | 13 | 28 | 33 | 61 | 37 | 31 | 211 |
| 06:00 | 0 | 0 | 0 | 4 | 6 | 5 | 21 | 29 | 49 | 91 | 77 | 72 | 52 | 37 | 443 |
| 07:00 | 0 | 0 | 0 | 0 | 2 | 1 | 6 | 26 | 65 | 153 | 159 | 86 | 65 | 41 | 604 |
| 08:00 | 0 | 0 | 0 | 0 | 0 | 3 | 5 | 31 | 68 | 147 | 151 | 125 | 77 | 43 | 650 |
| 09:00 | 0 | 0 | 0 | 0 | 1 | 3 | 9 | 26 | 78 | 169 | 176 | 125 | 62 | 35 | 684 |
| 10:00 | 0 | 0 | 0 | 0 | 0 | 5 | 6 | 21 | 106 | 154 | 162 | 127 | 49 | 39 | 669 |
| 11:00 | 0 | 0 | 0 | 0 | 1 | 1 | 8 | 37 | 79 | 175 | 179 | 129 | 62 | 35 | 706 |
| 12 PM | 0 | 0 | 0 | 0 | 0 | 6 | 13 | 48 | 76 | 188 | 196 | 131 | 78 | 44 | 780 |
| 13:00 | 0 | 0 | 0 | 1 | 3 | 6 | 17 | 54 | 106 | 209 | 187 | 121 | 64 | 33 | 801 |
| 14:00 | 0 | 0 | 1 | 8 | 14 | 23 | 53 | 76 | 144 | 164 | 147 | 124 | 49 | 22 | 825 |
| 15:00 | 0 | 0 | 5 | 28 | 29 | 47 | 60 | 104 | 130 | 151 | 118 | 81 | 36 | 13 | 802 |
| 16:00 | 0 | 0 | 1 | 8 | 20 | 50 | 56 | 104 | 167 | 202 | 144 | 66 | 35 | 12 | 865 |
| 17:00 | 0 | 0 | 14 | 39 | 55 | 83 | 76 | 109 | 129 | 161 | 103 | 51 | 14 | 4 | 838 |
| 18:00 | 0 | 0 | 0 | 1 | 2 | 5 | 22 | 60 | 152 | 262 | 231 | 121 | 61 | 22 | 939 |
| 19:00 | 0 | 0 | 0 | 0 | 1 | 5 | 17 | 33 | 81 | 161 | 178 | 129 | 77 | 54 | 736 |
| 20:00 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 4 | 37 | 130 | 167 | 128 | 103 | 62 | 632 |
| 21:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 30 | 80 | 115 | 118 | 83 | 37 | 466 |
| 22:00 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 12 | 42 | 75 | 89 | 74 | 52 | 346 |
| 23:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 7 | 43 | 46 | 81 | 78 | 55 | 315 |
| Total | 0 | 0 | 21 | 89 | 135 | 246 | 372 | 782 | 1535 | 2735 | 2701 | 2061 | 1245 | 790 | 12712 |

Daily

50th Percentile 85th Percentile : 95th Percentile :

Mean Speed(Average) :
0 MPH Pace Speed :
Number in Pace :
Percent in Pace :
Number of Vehicles $>30 \mathrm{MPH}$ :
Percent of Vehicles > 30 MPH :

24 MPH
30 MPH
35 MPH
37 MPH
30 MPH
$27-36 \mathrm{MPH}$
8009
$63.0 \%$
6797
53.5\%

Location : Route 18
Location : North of Columbian Street
City/State: Weymouth, MA
8135SPD1
SB

| Start | 1 | 4 | 7 | 10 | 13 | 16 | 19 | 22 | 25 | 28 | 31 | 34 | 37 | 40 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | 3 | 6 | 9 | 12 | 15 | 18 | 21 | 24 | 27 | 30 | 33 | 36 | 39 | 999 | Total |
| 01/11/19 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 14 | 24 | 37 | 35 | 63 | 176 |
| 01:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 6 | 21 | 18 | 22 | 17 | 87 |
| 02:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 12 | 19 | 12 | 21 | 66 |
| 03:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 4 | 10 | 6 | 20 | 43 |
| 04:00 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 1 | 4 | 2 | 14 | 17 | 13 | 12 | 65 |
| 05:00 | 0 | 0 | 0 | 0 | 0 | 1 | 6 | 6 | 17 | 28 | 43 | 32 | 28 | 27 | 188 |
| 06:00 | 0 | 0 | 0 | 2 | 2 | 1 | 5 | 35 | 56 | 95 | 84 | 67 | 66 | 35 | 448 |
| 07:00 | 0 | 0 | 0 | 0 | 0 | 4 | 9 | 30 | 64 | 137 | 164 | 110 | 59 | 44 | 621 |
| 08:00 | 0 | 0 | 0 | 0 | 0 | 1 | 4 | 21 | 88 | 137 | 180 | 117 | 66 | 64 | 678 |
| 09:00 | 0 | 0 | 0 | 0 | 4 | 3 | 15 | 39 | 61 | 141 | 173 | 109 | 76 | 33 | 654 |
| 10:00 | 0 | 0 | 0 | 0 | 2 | 5 | 7 | 35 | 44 | 149 | 185 | 129 | 78 | 41 | 675 |
| 11:00 | 0 | 0 | 0 | 0 | 0 | 4 | 5 | 15 | 68 | 183 | 208 | 146 | 74 | 46 | 749 |
| 12 PM | 0 | 0 | 0 | 1 | 2 | 7 | 14 | 38 | 133 | 178 | 214 | 127 | 61 | 43 | 818 |
| 13:00 | 0 | 0 | 0 | 0 | 2 | 0 | 18 | 31 | 106 | 212 | 210 | 133 | 75 | 32 | 819 |
| 14:00 | 0 | 0 | 1 | 1 | 3 | 9 | 29 | 94 | 142 | 207 | 203 | 100 | 55 | 13 | 857 |
| 15:00 | 0 | 0 | 4 | 9 | 17 | 25 | 64 | 118 | 142 | 216 | 156 | 88 | 21 | 7 | 867 |
| 16:00 | 0 | 0 | 2 | 14 | 36 | 51 | 79 | 100 | 191 | 179 | 142 | 53 | 33 | 18 | 898 |
| 17:00 | 0 | 0 | 6 | 10 | 18 | 36 | 65 | 111 | 163 | 159 | 133 | 61 | 34 | 16 | 812 |
| 18:00 | 0 | 0 | 0 | 0 | 1 | 2 | 6 | 37 | 73 | 224 | 201 | 140 | 70 | 29 | 783 |
| 19:00 | 0 | 0 | 0 | 0 | 2 | 5 | 13 | 35 | 86 | 156 | 197 | 128 | 56 | 34 | 712 |
| 20:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 38 | 104 | 155 | 142 | 117 | 64 | 631 |
| 21:00 | 0 | 0 | 0 | 0 | 1 | 0 | 2 | 9 | 39 | 88 | 110 | 123 | 98 | 62 | 532 |
| 22:00 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 8 | 16 | 72 | 114 | 118 | 68 | 67 | 464 |
| 23:00 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 5 | 17 | 47 | 88 | 90 | 73 | 43 | 366 |
| Total | 0 | 0 | 13 | 37 | 90 | 156 | 345 | 781 | 1553 | 2738 | 3035 | 2114 | 1296 | 851 | 13009 |

Daily

| 15th Percentile : | 24 MPH |
| ---: | ---: |
| 50th Percentile : | 30 MPH |
| 85th Percentile : | 35 MPH |
| 95th Percentile : | 37 MPH |
|  |  |
| Mean Speed(Average) : | 30 MPH |
| 10 MPH Pace Speed : | $27-36 \mathrm{MPH}$ |
| Number in Pace : | 8405 |
| Percent in Pace : | $64.6 \%$ |
| Number of Vehicles $>30 \mathrm{MPH}:$ | 7296 |
| Percent of Vehicles > 30 MPH : | $56.1 \%$ |

Location : North of Columbian Street
City/State: Weymouth, MA
SB

| Start | 1 | 4 | 7 | 10 | 13 | 16 | 19 | 22 | 25 | 28 | 31 | 34 | 37 | 40 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | 3 | 6 | 9 | 12 | 15 | 18 | 21 | 24 | 27 | 30 | 33 | 36 | 39 | 999 | Total |
| 01/12/19 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 8 | 26 | 54 | 54 | 59 | 49 | 251 |
| 01:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 27 | 32 | 51 | 56 | 171 |
| 02:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 3 | 19 | 24 | 26 | 30 | 103 |
| 03:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 8 | 18 | 20 | 20 | 69 |
| 04:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 3 | 5 | 8 | 16 | 25 | 60 |
| 05:00 | 0 | 0 | 0 | 0 | 1 | 0 | 4 | 3 | 9 | 13 | 29 | 25 | 23 | 21 | 128 |
| 06:00 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 10 | 22 | 56 | 61 | 74 | 40 | 36 | 302 |
| 07:00 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 7 | 20 | 54 | 67 | 71 | 63 | 84 | 368 |
| 08:00 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 20 | 39 | 83 | 117 | 123 | 77 | 82 | 545 |
| 09:00 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 8 | 48 | 129 | 180 | 158 | 110 | 68 | 702 |
| 10:00 | 0 | 0 | 0 | 0 | 0 | 1 | 5 | 18 | 70 | 125 | 219 | 168 | 111 | 58 | 775 |
| 11:00 | 0 | 0 | 0 | 0 | 0 | 6 | 19 | 30 | 98 | 223 | 226 | 128 | 63 | 35 | 828 |
| 12 PM | 0 | 0 | 0 | 0 | 1 | 3 | 9 | 37 | 111 | 188 | 216 | 155 | 70 | 36 | 826 |
| 13:00 | 0 | 0 | 1 | 0 | 3 | 6 | 19 | 51 | 99 | 230 | 210 | 118 | 64 | 41 | 842 |
| 14:00 | 0 | 0 | 0 | 1 | 3 | 6 | 13 | 37 | 129 | 228 | 256 | 152 | 89 | 37 | 951 |
| 15:00 | 0 | 0 | 0 | 0 | 0 | 1 | 10 | 15 | 107 | 189 | 226 | 152 | 92 | 40 | 832 |
| 16:00 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 22 | 69 | 187 | 238 | 153 | 110 | 55 | 838 |
| 17:00 | 0 | 0 | 0 | 0 | 1 | 1 | 7 | 29 | 81 | 208 | 186 | 179 | 100 | 52 | 844 |
| 18:00 | 0 | 0 | 0 | 0 | 1 | 1 | 6 | 29 | 95 | 180 | 224 | 150 | 85 | 39 | 810 |
| 19:00 | 0 | 0 | 0 | 0 | 0 | 6 | 2 | 13 | 46 | 114 | 173 | 161 | 87 | 62 | 664 |
| 20:00 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 6 | 23 | 71 | 113 | 124 | 80 | 49 | 469 |
| 21:00 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 21 | 75 | 133 | 108 | 76 | 67 | 482 |
| 22:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 23 | 54 | 119 | 121 | 85 | 70 | 480 |
| 23:00 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 13 | 29 | 74 | 113 | 91 | 65 | 388 |
| Total | 0 | 0 | 1 | 1 | 10 | 32 | 113 | 347 | 1134 | 2476 | 3180 | 2569 | 1688 | 1177 | 12728 |


| 15th Percentile : | 27 MPH |
| ---: | ---: |
| 50th Percentile : | 31 MPH |
| 85th Percentile : | 35 MPH |
| 95th Percentile : | 37 MPH |
| Mean Speed(Average) : | 32 MPH |
| 10 MPH Pace Speed : | $28-37 \mathrm{MPH}$ |
| Number in Pace : | 8788 |
| Percent in Pace : | $69.0 \%$ |
| Number of Vehicles > 30 MPH : | 8614 |
| Percent of Vehicles > 30 MPH : | $67.7 \%$ |



Location : Route 18
Location : North of Columbian Street
City/State: Weymouth, MA
8135SPD1


Daily

| 15th Percentile : | 16 MPH |
| ---: | ---: |
| 50th Percentile : | 26 MPH |
| 85th Percentile : | 33 MPH |
| 95th Percentile : | 36 MPH |
| Mean Speed(Average) : | 26 MPH |
| 10 MPH Pace Speed : | $25-34 \mathrm{MPH}$ |
| Number in Pace : | 11076 |
| Percent in Pace : | $44.2 \%$ |
| Number of Vehicles $>30 \mathrm{MPH}:$ | 8757 |
| Percent of Vehicles $>30 \mathrm{MPH}:$ | $35.0 \%$ |

Location : Route 18
Location : North of Columbian Street
City/State: Weymouth, MA
8135SPD1
NB, SB


Daily

| 15th Percentile : | 16 MPH |
| ---: | ---: |
| 50th Percentile : | 26 MPH |
| 85th Percentile : | 33 MPH |
| 95th Percentile : | 36 MPH |
| Mean Speed(Average) : | 26 MPH |
| 10 MPH Pace Speed : | $25-34 \mathrm{MPH}$ |
| Number in Pace : | 11506 |
| Percent in Pace : | $44.7 \%$ |
| Number of Vehicles $>30 \mathrm{MPH}:$ | 9267 |
| Percent of Vehicles $>30 \mathrm{MPH}:$ | $36.0 \%$ |

Location : Route 18
Location : North of Columbian Street
City/State: Weymouth, MA
8135SPD1


Daily

| 15th Percentile : | 17 MPH |
| ---: | ---: |
| 50th Percentile : | 28 MPH |
| 85th Percentile : | 34 MPH |
| 95th Percentile : | 37 MPH |
| Mean Speed(Average) : | 27 MPH |
| 10 MPH Pace Speed : | $27-36 \mathrm{MPH}$ |
| Number in Pace : | 11982 |
| Percent in Pace : | $48.8 \%$ |
| Number of Vehicles > 30 MPH : | 11021 |
| Percent of Vehicles > 30 MPH : | $44.9 \%$ |



| Start | 1 | 16 | 21 | 26 | 31 | 36 | 41 | 46 | 51 | 56 | 61 | 66 | 71 | 76 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | 15 | 20 | 25 | 30 | 35 | 40 | 45 | 50 | 55 | 60 | 65 | 70 | 75 | 999 | Total |
| 01/10/19 | 0 | 0 | 1 | 1 | 4 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 |
| 01:00 | 0 | 0 | 2 | 2 | 5 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 11 |
| 02:00 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| 03:00 | 0 | 0 | 1 | 2 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 |
| 04:00 | 0 | 0 | 0 | 1 | 4 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 7 |
| 05:00 | 0 | 1 | 0 | 11 | 8 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 25 |
| 06:00 | 0 | 3 | 4 | 19 | 44 | 21 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 94 |
| 07:00 | 0 | 8 | 7 | 36 | 74 | 17 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 145 |
| 08:00 | 3 | 12 | 12 | 43 | 64 | 20 | 3 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 158 |
| 09:00 | 4 | 4 | 17 | 53 | 77 | 35 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 192 |
| 10:00 | 0 | 4 | 12 | 46 | 89 | 31 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 186 |
| 11:00 | 2 | 11 | 8 | 46 | 104 | 34 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 211 |
| 12 PM | 1 | 4 | 10 | 60 | 86 | 27 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 191 |
| 13:00 | 1 | 4 | 19 | 62 | 88 | 26 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 204 |
| 14:00 | 1 | 6 | 16 | 78 | 111 | 27 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 243 |
| 15:00 | 1 | 15 | 28 | 115 | 135 | 25 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 320 |
| 16:00 | 1 | 9 | 36 | 113 | 137 | 36 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 335 |
| 17:00 | 1 | 13 | 43 | 173 | 133 | 14 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 380 |
| 18:00 | 0 | 3 | 22 | 144 | 107 | 19 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 296 |
| 19:00 | 0 | 6 | 9 | 77 | 82 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 186 |
| 20:00 | 0 | 6 | 2 | 27 | 55 | 13 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 105 |
| 21:00 | 0 | 1 | 2 | 20 | 39 | 10 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 73 |
| 22:00 | 0 | 1 | 1 | 16 | 18 | 6 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 44 |
| 23:00 | 0 | 0 | 2 | 12 | 18 | 6 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 39 |
| Total | 15 | 111 | 254 | 1159 | 1484 | 391 | 45 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 3461 |

Daily

| 15th Percentile : | 25 MPH |
| ---: | ---: |
| 50th Percentile : | 30 MPH |
| 85th Percentile : | 34 MPH |
| 95th Percentile : | 38 MPH |
| Mean Speed(Average) : | 31 MPH |
| 10 MPH Pace Speed : | $26-35 \mathrm{MPH}$ |
| Number in Pace : | 2643 |
| Percent in Pace : | $76.4 \%$ |
| Number of Vehicles $>30 \mathrm{MPH}:$ | 1922 |
| Percent of Vehicles $>30 \mathrm{MPH}:$ | $55.5 \%$ |


| Start | 1 | 16 | 21 | 26 | 31 | 36 | 41 | 46 | 51 | 56 | 61 | 66 | 71 | 76 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | 15 | 20 | 25 | 30 | 35 | 40 | 45 | 50 | 55 | 60 | 65 | 70 | 75 | 999 | Total |
| 01/11/19 | 0 | 0 | 0 | 3 | 7 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 15 |
| 01:00 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| 02:00 | 0 | 0 | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 |
| 03:00 | 0 | 0 | 0 | 2 | 1 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 |
| 04:00 | 0 | 0 | 0 | 1 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 |
| 05:00 | 1 | 0 | 0 | 6 | 5 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 18 |
| 06:00 | 0 | 3 | 5 | 19 | 31 | 16 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 77 |
| 07:00 | 0 | 9 | 7 | 27 | 52 | 16 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 113 |
| 08:00 | 0 | 15 | 10 | 33 | 77 | 30 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 166 |
| 09:00 | 1 | 5 | 4 | 42 | 81 | 39 | 4 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 177 |
| 10:00 | 1 | 2 | 16 | 47 | 94 | 20 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 181 |
| 11:00 | 1 | 9 | 6 | 49 | 88 | 32 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 192 |
| 12 PM | 0 | 4 | 15 | 44 | 87 | 46 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 197 |
| 13:00 | 0 | 5 | 19 | 58 | 114 | 31 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 229 |
| 14:00 | 1 | 12 | 26 | 76 | 139 | 35 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 291 |
| 15:00 | 2 | 11 | 11 | 109 | 151 | 40 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 328 |
| 16:00 | 2 | 10 | 25 | 133 | 145 | 33 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 351 |
| 17:00 | 2 | 8 | 29 | 144 | 113 | 21 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 318 |
| 18:00 | 1 | 6 | 17 | 105 | 124 | 20 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 274 |
| 19:00 | 0 | 1 | 17 | 59 | 79 | 15 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 174 |
| 20:00 | 0 | 2 | 7 | 39 | 48 | 20 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 119 |
| 21:00 | 1 | 6 | 4 | 28 | 41 | 13 | 3 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 97 |
| 22:00 | 0 | 1 | 5 | 10 | 39 | 7 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 64 |
| 23:00 | 0 | 2 | 0 | 12 | 20 | 8 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 45 |
| Total | 13 | 111 | 223 | 1047 | 1540 | 460 | 44 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 3443 |



Daily

| 15th Percentile : | 26 MPH |
| ---: | ---: |
| 50th Percentile : | 31 MPH |
| 85th Percentile : | 36 MPH |
| 95th Percentile : | 39 MPH |
| Mean Speed(Average) : | 32 MPH |
| 10 MPH Pace Speed : | $26-35 \mathrm{MPH}$ |
| Number in Pace : | 2173 |
| Percent in Pace : | $72.8 \%$ |
| Number of Vehicles $>30 \mathrm{MPH}:$ | 2003 |
| Percent of Vehicles $>30 \mathrm{MPH}:$ | $67.1 \%$ |



Location : Columbian Street
Location : East of Cypress Street
City/State: Weymouth, MA
8135SPD2

## WB



Daily

| 15th Percentile : | 26 MPH |
| ---: | ---: |
| 50th Percentile : | 32 MPH |
| 85th Percentile : | 38 MPH |
| 95th Percentile : | 40 MPH |
| Mean Speed(Average) : | 33 MPH |
| 10 MPH Pace Speed : | $31-40 \mathrm{MPH}$ |
| Number in Pace : | 2283 |
| Percent in Pace : | $68.6 \%$ |
| Number of Vehicles $>30 \mathrm{MPH}:$ | 2451 |
| Percent of Vehicles $>30 \mathrm{MPH}:$ | $73.6 \%$ |

## WB



| 15th Percentile : | 27 MPH |
| ---: | ---: |
| 50th Percentile : | 33 MPH |
| 85th Percentile : | 38 MPH |
| 95th Percentile : | 40 MPH |
|  |  |
| Mean Speed(Average) : | 33 MPH |
| 10 MPH Pace Speed : | $31-40 \mathrm{MPH}$ |
| Number in Pace : | 2383 |
| Percent in Pace : | $71.3 \%$ |
| Number of Vehicles $>30 \mathrm{MPH}:$ | 2576 |
| Percent of Vehicles $>30 \mathrm{MPH}:$ | $77.1 \%$ |



| EB, WB |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Start | 1 | 16 | 21 | 26 |
| Time | 15 | 20 | 25 | 30 |
| 01/10/19 | 0 | 0 | 2 | 4 |
| 01:00 | 0 | 0 | 2 | 3 |
| 02:00 | 0 | 0 | 0 | 3 |
| 03:00 | 0 | 0 | 2 | 3 |
| 04:00 | 0 | 0 | 0 | 4 |
| 05:00 | 0 | 1 | 0 | 15 |
| 06:00 | 1 | 5 | 8 | 44 |
| 07:00 | 0 | 10 | 17 | 70 |
| 08:00 | 6 | 24 | 24 | 84 |
| 09:00 | 5 | 8 | 29 | 102 |
| 10:00 | 0 | 6 | 16 | 72 |
| 11:00 | 2 | 17 | 24 | 80 |
| 12 PM | 1 | 10 | 17 | 100 |
| 13:00 | 2 | 7 | 28 | 99 |
| 14:00 | 3 | 17 | 28 | 117 |
| 15:00 | 3 | 25 | 42 | 164 |
| 16:00 | 1 | 16 | 50 | 171 |
| 17:00 | 2 | 21 | 69 | 238 |
| 18:00 | 0 | 6 | 29 | 186 |
| 19:00 | 0 | 8 | 17 | 99 |
| 20:00 | 0 | 12 | 3 | 41 |
| 21:00 | 0 | 2 | 4 | 37 |
| 22:00 | 0 | 3 | 3 | 24 |
| 23:00 | 0 | 0 | 3 | 15 |
| Total | 26 | 198 | 417 | 1775 |
| Daily | 15th Percentile : 50th Percentile : 85th Percentile : 95th Percentile : |  |  | 26 MPH |
|  |  |  |  | 31 MPH |
|  |  |  |  | 36 MPH |
|  |  |  |  | 39 MPH |
|  | Mean Speed(Average) :10 MPH Pace Speed : |  |  | 32 MPH |
|  |  |  |  | 26-35 MPH |
|  | Number in Pace : |  |  | 4714 |
|  | Percent in Pace : |  |  | 69.4\% |
|  | Number of Vehicles > 30 MPH : |  |  | 4373 |
|  | Percent of Vehicles > 30 MPH : |  |  | 64.4\% |


| EB, WB |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Start | 1 | 16 | 21 | 26 |
| Time | 15 | 20 | 25 | 30 |
| 01/11/19 | 0 | 0 | 2 | 6 |
| 01:00 | 0 | 0 | 0 | 1 |
| 02:00 | 0 | 0 | 1 | 0 |
| 03:00 | 0 | 0 | 0 | 4 |
| 04:00 | 0 | 0 | 0 | 5 |
| 05:00 | 1 | 0 | 0 | 18 |
| 06:00 | 2 | 5 | 9 | 38 |
| 07:00 | 1 | 15 | 19 | 64 |
| 08:00 | 0 | 21 | 26 | 67 |
| 09:00 | 1 | 12 | 8 | 63 |
| 10:00 | 1 | 4 | 23 | 77 |
| 11:00 | 3 | 12 | 23 | 86 |
| 12 PM | 0 | 8 | 22 | 71 |
| 13:00 | 0 | 8 | 25 | 87 |
| 14:00 | 3 | 17 | 35 | 106 |
| 15:00 | 5 | 16 | 19 | 139 |
| 16:00 | 3 | 11 | 37 | 189 |
| 17:00 | 4 | 18 | 38 | 185 |
| 18:00 | 2 | 10 | 26 | 142 |
| 19:00 | 0 | 3 | 23 | 86 |
| 20:00 | 2 | 3 | 14 | 62 |
| 21:00 | 1 | 9 | 10 | 44 |
| 22:00 | 0 | 3 | 7 | 20 |
| 23:00 | 0 | 5 | 1 | 21 |
| Total | 29 | 180 | 368 | 1581 |
| Daily | 15th Percentile 50th Percentile 85th Percentile 95th Percentile |  |  | 26 MPH |
|  |  |  |  | 32 MPH |
|  |  |  |  | 37 MPH |
|  |  |  |  | 39 MPH |
|  | Mean Speed(Average) |  |  | 32 MPH |
|  | 10 MPH Pace Speed: |  |  | 26-35 MPH |
|  | Number in Pace : |  |  | 4600 |
|  | Percent in Pace : |  |  | 67.8\% |
|  | Number of Vehicles > 30 MPH : |  |  | 4625 |
|  | Percent of Vehicles > 30 MPH : |  |  | 68.2\% |



Daily

| 15th Percentile : | 27 MPH |
| ---: | ---: |
| 50th Percentile : | 32 MPH |
| 85th Percentile : | 37 MPH |
| 95th Percentile : | 39 MPH |
| Mean Speed(Average) : | 33 MPH |
| 10 MPH Pace Speed : | $31-40 \mathrm{MPH}$ |
| Number in Pace : | 3968 |
| Percent in Pace : | $69.2 \%$ |
| Number of Vehicles $>30 \mathrm{MPH}:$ | 4236 |
| Percent of Vehicles $>30 \mathrm{MPH}:$ | $73.9 \%$ |


| Grand Total | 81 | 517 | 1041 | 4432 | 8506 | 4003 | 651 | 64 | 8 | 1 | 1 | 0 | 0 | 0 | 19305 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Overall |  |  | 15th Percentile : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 50th Percentile : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 85th Percentile : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 95th Percentile : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Mean | Speed(Average) : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 10 MP | PH Pace Speed : | 26-3 |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | Number in Pace : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | Percent in Pace : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Number of Vehi | icles > 30 MPH : |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Percent of Vehi | hicles > 30 MPH : |  |  |  |  |  |  |  |  |  |  |  |  |

## MassDOT Top Crash Locations



# ROAD SAFETY AUDIT 

# Main Street (Route 18) at Various Locations 

Town of Weymouth

June 2011

Prepared for:
Massachusetts Department of Transportation

Prepared by: Howard/Stein-Hudson Associates 38 Chauncy Street Boston, MA 02111

Table 2. Summary of Potential Safety Enhancements

| Location | Safety Enhancement | Safety Payoff | Time Frame | Cost | Responsibility |
| :---: | :---: | :---: | :---: | :---: | :---: |
| General Observations | Conduct statistical review of crashes throughout the corridor to identify outliers in trends (e.g., time of day, day of week, etc.). | Medium | Short-term | Low | MassDOT |
|  | Review minimum green times and clearance intervals on a corridor-wide basis taking into account grade and heavy vehicles, per Institute of Transportation Engineers (ITE) standards. | High | Short-term | Low | MassDOT |
| Intersection \#1: Main Street (Route 18)/Middle Street/West Street | Evaluate the appropriateness of providing protected only phasing for left-turns at the intersection. Consider the impacts on capacity versus the resulting safety benefits and the need for additional equipment (i.e., additional left-turn signal head for Route 18 northbound). | High | Short-term | Medium | MassDOT |
|  | Consider the appropriateness of adding supplemental indications on the far-side of the intersection for left turning vehicles. | Medium | Long-term | Medium | MassDOT |
|  | Review, and adjust as necessary, all minimum green times and clearance intervals taking into account grade and heavy vehicles, per Institute of Transportation Engineers (ITE) standards to help reduce the occurrence of rear-end and angle type crashes. | High | Short-term | Low | MassDOT |
|  | Consider the appropriateness of installing a Dilemma-Zone Detection system to help reduce the frequency of red-light violations; crashes associated with the traffic signal phase change (e.g., rear-end and angle crashes), and reduce delay and stop frequency on the major road. | Medium | Long-term | Medium | MassDOT |
|  | Realign traffic signal faces on span wire to improve visibility at each approach, and/or to center over travel lane, as appropriate/feasible. | Low | Short-term | Low | MassDOT |
|  | Consider the appropriateness of tightening the radii at the corners of the intersection to reduce the turning speed of rightturning vehicles and improve safety. Consider heavy vehicle turning requirements and the impact (possible reduction) on clearance interval requirements. | Low | Long-term | High | MassDOT |


| Location | Safety Enhancement | Safety Payoff | Time Frame | Cost | Responsibility |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection \#1: Main Street (Route 18)/Middle Street/West Street (continued) | Re-stripe merge area along the Middle Street eastbound travel lane, to the east of the intersection. | Low | Short-term | Low | MassDOT |
| Intersection \#2: Main <br> Street (Route <br> 18)/Park <br> Avenue/Park Avenue <br> West | Prohibit left-turns into Dunkin Donuts from Route 18 northbound by installing No Left Turn signage (i.e., R3-2) to encourage patrons to use the entrance via the traffic signal on Park Avenue West. | Medium | Short-term | Low | MassDOT |
|  | Consider the appropriateness/feasibility of installing a median along the center of Route 18 to physically prohibit left-turns into driveways. Consider emergency vehicle access. | Medium | Long-term | High | MassDOT |
|  | Reduce slope/grade change at Dunkin Donuts driveway to improve traffic flow into the site. Consider the appropriateness of replacing driveway apron with a curb cut and crosswalk or feasibility of re-grading the existing apron. | Medium | Short-term | Medium | MassDOT/ Dunkin Donuts |
|  | Evaluate the appropriateness of providing protected only phasing for left-turns at the intersection. Consider the impacts on capacity versus the resulting safety benefits. | High | Short-term | Medium | MassDOT |
|  | Consider the appropriateness of adding supplemental indications on the far-side of the intersection for left turning vehicles. | Medium | Long-term | Medium | MassDOT |
|  | Review, and adjust as necessary, all minimum green times and clearance intervals taking into account grade and heavy vehicles, per Institute of Transportation Engineers (ITE) standards to help reduce the occurrence of rear-end and angle type crashes. | High | Short-term | Low | MassDOT |


| Location | Safety Enhancement | Safety Payoff | Time Frame | Cost | Responsibility |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection \#3: Main Street (Route 18)/Columbian Street | Review, and adjust as necessary, all minimum green times and clearance intervals taking into account grade and heavy vehicles, per Institute of Transportation Engineers (ITE) standards to help reduce the occurrence of rear-end and angle type crashes. | High | Short-term | Low | MassDOT |
|  | Evaluate the appropriateness of providing protected only phasing for left-turns at the intersection. Consider the impacts on capacity versus the resulting safety benefits. | High | Short-term | Medium | MassDOT |
|  | Consider the appropriateness of adding supplemental indications on the far-side of the intersection for left turning vehicles. | Medium | Long-term | Medium | MassDOT |
| Intersection \#4: Main Street (Route 18)/Pond Street/Pleasant Street/Pleasant Street Shopping Center Driveway | Rotate Route 18 northbound signal head and No Left Turn sign (R3-2), where feasible, so that they are not/less visible from Pond Street eastbound approach (short-term). | Low | Short-term | Low | MassDOT |
|  | Rotate the Pleasant Street westbound signal head, where feasible, so that it is not/less visible from the Route 18 northbound approach | Low | Short-term | Low | MassDOT |
|  | Consider pavement marking enhancements at the Shopping Center Driveway intersection to make it "look" more like an intersection (short-term). Possible modifications for consideration may include changing the solid lane line to a dotted line, eliminating the solid edge line, changing the solid double yellow center line to a dotted yellow center line, and adding "do not block the box" signage and a box pavement marking, where enforceable. | Medium | Short-term | Low | MassDOT |
|  | Consider reducing the length of the right-turn lane so that the Route 18 northbound approach is only two (through) lanes in the vicinity of the Shopping Center Driveway (long-term). | High | Long-term | High | MassDOT |


| Location | Safety Enhancement | Safety Payoff | Time Frame | Cost | Responsibility |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection \#5: Main Street (Route 18)/Pond Street/Route 58 | Pull back median along the southern leg of the intersection that separates Route 18 northbound and southbound traffic to facilitate left turns from Route 58 north-westbound onto Route 18 southbound and maintain Keep Right signage (i.e., R4-7) | Medium | Long-term | Medium | MassDOT |
|  | Remove/relocate signage, where feasible, that obstructs sight lines to the south of the CVS driveway in the short-term and relocate the traffic signal controller and any remaining signage that obstructs sight lines as part of the redesign (long-term). | Low | Short-longterm | Low/High | MassDOT |
|  | Evaluate the appropriateness of providing protected only phasing for left-turns at the intersection. Consider the impacts on capacity versus the resulting safety benefits. | High | Short-term | Medium | MassDOT |
|  | Consider the appropriateness of adding supplemental indications on the far-side of the intersection for left turning vehicles. | Medium | Long-term | Medium | MassDOT |
|  | Review, and adjust as necessary, all minimum green times and clearance intervals taking into account grade and heavy vehicles, per Institute of Transportation Engineers (ITE) standards to help reduce the occurrence of rear-end and angle type crashes. | High | Short-term | Low | MassDOT |
|  | Review the Route 18 southbound left-turn volumes and consider the appropriateness of adding a dual left-turn and the impacts on adjacent property (e.g., need for land acquisition) or investigate the travel time/conditions of the parallel route (Rout e 18 vs. Route 58) for potential signal timing changes. | Medium | Long-term | High | MassDOT |
|  | Replace existing incandescent traffic signal bulbs with LED indications as part of the reconstruction. | Low | Long-term | Low | MassDOT |

## Typical Daily Operations - McDonald Keohane Funeral Home, South Weymouth

We are a 7 day per week operation. Monday through Friday, our team begins arriving at the funeral home at 7:30 AM. In a typical workday, our full time staff leaves work at 4:30 PM. On any given day, we have about 8 full time employees in the office.

We have a reduced staff on weekends. On Saturdays, we typically arrive at 8:30 and on Sundays, we typically arrive at 10 AM. Our full time weekend staff is typically 3 people.

On mornings that we have funeral services, our part time support staff arrives about 1.5 hours prior to the time of the funeral. Our part time support teams typically consist of 4 people.

When families decide to have their loved one's funeral in a church, we typically gather in the funeral home prior to leaving in procession for church. Our support team lines up the cars and handles traffic control as we exit the parking lot. The average size of a funeral procession is about 25 cars with about 60 people, but this can vary greatly.

More and more families are choosing to have their loved one's services in the funeral home. Since more of these same families are choosing cremation, we have significantly less funeral processions leaving the parking lot. An average service in the funeral home would have about 60 attendees. Currently, our largest room can comfortably hold about 50 attendees. Often times, people have to stand in the hallway or sit on the other side of the building where they can't see the service. After a funeral home service that is followed by cremation, everybody (other than the immediate family) tends to leave the services at the same time. In these situations, our support staff handles traffic control just as they would if we were leaving in procession.

Our hope with the addition is to better serve the growing number of families that are choosing to have their loved one's service in the funeral home followed by (or preceded by) cremation. There is a real need for people to gather together in the same room for these services. It makes it a significantly more comfortable environment where families can better feel the support of the community around them. These types of services would be in the morning.

Our afternoon services consist of visiting hours. This is a less formal gathering of family and friends to share stories and offer condolences. Guests typically arrive in a 4 hour window. Most families choose the 4-8 PM hour timeframe on weekdays. On Sundays, most families choose 26 PM. Although there are rare exceptions, the vast majority of families leave 15 to 20 minutes after the end of the visiting hours. Once the cars leave the lot, our staff turns off the parking lot lights. The number of cars varies greatly for each visitation. Currently, if our lot gets full, people tend to park in the neighboring medical office properties. This is not appreciated by the neighbors. In our new parking plan, we want to have the capacity to fully cover all parking needs for over $99 \%$ of all visitations, relieving the need for people to pull back out onto Main Street in search of an offsite parking spot.

We have two limousines, 1 formal hearse and two minivans (that are used to transfer deceased people into our care from homes, hospitals and nursing homes) on property. The formal hearse and limousines would be garaged. They are only used for morning funerals so they would stay
garaged for the rest of the day. In the new addition, one of the mini vans would stay inside the attached garage on the hospital side of the property. The minivans go out 24 hours per day, but since they would be kept on the hospital side of the property, headlights would rarely be seen on the residential side of the property. Currently, our minivans are not under cover and we park them on the residential side of the property (not an ideal situation).

Funeral service is not like other business. The death rate controls how busy we are at any given time. Families are typically very loyal to the funeral home that has served them in the past, so it is exceedingly difficult to serve new families. Because of this, we do not expect a sizable growth in our business. Also, because there is a growing segment that is deciding not have services at all, our facility is not being used for every family like it was in the past. The main goal of this addition is to better serve the growing segment of families that prefer the "one stop shopping" experience. The want to comfort and convenience of a modern building with ample parking. Increasingly, more families are rejecting the traditional, old fashioned funeral home model. There is still a great need for families to gather with friends and family when a loved one dies, but the surroundings need to be more open and filled with natural light with modern conveniences.

## Trip Generation Estimates

## Operational Characteristics:

Hours of Operation $=7: 00$ AM - 5: 00 PM; Weekdays
8: 00 AM - 5: 00 PM; Saturday
10: 00 AM - 5: 00 PM; Sunday
On appointment outside of these hours
Full - Time Employees $=8$ (Weekday); 3 (Weekends)
Note: Full-Time staff are assumed to make 4 total site trips; 1 entering from residence, 1 exiting to break, 1 entering from break, and 1 exiting to residence.
Part - Time Employees $=4$ to support events, arriving 45 minutes prior to event
Note: Part-Time staff are assumed to make 2 total site trips; 1 entering from residence and 1 exiting to residence.

## Typical Funeral/Cremation Characteristics:

Note: Funeral and Cremation services are virtually identical from a transportation operations perspective
Note: One Funeral/Cremation service is held at the facility at a time
Funeral/Cremation Services typically begin at 10: 30 AM and last 1 hour
Funeral/Cremation Services typically have 60 guests, arriving in 25 vehicles.
$V O R=\frac{60 \text { guests }}{25 \text { vehicles }}=2.40$

## Typical Visitation/Wake Characteristics:

Note: Two Visitation/Wake services can occur simultaneously
Note: Funeral/Cremation and Visitation/Wake services do not occur on the same day
Visitation/Wake Services typically begin at 4: 00 PM on weekdays and 2: 00 PM on weekends
Visitation/Wake Services typically last 4 hours
Visitation/Wake Services were assumed to have approxiamtely 196 guests

- The family members and/or close personal relations typically attend the entire wake, with the remaining guests rotating throughout.
- For the purposes of this estimation the same VOR was assumed.
- It was assumed that half of the Funeral/Cremation guests would be family members and/or close personal relations.
- Per the funeral director, the proposed parking lot will hold the full balance of the wake/visitation guests; currently parking occurs on neighboring parcels. The site will provide 117 parking spaces, 4 will be used by facility vehicles, 25 by the family/close personal guests, leaving a total of 88 spaces for visitation/wake guests. An $80 \%$ parking lot occupancy rate was used
- $88 \times 0.80=70.4 \approx 70$ Spaces remianing for the two Visitations/Wakes.
- 35 Vehicles $\times 2.40$ VOR $\approx 84$ Guests per single Visitation/Wake
- It was assumed the remainder of the wake/visitation guests (not family/close personal) would rotate, with a duration of approximately 1 hour.


## General Assumptions:

Weekday Morning Peak Period $=7: 00-9: 00$ AM
Weekday Evening Peak Period $=4: 00-6: 00$ PM
Saturday Midday Peak Period $=11: 00$ AM $-2: 00$ PM

Average Weekday Daily Trip Generation:
8 Full - Time Employees $\times 4$ Trips (50\% In; 50\% Out) $=36$
Assume $10 \%$ for Misc.Trips $=36 \times 1.10=39.6 \approx 40$ (20 Enter; 20 Exit)
Average Weekday Morning Peak Hour Trip Generation:
8 Full - Time Employees $\times 1$ Trip ( $\sim 90 \%$ In; $\sim 10 \%$ Out) $=8$
Assume $10 \%$ for Misc.Trips $=8 \times 1.10=8.8 \approx 9$ ( 8 Enter; 1 Exit $)$
Average Weekday Evening Peak Hour Trip Generation:
8 Full - Time Employees $\times 1$ Trip ( $\sim 10 \%$ In; $\sim 90 \%$ Out) $=8$
Assume $10 \%$ for Misc.Trips $=8 \times 1.10=8.8 \approx 9$ (1 Enter $; 8$ Exit $)$
Average Saturday Trip Generation:
3 Full - Time Employees $\times 4$ Trips (50\% In; 50\% Out) $=12$
Assume $10 \%$ for Misc.Trips $=12 \times 1.10=13.2 \approx 14$ (7 Enter; 7 Exit)
Average Saturday Midday Peak Hour Trip Generation:
Note: Consists of the break switchover of Full-Time staff.
3 Full - Time Employees $\times 2$ Trips ( $50 \%$ In; $50 \%$ Out) $=6$
Assume $10 \%$ for Misc.Trips $=6 \times 1.10=6.6 \approx 7$ (4 Enter; 3 Exit)

Average Weekday Daily Trip Generation:
8 Full - Time Employees $\times 4$ Trips ( $50 \%$ In; $50 \%$ Out) $=36$
4 Part - Time Employees $\times 2$ Trips ( $50 \%$ In; $50 \%$ Out) $=\mathbf{8}$
$\frac{60 \text { Guests }}{2.40 \text { VOR }} \times 2$ Trips $(50 \%$ In; $50 \%$ Out $)=\mathbf{5 0}$
Assume $10 \%$ for Misc.Trips $=(36+8+50) \times 1.10=103.4 \approx 104(52$ Enter; 52 Exit $)$
Average Weekday Morning Peak Hour Trip Generation:
Note: Consists of the arrival of Full-Time staff only.
8 Full - Time Employees $\times 1$ Trip $(\sim 90 \%$ In; $\sim 10 \%$ Out $)=\mathbf{8}$
Assume $10 \%$ for Misc.Trips $=8 \times 1.10=8.8 \approx 9$ ( Enter $; 1$ Exit $)$
Average Weekday Evening Peak Hour Trip Generation:
Note: Consists of the departure of Full-Time staff only.
8 Full - Time Employees $\times 1$ Trip $(\sim 10 \%$ In; $\sim 90 \%$ Out $)=8$
Assume $10 \%$ for Misc.Trips $=8 \times 1.10=8.8 \approx 9$ (1 Enter $; 8$ Exit $)$
Average Saturday Trip Generation:
3 Full - Time Employees $\times 4$ Trips (50\% In; 50\% Out) $=12$
4 Part - Time Employees $\times 2$ Trips ( $50 \%$ In; $50 \%$ Out) $=\mathbf{8}$
$\frac{60 \text { Guests }}{2.40 \text { VOR }} \times 2$ Trips $(50 \%$ In; $50 \%$ Out $)=\mathbf{5 0}$
Assume $10 \%$ for Misc.Trips $=(12+8+50) \times 1.10=77.0 \approx 78(39$ Enter; 39 Exit $)$
Average Saturday Midday Peak Hour Trip Generation:
Note: Consists of the break switchover of Full-Time staff, the departure of the Part-Time staff, and the departure of the Guests.
3 Full - Time Employees $\times 2$ Trips ( $50 \%$ In; $50 \%$ Out) $=6$
4 Part - Time Employees $\times 1$ Trips ( $0 \%$ In; $100 \%$ Out) $=4$
$\frac{60 \text { Guests }}{2.40 \text { VOR }} \times 1$ Trips $(0 \%$ In; $100 \%$ Out $)=\mathbf{2 5}$
Assume $10 \%$ for Misc.Trips $=(6+4+25) \times 1.10=38.5 \approx 39$ ( 4 Enter; 35 Exit $)$

## Trip Generation Estimates - Visitation/Wake Services

## Average Weekday Daily Trip Generation:

Note: Consists of Full-Time staff, Part-Time staff, and two (2) concurrent Visitation/Wake Services, each consisting of 30 family/close personal guests and 240 visitation/wake guests.
8 Full - Time Employees $\times 4$ Trips (50\% In; 50\% Out) $=36$
4 Part - Time Employees $\times 2$ Trips ( $50 \%$ In; $50 \%$ Out) $=8$
$\frac{60 \text { Guests }}{2.40 \text { VOR }} \times 2$ Trips $(50 \%$ In; $50 \%$ Out $)=\mathbf{5 0}$
$\frac{168 \text { Guests }}{2.40 \text { VOR }} \times 2$ Trips $(50 \%$ In; $50 \%$ Out) $\approx \mathbf{1 4 0}$
Assume $10 \%$ for Misc.Trips $=(36+8+50+140) \times 1.10=257.4 \approx 258(129$ Enter $; 129$ Exit $)$

## Average Weekday Morning Peak Hour Trip Generation:

Note: Consists of the arrival of Full-Time staff only.
8 Full - Time Employees $\times 1$ Trip $(\sim 90 \%$ In; $\sim 10 \%$ Out $)=8$
Assume $10 \%$ for Misc.Trips $=8 \times 1.10=8.8 \approx 9$ ( 8 Enter $; 1$ Exit $)$

## Average Weekday Evening Peak Hour Trip Generation:

Note: Consists of the departure of Full-Time staff, the arrival of the Part-Time staff, the arrival of the family/close personal guests, the arrival and departure of the first wave of visitation/wake guests and the arrival of the second wave of visitation/wake guests; and the hosting of two concurrent visitation/wake services.
8 Full - Time Employees $\times 1$ Trip $(\sim 10 \%$ In; $\sim 90 \%$ Out $)=8$
4 Part - Time Employees $\times 1$ Trips (100\% In; 0\% Out) $=4$
60 Guests
$\frac{60 \text { Guests }}{2.40 \text { VOR }} \times 1$ Trips $(100 \%$ In; $0 \%$ Out $)=\mathbf{2 5}$
$\frac{1}{4} \times \frac{168 \text { Guests }}{2.40 \text { VOR }} \times 2$ Trips $(50 \%$ In; $50 \%$ Out $) \approx 36$
$\frac{1}{4} \times \frac{168 \text { Guests }}{2.40 \text { VOR }} \times 1$ Trip $(100 \%$ In; $0 \%$ Out $) \approx \mathbf{1 8}$
Assume $10 \%$ for Misc.Trips $=(8+4+25+41+20) \times 1.10=100.1 \approx \mathbf{1 0 0}(71$ Enter $; 29$ Exit $)$

## Average Saturday Trip Generation:

Note: Consists of Full-Time staff, Part-Time staff, and two (2) concurrent Visitation/Wake Services, each consisting of 30 family/close personal guests and 240 visitation/wake guests.
3 Full - Time Employees $\times 4$ Trips (50\% In; 50\% Out) $=\mathbf{1 2}$
4 Part - Time Employees $\times 2$ Trips (50\% In; 50\% Out) $=\mathbf{8}$
$\frac{60 \text { Guests }}{2.40 \text { VOR }} \times 2$ Trips $(50 \%$ In; $50 \%$ Out $)=\mathbf{5 0}$
$\frac{168 \text { Guests }}{2.40 \text { VOR }} \times 2$ Trips $(50 \%$ In; 50\% Out) $\approx \mathbf{1 4 0}$
Assume $10 \%$ for Misc.Trips $=(12+8+50+140) \times 1.10=231.0 \approx 232(116$ Enter; 116 Exit $)$

## Average Saturday Midday Peak Hour Trip Generation:

Note: Consists of the break switchover of Full-Time staff, the arrival of the Part-Time staff, the arrival of the family/close personal guests, the arrival of the first wave of visitation/wake guests; and the hosting of two concurrent visitation/wake services.
3 Full - Time Employees $\times 1$ Trip ( $50 \%$ In; $50 \%$ Out) $=6$
4 Part - Time Employees $\times 1$ Trips $(100 \%$ In; $0 \%$ Out $)=4$
$\frac{60 \text { Guests }}{2.40 \text { VOR }} \times 1$ Trips $(100 \%$ In; $0 \%$ Out $)=\mathbf{2 5}$
$\frac{1}{4} \times \frac{168 \text { Guests }}{2.40 \text { VOR }} \times 1$ Trip $(100 \%$ In; $0 \%$ Out $) \approx \mathbf{1 8}$
Assume $10 \%$ for Misc.Trips $=(6+4+25+18) \times 1.10=58.3 \approx 58(55$ Enter $; 3$ Exit $)$


[^0]:    ${ }^{1}$ For a funeral home use, the Zoning Ordinance requires that a minimum of one parking space be for each company vehicle (4 vehicles are envisioned), plus one parking space for each three seats in meeting rooms (approximately 120 seats are envisioned, resulting in 40 parking spaces).

[^1]:    ${ }^{2}$ The prevailing travel speed is also known as the $85^{\text {th }}$ percentile vehicle travel speed, or the speed at which 85 percent of the observed vehicles traveled at or below during the observation period.
    ${ }^{3}$ A minimum combined travel lane and paved shoulder width of 14 -feet is required to support bicycle travel in a shared traveled-way condition.
    ${ }^{4}$ Road Safety Audit, Main Street (Route 18) at Various Locations, Howard/Stein-Hudson Associates, June 2011.

[^2]:    $5^{5}$ A Policy on Geometric Design of Highway and Streets, $7^{\text {th }}$ Edition; American Association of State Highway and Transportation Officials (AASHTO); Washington D.C.; 2018.

[^3]:    ${ }^{6}$ Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, D.C.; 2009.

