



The Residences at 1500 Main

John M. Corcoran & Co. LLC



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ZONING SUMMARY

COMMERCIAL CORRIDOR OVERLAY DISTRICT - ROUTE 18 [CHAPTER 120, ARTICLE VIIIB]

DIMENSIONAL REQUIREMENT	AS-OF-RIGHT	PROPOSED CONDITION	ZONING RELIEF REQUIRED	MBTA STATION PROXIMITY EXCEPTIONS
FAR	1.25	1.07	No	MAY INCREASE BASE FAR BY 0.25
MAX BUILDING COVERAGE	60%	<26%	No	
MIN. OPEN SPACE	15%	>28%	No	
MAX IMPERVIOUS SURFACE	75%	<72%	No	
HEIGHT - MAXIMUM STORIES	5	5	No	
HEIGHT - MAXIMUM DIMENSION	70'	<65'	No	
HEIGHT - MINIMUM STORIES	3	5	No	
HEIGHT - MINIMUM DIMENSION	45'	>50'	No	
SETBACK - FRONT MINIMUM	25'	31'	No	REDUCTION
SETBACK - SIDES	10'	15' / 10'	No	
SETBACK - REAR	15'	81'	No	
SETBACK - FRONT MAXIMUM	70'	REF TO SHEET A0.02	No	
PARKING SPACES - RESIDENTIAL MINIMUM	259	362 *	No	15% OF DWELLING UNITS
PARKING SPACES - RESIDENTIAL MAXIMUM	400	362 *	No	
PARKING SPACES - COMMERCIAL MINIMUM	31	362 *	No	
PARKING SPACES - ELECTRIC VEHICLE CHARGING STATIONS MINIMUM	6	8	No	
PARKING SPACES - BICYCLES	36	>80	No	

*SEE DEVELOPMENT SUMMARY BELOW FOR BREAKDOWN OF PARKING SPACES

APPLICABLE CODES

ZONING: WEYMOUTH ZONING ORDINANCE
ACCESSIBILITY: MASSACHUSETTS ARCHITECTURAL ACCESS BOARD 521 CMR
THE AMERICANS WITH DISABILITIES ACT GUIDELINES (ADAAG)
DEPARTMENT OF HOUSING AND URBAN DEVELOPMENT FAIR HOUSING ACT
BUILDING CODES: MASSACHUSETTS STATE BUILDING CODE (780 CMR) - 9TH EDITION
ELECTRICAL: MASSACHUSETTS ELECTRICAL CODE 527 CMR, 12.00
ELEVATORS: MASSACHUSETTS ELEVATOR CODE 524 CMR
ENERGY: INTERNATIONAL ENERGY CONSERVATION CODE, 2015, AS ADOPTED AND AMENDED BY 780 CMR, 13 (IECC)
FIRE PROTECTION: MASSACHUSETTS FIRE PREVENTION REGULATIONS 527 CMR
MECHANICAL: INTERNATIONAL MECHANICAL CODE, 2015, AS ADOPTED AND AMENDED BY 780 CMR (IMC)
PLUMBING: MASSACHUSETTS FUEL GAS AND PLUMBING CODES, 248 CMR

DEVELOPMENT SUMMARY

USE & OCCUPANCY		OCCUPANCY CLASSIFICATION	
USE	DWELLING UNITS	R-2	A-2, A-3, B, M (TO BE DETERMINED)
COMMERCIAL			
PROJECT AREA			
SITE AREA	241,563 SF		
GROSS FLOOR AREA	257,674 SF		
BUILDING A	50,864 SF		
BUILDING B	99,773 SF		
BUILDING C	107,037 SF		
DWELLING UNIT MIX		PARKING	
STUDIO	36 UNITS	SURFACE LOTS	143 SPACES
1-BR	113 UNITS	ENCLOSED GARAGES	139 SPACES
2-BR	81 UNITS	PARALLEL STREET	7 SPACES
3-BR	7 UNITS	SHARED	73 SPACES
TOTAL	237 UNITS	TOTAL	362 (289 EXCLUSIVE) SPACES
RETAIL / COMMERCIAL			
LEASABLE	6,121 SF [INCLUDED IN TOTAL GFA ABOVE]		

THE RESIDENCES AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190

JOHN M. CORCORAN &
CO. LLC

100 GRANDVIEW ROAD, SUITE 203
BRAINTREE, MA 02184
P 781.849.0011

utile

ARCHITECTURE + URBAN DESIGN

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CHA COMPANIES

141 LONGWATER DR.
NORWELL, MA 02061
P 781 792.2277

BLW ENGINEERS

311 GREAT ROAD
LITTLETON, MA 01460
P 978 486.4301

VEITAS AND VEITAS

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BRAINTREE, MA 02184
P 781 849.2065

VANASSE & ASSOCIATES

35 NEW ENGLAND BUS CENTER DR.
SUITE 140
ANDOVER, MA 01810
P 978 474.8800

McPHAIL ASSOCIATES

2269 MASSACHUSETTS AVE.
CAMBRIDGE, MA 02140
P 617 868.1420



DATE: NOVEMBER 5, 2018
REVISION: B2A SUBMISSION
NOVEMBER 30, 2018 NOTICE OF INTENT

REVISIONS ON SHEET

SCALE: UTILE PROJECT NUMBER 1839

COVER SHEET

G0.00

SURVEY NOTES:

- THE EXISTING CONDITIONS INFORMATION SHOWN HEREON IS THE RESULT OF AN ON-THE-GROUND SURVEY PERFORMED BY CHA CONSULTING, INC. IN JULY OF 2018.
- ALL DEED REFERENCES ARE TO NORFOLK COUNTY REGISTRY OF DEEDS UNLESS OTHERWISE NOTED.
- LOCUS OWNER OF RECORD:
GMS REALTY, LLC
DEED BOOK 1837 PAGE 86
PARCEL ID 61427-25
- TOPOGRAPHY, CONTOURS AND BENCHMARKS ARE BASED ON THE TOWN OF WEYMOUTH VERTICAL DATUM. TEMPORARY BENCHMARKS, REFERENCED TO THE DATUM ARE INDICATED ON THE SURVEY. IN THE EVENT THAT BENCHMARKS (BMS), ESTABLISHED FOR THIS PROJECT AND PUBLISHED ON THIS SURVEY ARE DESTROYED, NOT RECOVERABLE OR A DISCREPANCY IS FOUND, THE USER SHOULD NOTIFY THIS FIRM IN WRITING PRIOR TO COMMENCING OR CONTINUING ANY WORK.
- THE PROJECT AREA IS LOCATED IN FLOOD ZONES "A" & "X" AS SHOWN ON FLOOD INSURANCE RATE MAP FOR NORFOLK COUNTY, COMMUNITY PANEL NUMBER 2602102396, EFFECTIVE DATE JULY 17, 2012.
- LOCATION OF SUBSURFACE UTILITIES SHOWN HEREON ARE APPROXIMATE AND ADDITIONAL UTILITIES MAY EXIST THAT ARE NOT SHOWN ON THIS PLAN. LOCATIONS ARE COMPILED FROM UTILITY PLANS OF RECORD AND DIG-SAFE FIELD MARKINGS. RIM AND INVERT INFORMATION HAS BEEN COMPILED AND FIELD VERIFIED WHERE POSSIBLE. THIS INFORMATION IS NOT TO BE USED FOR CONSTRUCTION. PRIOR TO ANY CONSTRUCTION, CONTACT DIG-SAFE (1-800-344-7233) TO FIELD VERIFY LOCATION OF ALL UTILITIES.
- PLAN REFERENCES:
PLAN 59 OF 1984
PLAN 454 OF 1986
MAIN STREET 2016 ALTERATION LAYOUT #838
PLAN BOOK 166, PAGE 1121
PLAN 273 OF 1981
- WETLAND FLAGS SHOWN HEREON ARE BASED ON FIELD LOCATIONS BY CHA CONSULTING, INC. IN JULY OF 2018. WETLAND FLAGS WERE DELINEATED BY CHA CONSULTING, INC. WETLAND SCIENTIST IN JULY OF 2018.

GENERAL

- PROTECTIONS**
 - PROVIDE PROTECTION NECESSARY TO PREVENT DAMAGE TO EXISTING IMPROVEMENTS, TREES OR VEGETATION.
 - PROTECT IMPROVEMENTS ON ADJOINING PROPERTIES AND ON OWNER'S PROPERTY.
 - RESTORE DAMAGED IMPROVEMENTS TO ORIGINAL CONDITION AS ACCEPTABLE TO PARTIES HAVING JURISDICTION.
- CONDUCT OPERATIONS TO ENSURE MINIMUM INTERFERENCE WITH OPERATIONS, STREETS, WALKS, AND OTHER ADJACENT FACILITIES. DO NOT CLOSE OR OBSTRUCT STREETS, WALKS, OR OTHER OCCUPIED OR USED FACILITIES WITHOUT PERMISSION FROM AUTHORITIES HAVING JURISDICTION. STREETS AND ROADWAYS SHALL BE THOROUGHLY CLEANED AND/OR SWEEP ON A DAILY BASIS OR MORE FREQUENTLY AS REQUIRED BY THE GOVERNING AUTHORITY.
- UNLESS SPECIFIED OTHERWISE ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE MASSACHUSETTS DOT SPECIFICATIONS FOR HIGHWAYS AND BRIDGES AND/OR THE APPROPRIATE LOCAL AUTHORITIES.
- ALL SLOPES, UNLESS OTHERWISE SPECIFIED, SHALL BE LOADED AND SEEDED FOR STABILIZATION AS SOON AS POSSIBLE TO PREVENT EROSION TOWARD RESCUE AREAS AND BUFFERS, ABUTTING PROPERTIES, OR PUBLIC WAYS. EROSION CONTROL BLANKETS ARE REQUIRED FOR ALL 2H:1V SLOPES. SLOPES MAY NOT EXCEED 2H:1V.
- ANY DEVIATIONS, I.E. "FIELD CHANGES" FROM THE DESIGN PLAN(S) MUST BE APPROVED BY THE DESIGN ENGINEER IN WRITING. CONTRACTOR SHOULD BE AWARE THAT LOCAL AND STATE AUTHORITIES HAVE JURISDICTION AND APPROVALS OBTAINED FROM THE APPROPRIATE AUTHORITIES PRIOR TO THE IMPLEMENTATION OF THE "FIELD CHANGE". CHA, INC. ASSUMES NO LIABILITY OR RESPONSIBILITY FOR WORK ASSOCIATED WITH FIELD CHANGES COMPLETED WITHOUT REGARD TO THE "FIELD CHANGE" PROCEDURE.
- RELOCATION OF ANY UTILITIES SHALL BE PERFORMED IN ACCORDANCE WITH THE PROVISIONS OF THE APPROPRIATE UTILITY COMPANY AND/OR REGULATORY AGENCY.
- " * * DIG SAFE NOTE * * " IN ACCORDANCE WITH MGL, CH. 93, SEC. 40 INCLUDING AMENDMENTS, ALL CONTRACTORS SHALL NOTIFY UTILITY COMPANIES AND GOVERNMENT AGENCIES, IN WRITING, OF THE INTENT TO EXCAVATE, BLAST, DEMOLISH, BORE, OR PERFORM OTHER EARTH MOVING OPERATIONS NO LESS THAN 72 HOURS AND NO MORE THAN 30 DAYS PRIOR TO THE COMMENCEMENT OF SUCH WORK (EXCLUSIVE OF SATURDAYS, SUNDAYS, AND LEGAL HOLIDAYS) OR CALL "DIG SAFE" AT 1-888-DIG-SAFE.
- ADDITIONAL BENCHMARKS TO BE SET BY CONTRACTOR PRIOR TO CONSTRUCTION TO ENSURE QUALITY WORKMANSHIP.
- ANY STILLING AND/OR DETENTION BASINS SHOULD RECEIVE PERIODIC MAINTENANCE DURING CONSTRUCTION TO REMOVE DEPOSITED SILTS AND DEBRIS TO ENSURE PROPER DRAINAGE AND SETTLING OF PARTICULATE MATTER.
- ALL MANHOLE COVERS FOR CROSS-COUNTRY LOCATIONS OR IN PUBLIC GATHERING LOCATIONS SHALL BE FITTED WITH BOLT LOOKS OR EQUIVALENT.
- UNLESS OTHERWISE LABELED, ALL REINFORCED CONCRETE PIPE, RCP, SHALL BE CLASS III; ALL DUCTILE IRON PIPE SHALL BE CEMENT LINING CLASS S2; ALL PVC SEWER SHALL BE SDR 35; ALL HDPE PIPE TO BE N-12 WATER TIGHT JOINT OR APPROVED EQUAL.
- PERMITS: THE SITE IS SUBJECT TO A SPECIAL PERMIT WITH SITE PLAN REVIEW FROM THE WEYMOUTH ZONING BOARD OF APPEALS. IT IS ALSO SUBJECT TO A NPDES CONSTRUCTION GENERAL PERMIT TO BE ISSUED PRIOR TO COMMENCEMENT OF WORK. THE PROJECT HAS BEEN DESIGNED TO REMAIN ENTIRELY OUTSIDE OF ALL RESOURCE AREAS, RIPARIAN ZONES AND ASSOCIATED BUFFERS. CONTRACTOR SHALL LIMIT WORK AREA TO WITHIN THE "UNIT OF WORK" LINE IDENTIFIED ON THE PLANS. CONTRACTOR SHALL MAINTAIN COPIES OF ALL PERMITS ON SITE DURING CONSTRUCTION AND SHALL ADHERE TO ANY / ALL CONDITIONS IMPOSED BY THOSE PERMITS.

SITE WORK

1. CAUTION - NOTICE TO CONTRACTOR

THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON DESIGN PLANS AND LIMITED AS-BUILT INFORMATION. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANIES AT LEAST 72 HOURS (EXCLUSIVE OF SATURDAYS, SUNDAYS, AND LEGAL HOLIDAYS) PRIOR TO ANY EXCAVATION, DEMOLITION, BORING, OR OTHER EARTH MOVING OPERATIONS TO REQUEST EXACT FIELD LOCATIONS OF UTILITIES. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS AT NO ADDITIONAL EXPENSE TO OWNER.

2. FILL MATERIAL

- ENSURE THAT AREAS TO BE FILLED ARE FREE OF STANDING WATER, FROST, FROZEN MATERIAL, TRASH, AND DEBRIS PRIOR TO FILL PLACEMENT.
- PLACE APPROPRIATE FILL MATERIAL, AS DESIGNATED BY THE GEOTECHNICAL ENGINEER IN HORIZONTAL LAYERS NOT EXCEEDING EIGHT INCHES (8") IN LOOSE DEPTH AND COMPACT EACH LAYER AT OPTIMUM MOISTURE CONTENT TO THE GREATER OF:
 - ADJACENT UNDISTURBED SOIL OR
 - 90% OF THE MAXIMUM DRY DENSITY OF THE EMBANKMENT MATERIAL AS DETERMINED BY AASHTO STANDARD METHOD T99, METHOD C.

3. FINISH GRADING

- GRADE ALL AREAS WHERE FINISH GRADE ELEVATIONS ARE INDICATED ON DRAWINGS, OTHER THAN PAVED AREAS AND BUILDINGS, INCLUDING EXCAVATED AREAS, FILLED AND TRANSITION AREAS, AND LANDSCAPED AREAS. GRADED AREAS SHALL BE UNIFORM AND SMOOTH, FREE FROM DEBRIS, OR IRREGULAR SURFACE CHANGES. FINISHED SUBGRADE SURFACE SHALL NOT BE MORE THAN 0.10 FEET ABOVE OR BELOW ESTABLISHED SUBGRADE ELEVATIONS, AND ALL GROUND SURFACES SHALL VARY UNIFORMLY FROM INDICATED ELEVATIONS. FINISH DITCHES SHALL BE GRADED TO ALLOW FOR PROPER DRAINAGE WITHOUT PONDING AND IN A MANNER THAT WILL MINIMIZE EROSION POTENTIAL.
- GRADE SURFACE TO MATCH ADJACENT GRADES AND TO PROVIDE FLOW TO SURFACE DRAINAGE STRUCTURES, OR GRADE AS DESIGNATED ON THE PLANS AFTER FILL PLACEMENT AND COMPACTION.
- THE CONTRACTOR IS RESPONSIBLE FOR GENERAL CLEANUP OF THE PROJECT ON A DAILY BASIS AND AT THE COMPLETION OF THE PROJECT. OPEN TRENCHES, DITCHES, EXCAVATIONS, ETC. SHALL NOT BE PERMITTED TO BE LEFT OPEN OVERNIGHT. CONTRACTOR WILL BACKFILL OR UTILIZE SUITABLE STEEL PILES FOR THE SECURING OF THE PROJECT SITE PRIOR TO CEASING WORK EACH DAY.
- APPROPRIATE TRAFFIC CONTROL, I.E. SIGNAGE, BARRICADES, AND OTHER MEANS, WILL BE SUPPLIED BY THE CONTRACTOR IN ACCORDANCE WITH ALL FEDERAL, STATE AND LOCAL AGENCIES.
- UNDER NO CIRCUMSTANCES WILL ANY UTILITY, STRUCTURE, AND/OR REPAIR BE BACKFILLED UNLESS INSPECTED AND APPROVED BY THE TOWN OFFICIALS AND/OR REPRESENTATIVE. RECEIPT OF APPROVAL TO BACKFILL WILL NOT RELEASE THE CONTRACTOR FROM ANY RESPONSIBILITY OR LIABILITY FOR PERFORMANCE TESTS REQUIRED AS PART OF THIS PROJECT.
- PROPER SHORING AND TRENCH BOXES SHALL BE UTILIZED AS REQUIRED BY LOCAL, STATE, AND FEDERAL REGULATORY AGENCIES TO PROVIDE A SAFE WORKING ENVIRONMENT. SHORING SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF MASSACHUSETTS WITH EXPERIENCE IN SHORING DESIGN.
- ALL UTILITIES DISTURBED DURING CONSTRUCTION SHALL BE REPAIRED BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.
- WATER**
 - ALL WATER PIPING, VALVES, HYDRANTS, AND FITTINGS ETC. TO CONFORM TO LOCAL GUIDELINES OR AS DIRECTED BY THE WATER DEPARTMENT. CONSTRUCTION OF WATER LINE TO CONFORM TO ALL LOCAL AND STATE AGENCIES HAVING JURISDICTION.
 - ALL WATER PIPE SHALL BE THICKNESS CLASS S2 DUCTILE IRON. ALL PIPES AND FITTINGS SHALL HAVE A CEMENT LINING TWICE THE THICKNESS SPECIFIED IN AWWA C104 AND SHALL HAVE A BITUMINOUS SEAL COAT APPLIES INSIDE AND OUTSIDE CONFORMING TO AWWA C104, "TYTON" OR MECHANICAL JOINTS ARE PERMITTED UNLESS OTHERWISE DIRECTED.
 - REFER TO PLUMBING PLANS FOR WATER SERVICE, FIRE PROTECTION, AND SANITARY SEWER CONNECTIONS UNLESS OTHERWISE NOTED.
- CONTRACTOR SHALL OBTAIN APPROVAL FOR ALL TRANSFORMER LOCATIONS FROM THE FIRE DEPARTMENT AND BUILDING INSPECTOR PRIOR TO CONSTRUCTION.

SEWER

ALL TESTING SHALL BE PER TOWN OF WEYMOUTH STANDARDS.

- THESE NOTES ARE INTENDED TO SUPPLEMENT THE LOCAL REQUIREMENTS FOR MATERIALS AND WORKMANSHIP.
- WATER AND SEWER MAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST LOCAL AND STATE CODES INCLUDING THE RECOMMENDATIONS OF THE AMERICAN WATER WORKS ASSOCIATION AND THE NEW ENGLAND INTERSTATE WATER POLLUTION CONTROL COMMISSION TECHNICAL REPORT 16. CONSTRUCTION SHALL PROCEED IN A WORKMANLIKE MANNER WITH STATE-OF-THE-ART CONSTRUCTION TECHNIQUES.
- THE CONTRACTOR SHALL INSULATE WATER AND SEWER MAINS AS INDICATED ON THE PLANS OR WHEN DESIGN OR CONSTRUCTION ENCUMBRANCES DICTATE ALIGNMENT TO OCCUR ABOVE THE Frost Line. PROCUREMENT AND INSTALLATION OF PIPE INSULATION SHALL CONFORM TO THE REQUIREMENTS LISTED IN THE LATEST MASS. DOT STANDARD SPECIFICATIONS FOR SECTION 301.60P AND MATERIAL SPECIFICATION MB.11.1. THE PIPE INSULATION SHALL BE PRE-MOLDED TYPE CELLULAR GLASS INSULATION WITH ALUMINUM JACKET CONFORMING TO 637M-622 OR APPROVED EQUAL.
- THE CONTRACTOR SHALL FOLLOW ALTERNATE CONSTRUCTION PROCEDURES WHEN DESIGN OR CONSTRUCTION ENCUMBRANCES PREVENT HORIZONTAL SEPARATION OF 10 FEET OR THE ALTERNATE OF 18 INCHES OF VERTICAL SEPARATION BETWEEN WATER AND SEWER MAINS. IN AREAS WHERE THE ABOVE OFFSETS CANNOT BE MAINTAINED, THE WATER MAIN SHALL BE CONSTRUCTED WITH MEGA-LUG MECHANICAL TYPE FITTINGS OR APPROVED EQUAL FOR A DISTANCE OF 10 FEET ON EITHER SIDE OF THE CROSSING OR LATERAL ENCROACHMENT AND SHALL STRADDLE A FULL LENGTH OF CLASS 52 CEMENTED LINED DUCTILE IRON WATER PIPE.
- THE DEFLECTION IN ALL GRAVITY SEWER PIPE SHALL BE TESTED USING A GO, NO-GO MANHOLE TEST TO ENSURE THAT PROPER INSTALLATION HAS OCCURRED. TEST SHALL CONFORM WITH PIPE MANUFACTURER'S RECOMMENDATIONS AND SHALL NOT INDICATE MORE THAN 7.5% DEFLECTION, 0.0 N.
- EACH SEGMENT OF THE SEWER MAIN INCLUDING MANHOLES SHALL BE LEAK TESTED AND OBSERVED BY A REPRESENTATIVE OF THE TOWN AND/OR ENGINEER IN ACCORDANCE WITH THE FOLLOWING PROCEDURES:

EXFILTRATION TEST FOR NEW SEWER MAIN

- PREPARATION OF TEST: AFTER THE MANHOLE HAD BEEN ASSEMBLED IN PLACE, ALL LIFTING HOLES AND THOSE EXTERIOR JOINTS WITHIN SIX FEET OF THE GROUND SURFACE SHALL BE FILLED AND POINTED WITH AN APPROVED NON-SHRINKING MORTAR. THE TEST SHALL BE MADE PRIOR TO PLACING THE SHELF AND INVERT AND BEFORE FILLING AND POINTING THE HORIZONTAL JOINTS BELOW THE 6'-FOOT DEPTH LINE. IF THE GROUNDWATER TABLE HAS BEEN ALLOWED TO RISE ABOVE THE BOTTOM OF THE MANHOLE, IT SHALL BE LOWERED FOR THE DURATION OF THE TEST. ALL PIPES AND OTHER OPENINGS INTO THE MANHOLE SHALL BE SUITABLE PLUGGED AND PLUGS BRACED TO PREVENT BLOW OUT.
- TEST PROCEDURE: THE MANHOLE SHALL THEN BE FILLED WITH WATER TO THE TOP OF THE CONE SECTION. IF THE EXCAVATION HAS NOT BEEN BACKFILLED AND OBSERVATION INDICATED NO VISIBLE LEAKAGE, THAT IS, NO WATER VISIBLY MOVING DOWN THE SURFACE OF THE MANHOLE, THE MANHOLE MAY BE CONSIDERED TO BE SATISFACTORILY WATERTIGHT. IF THE TEST AS DESCRIBED ABOVE IS UNSATISFACTORY AS DETERMINED BY THE ENGINEER OR IF THE MANHOLE EXCAVATION HAS BEEN BACKFILLED THE TEST SHALL BE CONTINUED A PERIOD OF TIME MAY BE PERMITTED, IF THE CONTRACTOR WISHES, TO ALLOW FOR ABSORPTION.
- AT THE END OF THIS PERIOD, THE MANHOLE SHALL BE REFILLED TO THE TOP OF THE CONE, IF NECESSARY, AND THE MEASURING TIME OF AT LEAST EIGHT HOURS BEGIN. AT THE END OF THE TEST PERIOD, THE MANHOLE SHALL BE REFILLED TO THE TOP OF THE CONE, MEASURING THE VOLUME OF WATER ADDED. THIS AMOUNT SHALL BE EXTRAPOLATED TO A 24-HOUR RATE AND THE LEAKAGE DETERMINED ON THE BASIS OF DEPTH. THE LEAKAGE FOR EACH MANHOLE SHALL NOT EXCEED ONE GALLON PER VERTICAL FOOT FOR A 24-HOUR PERIOD. IF THE TEST FAILS THIS REQUIREMENTS, BY THE LEAKAGE DOES NOT EXCEED THREE GALLONS PER VERTICAL FOOT PER DAY, REPAIRS BY APPROVED METHODS MAY BE MADE AS DIRECTED BY THE ENGINEER TO BRING THE LEAKAGE WITHIN THE ALLOWABLE RATE ON ONE GALLON PER VERTICAL FOOT PER DAY. LEAKAGE DUE TO A DEFECTIVE SECTION OR JOINT OF EXCEEDING THE THREE-GALLON PER VERTICAL FOOT PER DAY RATE, SHALL BE CAUSE FOR THE REJECTION OF THE MANHOLE. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO UNCOVER, DISASSEMBLE, RECONSTRUCT OR REPLACE THE MANHOLE AS DIRECTED BY THE ENGINEER. THE MANHOLE SHALL THEN BE RE-TESTED AND, IF SATISFACTORY, INTERIOR JOINTS SHALL BE FILLED AND POINTED.
- BACKFILLING: THE TEST MAY BE CONDUCTED EITHER BEFORE OR AFTER BACKFILLING AROUND THE MANHOLE. HOWEVER, IF THE CONTRACTOR ELECTS TO BACKFILL PRIOR TO TESTING, IT SHALL BE AT HIS OWN RISK AND IT SHALL BE INCIDENT UPON THE CONTRACTOR TO DETERMINE THE REASON FOR ANY FAILURE OF THE TEST. NO ADJUSTMENT IN THE LEAKAGE MEASUREMENTS WILL BE MADE FOR UNKNOWN CAUSES SUCH AS LEAKING PLUGS, ABSORPTION, ETC., I.E. IT WILL BE ASSUMED THAT ALL LOSS OF WATER DURING THE TEST IS A RESULT OF LEAKS THROUGH THE JOINTS OF THROUGH THE CONCRETE. FURTHERMORE, THE CONTRACTOR SHALL TAKE ANY STEPS NECESSARY TO ASSURE THE ENGINEER THAT THE WATER TABLE IS BELOW THE BOTTOM OF THE MANHOLE THROUGHOUT THE TEST.

VACUUM TEST FOR NEW SEWER MAIN

- THE VACUUM TESTING SYSTEM SHALL BE SUPPLIED BY NPC SYSTEMS, INC. OR EQUIVALENT AS APPROVED BY THE ENGINEER. THE TESTING SHALL BE DONE IMMEDIATELY AFTER ASSEMBLY OF THE MANHOLE AND BEFORE BACKFILLING. A 60-LB.-FT. TORQUE WRENCH SHALL BE USED TO TIGHTEN EXTERNAL CLAMPS THAT SECURE THE TEST COVER TO THE TOP OF THE MANHOLE. ALL LIFT HOLES SHALL BE PLUGGED WITH A NON-SHRINKING MORTAR. THE CONTRACTOR SHALL PLUG THE PIPE OPENINGS, TAKING CARE TO SECURELY BRACE THE PLUGS AND THE PIPE TO PREVENT THE FLUSH FROM DRAWN INTO THE MANHOLE.
- A VACUUM OF 10 INCHES OF MERCURY, HG (4.9 PSI), SHALL BE DRAWN AND THE VACUUM PUMP SHUT OFF. THE MANHOLE PASSES THE TEST IF THE VACUUM REMAINS GREATER THAN OR EQUAL TO 9 INCHES HG (4.4 PSI) FOR A PERIOD GREATER THAN ONE MINUTE FOR MANHOLES UP TO 10 FEET DEEP; ONE MINUTE FIFTEEN SECONDS FOR MANHOLES 10-15 FEET DEEP; AND ONE MINUTE THIRTY SECONDS FOR MANHOLES 15-25 FEET DEEP.
- IF THE MANHOLE FAILS THE INITIAL TEST, THE CONTRACTOR SHALL LOCATE THE LEAKS AND MAKE PROPER REPAIRS. LEAKS MAY BE FILLED WITH A WET SLURRY OF ACCEPTED QUICK SETTING MATERIAL. IF THE MANHOLE FAILS THE VACUUM TEST AGAIN, ADDITIONAL REPAIRS MUST BE MADE, AND THE MANHOLE MUST BE TESTED BY EXFILTRATION AS OUTLINED IN PARAGRAPH 3.03 (A) FIELD QUALITY CONTROL.

LEAKAGE TEST

- THE PIPELINES SHALL BE MADE AS NEARLY WATERTIGHT AS PRACTICABLE, AND LEAKAGE TESTS AND MEASUREMENTS SHALL BE MADE AFTER THE PIPELINE HAS BEEN BACKFILLED.
- WHERE THE GROUNDWATER LEVEL IS MORE THAN 1 FT. ABOVE THE TOP OF THE PIPE AT ITS UPPER END, THE CONTRACTOR SHALL CONDUCT EITHER INFILTRATION TESTS OR LOW PRESSURE AIR TESTS.
- WHERE THE GROUNDWATER LEVEL IS LESS THAN 1 FT. ABOVE THE TOP OF THE PIPE AT ITS UPPER END, THE CONTRACTOR SHALL CONDUCT EITHER EXFILTRATION TESTS OR LOW PRESSURE AIR TESTS.
- AT THE TIME OF THE TEST, THE CONTRACTOR SHALL DETERMINE THE GROUNDWATER ELEVATION FROM OBSERVATION WELLS, EXCAVATIONS OR OTHER MEANS, ALL SUBJECT TO REVIEW BY THE ENGINEER.
- FOR MAKING THE LOW PRESSURE AIR TESTS, THE CONTRACTOR SHALL USE EQUIPMENT SPECIFICALLY DESIGNED AND MANUFACTURED FOR THE PURPOSE OF TESTING SEWER PIPELINES USING LOW PRESSURE AIR. THE EQUIPMENT SHALL BE PROVIDED WITH AN AIR REGULATORY VALVE OR AIR SAFETY SO SET THAT THE INTERNAL AIR PRESSURE IN THE PIPELINE CANNOT EXCEED 8 PSIG.
- THE LEAKAGE TEST USING LOW PRESSURE AIR SHALL BE MADE ON EACH MANHOLE-TO-MANHOLE SECTION OF PIPELINE AFTER PLACEMENT OF THE BACKFILL.
- PNEUMATIC PLUGS SHALL HAVE A SEALING LENGTH EQUAL TO OR GREATER THAN THE DIAMETER OF THE PIPE TO BE TESTED. PNEUMATIC PLUGS SHALL RESIST INTERNAL TEST PRESSURES WITHOUT REQUIRING EXTERNAL BRACING OR BLOCKING.
- ALL AIR USED SHALL PASS THROUGH A SINGLE CONTROL PANEL.
- LOW PRESSURE AIR SHALL BE INTRODUCED INTO THE SEALED LINE UNTIL THE INTERNAL AIR PRESSURE REACHES 4 PSIG. GREATER THAN THE MAXIMUM PRESSURE EXERTED BY THE GROUNDWATER THAT MAY BE ABOVE THE INVERT OF THE PIPE AT THE TIME OF THE TEST. HOWEVER, THE INTERNAL AIR PRESSURE IN THE SEALED LINE SHALL NOT BE ALLOWED TO EXCEED 8 PSIG WHEN THE MAXIMUM PRESSURE EXERTED BY THE GROUNDWATER IS GREATER THAN 4 PSIG. THE CONTRACTOR SHALL CONDUCT ONLY AN INFILTRATION TEST.
- AT LEAST TWO MINUTES SHALL BE ALLOWED FOR THE AIR PRESSURE TO STABILIZE IN THE SECTION UNDER TEST. AFTER THE STABILIZATION PERIOD, THE LOW PRESSURE AIR SUPPLY HOSE SHALL BE QUICKLY DISCONNECTED FROM THE CONTROL PANEL. THE TIME REQUIRED IN MINUTES FOR THE PRESSURE IN THE SECTION UNDER TEST TO DECREASE FROM 2.5 TO 2.0 PSIG GREATER THAN THE MAXIMUM PRESSURE EXERTED BY GROUNDWATER THAT MAY BE ABOVE THE INVERT OF THE PIPE) SHALL NOT BE LESS THAN THAT SHOWN IN THE FOLLOWING TABLE:

PIPE DIAMETER IN INCHES VS. MINUTES

6"	5.0 MIN. 40 SEC.
8"	7.0 MIN. 34 SEC.
10"	9.0 MIN. 26 SEC.
12"	11.0 MIN. 20 SEC.
18"	14.0 MIN. 10 SEC.
18"	17.0 MIN. 0 SEC.
24"	19.0 MIN. 50 SEC.
24"	22.0 MIN. 40 SEC.
27"	25.0 MIN. 30 SEC.

- FOR MAKING THE INFILTRATION AND EXFILTRATION TESTS, THE CONTRACTOR SHALL FURNISH SUITABLE TEST PLUGS, WATER PUMPS, AND APPURTENANCES, AND ALL LABOR REQUIRED TO PROPERLY CONDUCT THE TESTS ON SECTIONS OF ACCEPTABLE LENGTH.
- FOR MAKING THE INFILTRATION TESTS, UNDERDRAINS, IF USED, SHALL BE PLUGGED AND OTHER GROUNDWATER DRAINAGE SHALL BE STOPPED TO PERMIT THE GROUNDWATER TO RETURN TO ITS NORMAL LEVEL INsofar AS PRACTICABLE.
- UPON COMPLETION OF A SECTION OF THE SEWER, THE CONTRACTOR SHALL DOWATER IT AND CONDUCT AN EXFILTRATION TEST TO MEASURE THE INFILTRATION FOR AT LEAST 24 HOURS. THE AMOUNT OF INFILTRATION, INCLUDING MANHOLES, TEES, AND CONNECTIONS, SHALL NOT EXCEED 200 GAL. PER INCH DIAMETER PER MILE OF SEWER PER 24 HOURS.
- FOR MAKING THE EXFILTRATION TESTS, THE SEWERS SHALL BE SUBJECTED TO AN INTERNAL PRESSURE BY PLUGGING THE PIPE AT THE LOWER END AND THEN FILLING THE PIPELINES AND MANHOLES WITH CLEAN WATER TO A HEIGHT OF 2 FT. ABOVE THE TOP OF THE SEWER AT ITS UPPER END, WHERE CONDITIONS BETWEEN MANHOLES, MAY RESULT IN TEST PRESSURES WHICH WOULD CAUSE LEAKAGE AT THE STOPPERS IN BRANCHES. PROVISIONS SHALL BE MADE BY SUITABLE TEES, BRACES, AND WEDGES TO SECURE THE STOPPERS AGAINST LEAKAGE RESULTING FROM THE TEST PRESSURE.
- THE RATE OF LEAKAGE FROM THE SEWERS SHALL BE DETERMINED BY MEASURING THE AMOUNT OF WATER REQUIRED TO MAINTAIN THE LEVEL 2 FT. ABOVE THE TOP OF THE PIPE.
- LEAKAGE FROM THE SEWERS UNDER TEST SHALL NOT EXCEED THE REQUIREMENTS FOR LEAKAGE INTO SEWERS AS HEREIN BEFORE SPECIFIED.
- THE SEWERS SHALL BE TESTED BEFORE ANY CONNECTIONS ARE MADE TO BUILDINGS.
- THE CONTRACTOR SHALL CONSTRUCT WEIRS OR OTHER MEANS OF MEASUREMENTS AS MAY BE REQUIRED.
- SUITABLE BULKHEADS SHALL BE INSTALLED, AS REQUIRED, TO PERMIT THE TEST OF THE SEWER.
- SHOULD THE SECTIONS UNDER TEST FAIL TO MEET THE REQUIREMENTS, THE CONTRACTOR SHALL DO ALL WORK OF LOCATING AND REPAIRING LEAKS AND RETESTING AS THE ENGINEER MAY REQUIRE WITHOUT ADDITIONAL COMPENSATION.
- IF, IN THE JUDGMENT OF THE ENGINEER, IT IS IMPRACTICABLE TO FOLLOW THE FOREGOING PROCEDURES FOR ANY REASON, ACCEPTABLE MODIFICATIONS IN THE PROCEDURES SHALL BE MADE AS REQUIRED, BUT IN ANY EVENT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ULTIMATE TIGHTNESS OF THE LINE WITHIN THE ABOVE TEST REQUIREMENTS.

WATER TESTING

ALL TESTING SHALL CONFORM TO TOWN OF WEYMOUTH STANDARD SPECIFICATIONS.

- REQUIRED TESTS FOR WATERLINES AND FORCE MAINS:
 - PERFORM THE FOLLOWING AFTER THE PIPE HAS BEEN INSTALLED AND PRIOR TO FINAL ACCEPTANCE:
 - PRESSURE TEST. / A2. LEAKAGE TEST.
- PRESUMPTIVE HYDROSTATIC TESTS MAY BE PERFORMED WHEN THE SYSTEM IS PARTIALLY BACKFILLED TO "CHECK" THE WORK, BUT FINAL ACCEPTANCE SHALL BE BASED ON HYDROSTATIC TESTS PERFORMED ON THE FINISHED SYSTEM AFTER IT IS COMPLETELY BACKFILLED.
- PRESSURE TEST:**
 - TEST PIPING TO 1.5 TIMES THE PIPE WORKING PRESSURE, OR 150 PSI, WHICHEVER IS GREATER. MEASURE TEST PRESSURES AT THE LOWEST POINT IN THE PIPE SECTION AND CORRECT TO THE ELEVATION OF THE GAUGE.
 - RELIEVE TRAPPED AIR AT THE SECTION HIGH POINTS THROUGH HYDRANTS, OR TAPS INSTALLED FOR THIS PURPOSE, PROVIDED TEMPORARY INSTALLATIONS ARE REMOVED AND PLUGGED AFTER ACCEPTANCE.
 - MAINTAIN THE TEST PRESSURE FOR A PERIOD OF TWO (2) HOURS. AT THE END OF THE TEST PERIOD, IF THE TEST PRESSURE REMAINS CONSTANT, THE PIPE SECTION SHALL HAVE PASSED THE TEST. IF THE PRESSURE HAS DROPPED, IT SHALL BE BROUGHT BACK TO THE TEST PRESSURE BY PUMPING A KNOWN VOLUME OF WATER (BY PUMPING FROM A GRADUATED CONTAINER OR BY METERING) BACK INTO THE PIPE. THE VOLUME OF WATER THUS USED, REPRESENTING LEAKAGE FROM THE PIPE, SHALL BE RECORDED. IF THE LEAKAGE IS LESS THAN THE ALLOWABLE LEAKAGE SPECIFIED BELOW, THE PIPE SHALL HAVE PASSED THE TEST. IF THE LEAKAGE EXCEEDS THE ALLOWABLE LEAKAGE SPECIFIED, THE CONTRACTOR SHALL LOCATE THE LEAK, PERMANENTLY REPAIR THE SECTION OF PIPE WHERE THE LEAK IS OCCURRING TO THE SATISFACTION OF THE ENGINEER, AND RETEST THE PIPE AS SPECIFIED ABOVE.
- LEAKAGE TEST:**
 - CONDUCT THE LEAKAGE TEST CONCURRENTLY WITH THE PRESSURE TEST.
 - THE MAXIMUM ALLOWED LEAKAGE IS DETERMINED BY THE FOLLOWING FORMULA:
$$L = \frac{N \times D \times P \times 1.48}{7000}$$
WHERE L = ALLOWABLE LEAKAGE, IN GPH ; WHERE N = NO. OF JOINTS IN TEST SECTION WHERE D = NOMINAL PIPE DIAMETER, IN INCHES ; WHERE P = AVERAGE TEST PRESSURE, IN PSIG
- ACCEPTANCE SHALL BE DETERMINED ON THE BASIS OF ALLOWABLE LEAKAGE. IF ANY PIPE SECTION DISCLOSES LEAKAGE GREATER THAN THAT SPECIFIED, LOCATE, REPAIR AND RETEST UNTIL THE LEAKAGE IS WITHIN THE LIMITS SPECIFIED.
- MAKE ALL VISIBLE LEAKS TIGHT REGARDLESS OF THE AMOUNT OF LEAKAGE, AND IF THE LINES DO NOT MEET THE ABOVE LEAKAGE TEST, REPAIR AND RETEST AS NECESSARY UNTIL THE LEAKAGE REQUIREMENT IS MET. REPAIR OR REPLACE ALL DEFECTIVE WORK.

DISINFECTION OF POTABLE WATER MAINS

- DISINFECT ALL POTABLE WATER MAINS IN ACCORDANCE WITH THE LATEST VERSION OF AWWA C651, EXCEPT THAT THE PLACEMENT OF CHLORINE POWDER OR TABLETS INSIDE THE PIPE DURING INSTALLATION SHALL NOT BE ALLOWED. DISINFECT WATER MAINS AFTER THE PIPING HAS PASSED THE PRESSURE AND LEAKAGE TESTING.
- FLUSH THE PIPE WITH WATER AT A MINIMUM VELOCITY OF 2.5 FEET PER SECOND (FPS) TO CLEAR ALL FOREIGN MATERIAL FROM THE PIPE.
- APPLY A CHLORINE SOLUTION WITH A CONCENTRATION BETWEEN 50 PARTS PER MILLION (PPM) AND 100 PPM. THE CHLORINE SOLUTION SHALL REMAIN IN THE PIPING FOR A MINIMUM OF 24 HOURS. THE CONCENTRATION AT THE END OF THIS PERIOD SHALL BE AT LEAST 25 PPM IN ALL SECTIONS OF THE MAIN. REPEAT THE ENTIRE PROCEDURE IF THE RESIDUAL IS LESS THAN 25 PPM.
 - WHILE THE CHLORINATED WATER IS BEING ADDED, ALL APPURTENANCES (VALVES, HYDRANTS, ETC.) SHALL BE OPERATED SO AS TO COMPLETELY DISINFECT THE NEW WORK.
 - POSITION VALVES SO THAT THE CHLORINE SOLUTION IN THE SECTION BEING DISINFECTED WILL NOT FLOW INTO WATER MAINS IN ACTIVE SERVICE.
 - CHLORINE RESIDUAL SAMPLES SHALL BE TAKEN AS DIRECTED BY THE ENGINEER.
- AFTER THE TWENTY FOUR (24) HOUR RETENTION PERIOD, FLUSH THE MAIN UNTIL RESIDUAL TESTING INDICATES THAT THE CHLORINE CONCENTRATION IS APPROXIMATELY THAT OF THE NEIGHBORING SERVICE AREA.
 - DISPOSE OF HEAVILY CHLORINATED WATER INTO SANITARY SEWER OR TANK TRUCK.
 - THE OWNER AND THE OWNER OF THE SANITARY SEWER SYSTEM SHALL BE NOTIFIED A MINIMUM OF TWENTY-FOUR (24) HOURS PRIOR TO THE DISCHARGE OF ANY WATER TO THE SANITARY SEWER. CONTRACTOR SHALL SUBMIT TO THE ENGINEER WRITTEN CONFIRMATION THAT THE OWNER OF SANITARY SEWER SYSTEM (THE TOWN), HAS APPROVED THE DISCHARGE OF WATER TO ITS SANITARY SEWER.
 - UNDER NO CIRCUMSTANCES WILL THE EMPTYING OF WATER ONTO ROADWAYS, OR INTO DITCHES, CULVERTS, STREAMS OR WETLANDS BE ALLOWED.
- AFTER DISINFECTION AND FINAL FLUSHING, AND PRIOR TO PLACING THE LINES IN SERVICE, THE CONTRACTOR SHALL COLLECT BACTERIOLOGICAL SAMPLES (BOTH COLIFORM AND HETEROTROPIC PLATE COUNT) AND SUBMIT SAMPLES TO AN APPROVED TESTING LABORATORY. TWO CONSECUTIVE SETS OF SAMPLES SHALL BE TAKEN AT LEAST 24 HOURS APART IN ACCORDANCE WITH AWWA C651. THE COLLECTION POINTS SHALL BE AS DIRECTED BY THE ENGINEER AND LOCAL AUTHORITY HAVING JURISDICTION.
 - THE TESTING LABORATORY PERFORMING THE BACTERIOLOGICAL ANALYSIS SHALL BE ACCEPTABLE TO THE ENGINEER.
 - SUBMIT THREE (3) COPIES OF THE LABORATORY ANALYSIS TO THE ENGINEER.
 - SHOULD SAFE RESULTS NOT OCCUR AFTER LABORATORY TESTS, THE CONTRACTOR SHALL, AT HIS EXPENSE, REPEAT THE DISINFECTION PROCEDURE UNTIL SAFE RESULTS ARE OBTAINED. THIS INCLUDES A POSITIVE RESULT FOR COLIFORM OR A MEASURED HETEROTROPIC PLATE COUNT OF GREATER THAN 500 COLONY FORMING UNITS PER ML.
 - CONTRACTOR SHALL PAY FOR ALL TESTING REQUIRED.
- ALL PRECAUTION SHALL BE TAKEN TO MAINTAIN DRY AND SANITARY CONDITIONS AND PREVENT CONTAMINATION OF ANY PIPING. IF, IN THE OPINION OF THE ENGINEER, CONTAMINATION HAS OCCURRED, THE CONTRACTOR SHALL REPEAT THE DISINFECTION PROCEDURE AND TESTING AT HIS COST AND EXPENSE.

LEGEND - EXISTING

	BUILDING LINE		ELECTRIC BOX
	PARCEL BOUNDARY LINE		WETLAND FLAG
	EASEMENT LINE		BOLLARD
	ADJOINING PARCEL LINE		SINGLE POST SIGN
	STREET/HIGHWAY LINE		DOUBLE POST SIGN
	EDGE OF ASPHALT		MONITORING WELL
	EDGE OF CONCRETE		POST
	EDGE OF GRAVEL/CRUSHED STONE		HANDICAP ICON
	WATER LINE		PINE TREE
	DRAIN LINE		DECIDUOUS TREE
	SEWER LINE		SHRUB
	GAS LINE		FLOOR DRAIN
	OVERHEAD UTILITY LINE W/POWER POLE		SQUARE CATCH BASIN
	TREE LINE		STORM MANHOLE
	TELEPHONE PEDESTAL		INVERT
	GAS VALVE		WATER SHUT OFF
	GAS METER		WATER VALVE
	SEWER MANHOLE		HYDRANT
	VENT		
	UTILITY POLE		
	LIGHT POLE		
	GUY WIRE		
	ELECTRIC MANHOLE		

LEGEND - PROPOSED

	MAJOR CONTOUR		CATCH BASIN
	MINOR CONTOUR		DOUBLE CATCH BASIN
	DOMESTIC WATER		DRAIN MANHOLE
	FIRE PROTECTION		HYDRANT ASSEMBLY
	UNDERGROUND ELECTRIC		LINE VALVE
	SANITARY SEWER		ELBOW WITH THRUST BLOCK
	FORCEMAIN		POST INDICATOR VALVE
	STORM SEWER		SANITARY MANHOLE
	BIORETENTION LIMIT LINE		ELECTRIC MANHOLE
	NATURAL GAS		SEWER GRINDER PUMP
	CHAINLINK FENCE		OIL/WATER SEPARATOR
	RETAINING WALL		LIGHT FIXTURE

THE RESIDENCES AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190

PROJECT

JOHN M. CORCORAN &
CO. LLC

100 GRANDVIEW ROAD, SUITE 203
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OWNER

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GEOTECH

KEY PLAN



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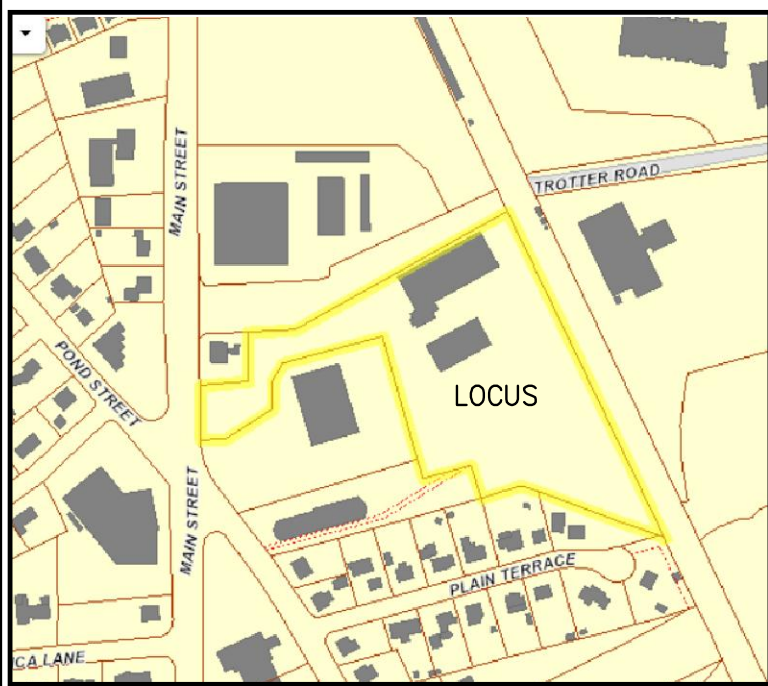
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NOVEMBER 5, 2018 BZA SUBMISSION

REVISIONS ON SHEET

SCALE UTILE PROJECT NUMBER
1839

NOTES-LEGEND

C-001



- GENERAL NOTES:**
1. THE EXISTING CONDITIONS INFORMATION SHOWN HEREON IS THE RESULT OF AN ON-THE-GROUND SURVEY PERFORMED BY CHA CONSULTING, INC. IN JULY OF 2018.
 2. ALL DEED REFERENCES ARE TO NORFOLK COUNTY REGISTRY OF DEEDS UNLESS OTHERWISE NOTED.
 3. LOCUS OWNER OF RECORD:
OWS REALTY, LLC
DEED BOOK 16371 PAGE 86
PARCEL ID 61-627-25
 4. TOPOGRAPHY, CONTOURS AND BENCHMARKS ARE BASED ON THE TOWN OF WEYMOUTH VERTICAL DATUM. TEMPORARY BENCHMARKS, REFERENCED TO THE DATUM ARE INDICATED ON THE SURVEY.

IN THE EVENT THAT BENCHMARKS (BM'S), ESTABLISHED FOR THIS PROJECT AND PUBLISHED ON THIS SURVEY ARE DESTROYED, NOT RECOVERABLE OR A DISCREPANCY IS FOUND, THE USER SHOULD NOTIFY THIS FIRM IN WRITING PRIOR TO COMMENCING OR CONTINUING ANY WORK.

5. THE PROJECT AREA IS LOCATED IN FLOOD ZONES "A" & "X" AS SHOWN ON FLOOD INSURANCE RATE MAP FOR NORFOLK COUNTY, COMMUNITY PANEL NUMBER 2502100239E, EFFECTIVE DATE: JULY 17, 2012.

6. THE LOCUS PARCEL IS LOCATED IN THE TOWN OF WEYMOUTH LIMITED BUSINESS DISTRICT (B-1) AS DEFINED BY THE TOWN OF WEYMOUTH ZONING MAP.

FRONT SETBACK: 30'
SIDE SETBACK: 10'
REAR SETBACK: 10'

7. LOCATION OF SUBSURFACE UTILITIES SHOWN HEREON ARE APPROXIMATE AND ADDITIONAL UTILITIES MAY EXIST THAT ARE NOT SHOWN ON THIS PLAN. LOCATIONS ARE COMPILED FROM UTILITY PLANS OF RECORD AND DIG-SAFE FIELD MARKINGS. RIM AND INVERT INFORMATION HAS BEEN COMPILED AND FIELD VERIFIED WHERE POSSIBLE. THIS INFORMATION IS NOT TO BE USED FOR CONSTRUCTION. PRIOR TO ANY CONSTRUCTION, CONTACT DIG-SAFE (1-800-344-7233) TO FIELD VERIFY LOCATION OF ALL UTILITIES.

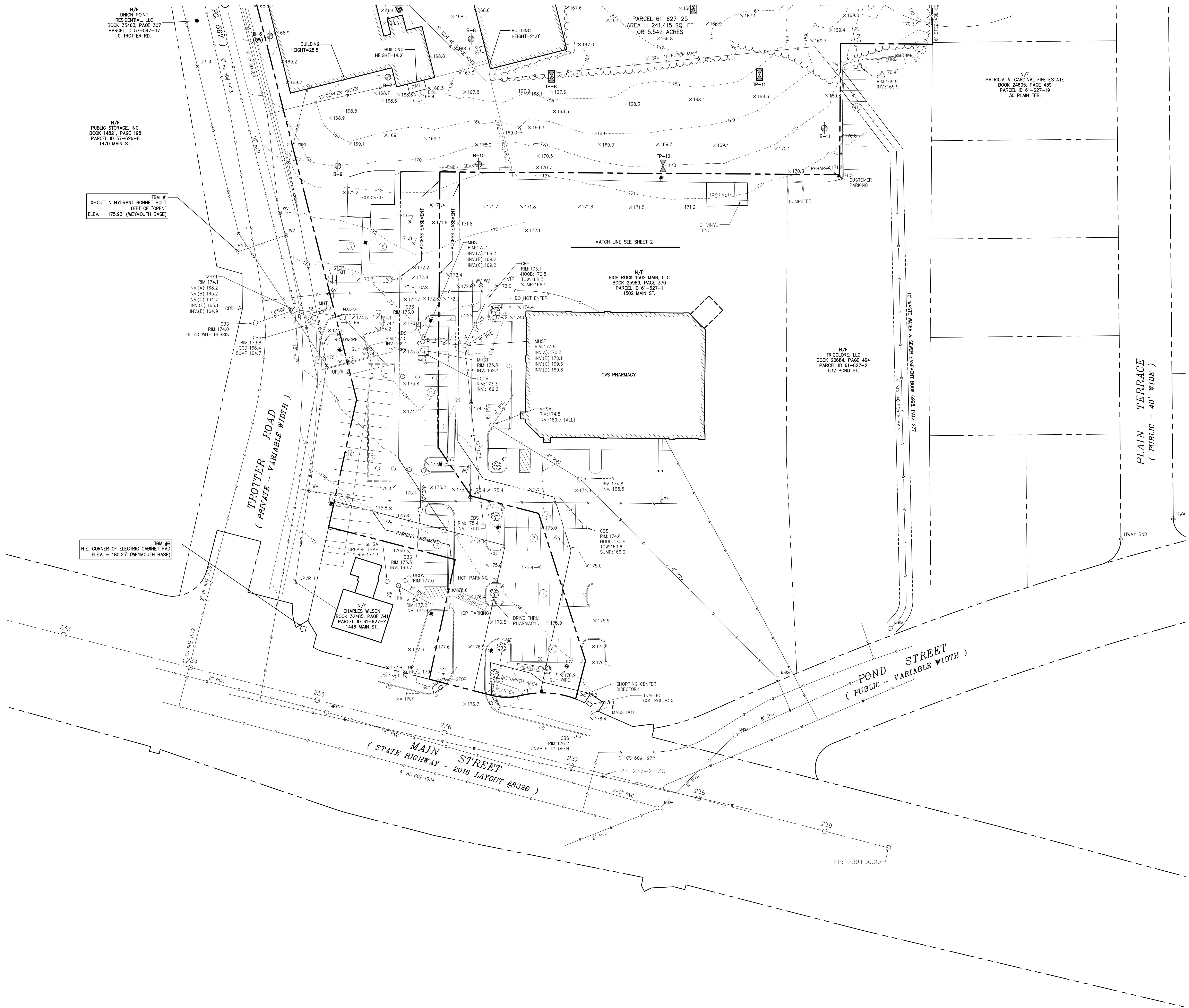
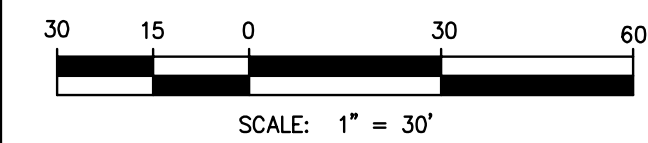
8. PLAN REFERENCES:
PLAN 59 OF 1984
PLAN 454 OF 1988
MAIN STREET 2016 ALTERATION LAYOUT #8326
PLAN BOOK 166, PAGE 1121
PLAN 273 OF 1964

9. WETLAND FLAGS SHOWN HEREON ARE BASED ON FIELD LOCATIONS BY CHA CONSULTING, INC. IN JULY OF 2018. WETLAND FLAGS WERE DELINEATED BY CHA CONSULTING, INC. SCIENTIST IN JULY OF 2018.

LEGEND:

- | | |
|--|-----------------------|
| | BUILDING LINE |
| | PARCEL BOUNDARY LINE |
| | EASEMENT LINE |
| | ADJOINING PARCEL LINE |
| | STREET/HIGHWAY LINE |
| | EDGE OF ASPHALT |
| | EDGE OF CONCRETE |
| | EDGE OF GRAVEL |
| | STONE WALL |
| | WIRE FENCE LINE |
| | GAS LINE |
| | WATER LINE |
| | SEWER LINE |
| | DRAIN LINE |
| | ELECTRIC LINE |
| | OVERHEAD UTILITY LINE |
| | TREE LINE |

- | | | | |
|--|--------------------|--|----------------------|
| | CONCRETE BOUND | | POST |
| | HIGHWAY BOUND | | SINGLE POST SIGN |
| | STONE BOUND | | DOUBLE POST SIGN |
| | REBAR | | PINE TREE |
| | GAS VALVE | | DECIDUOUS TREE |
| | GAS METER | | SHRUB |
| | SEWER MANHOLE | | SQUARE CATCH BASIN |
| | UTILITY POLE | | STORM MANHOLE |
| | LIGHT POLE | | INVERT |
| | GUY WIRE | | POST INDICATOR VALVE |
| | MANHOLE | | WATER VALVE |
| | ELECTRIC MANHOLE | | HYDRANT |
| | ELECTRIC HAND HOLE | | |
| | MONITORING WELL | | |



THE RESIDENCES AT 1500 MAIN

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KEY PLAN



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DATE: NOVEMBER 5, 2018
REVISION: BZA SUBMISSION

REVISIONS ON SHEET

SCALE: UTILE PROJECT NUMBER 1839

EXISTING CONDITION PLAN (SHEET 1 OF 2 SHEETS)

C-002

1500 MAIN STREET
WEYMOUTH, MA 02190

PROJECT

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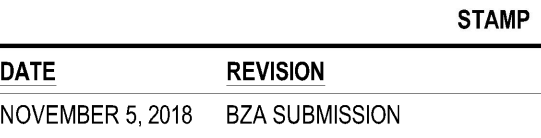
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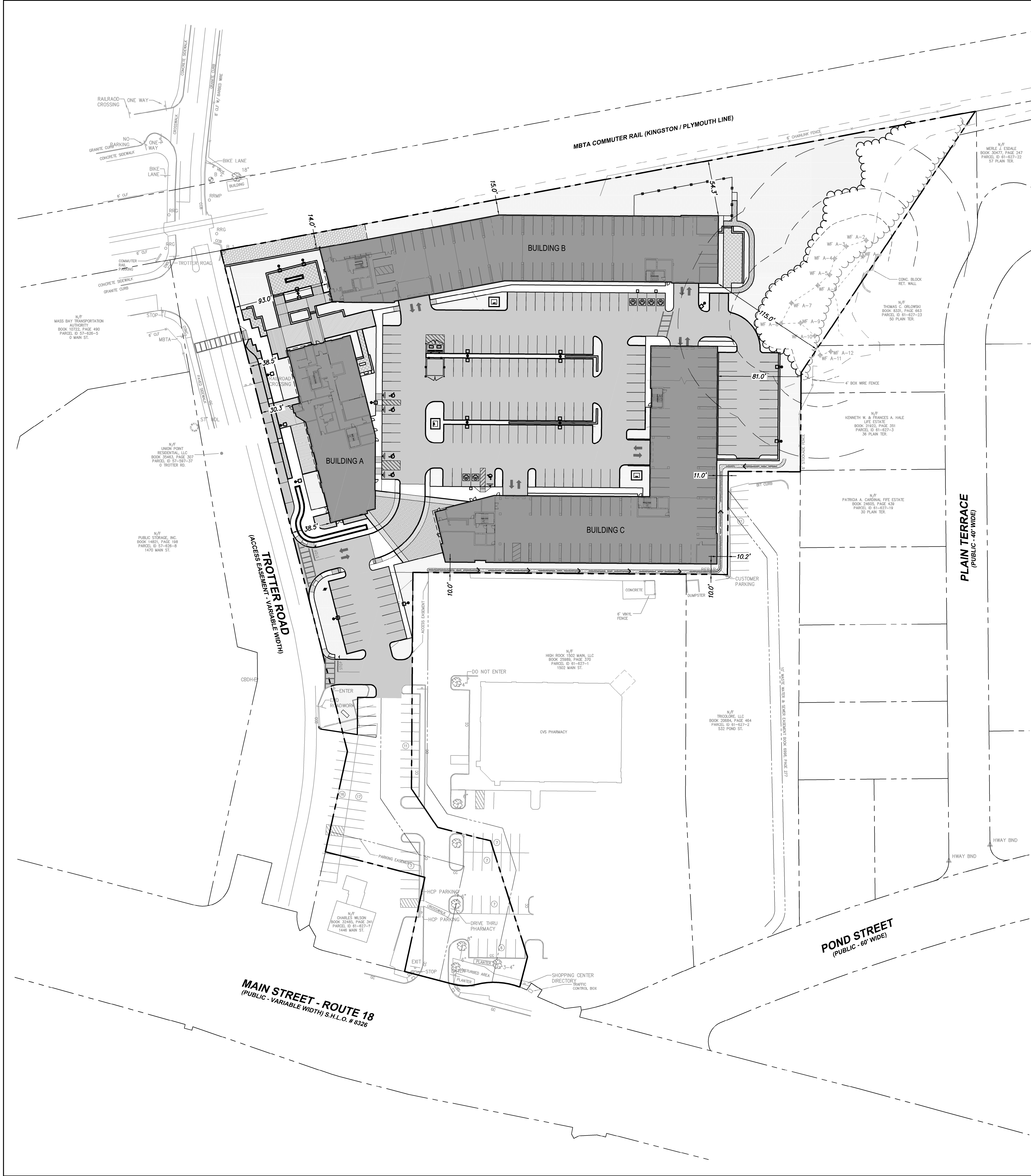
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DEMOLITION - EROSION

C-004



ZONING TABLE			
ZONING DISTRICT: (B-1) LIMITED BUSINESS , OVERLAY DISTRICT: (CCOD) COMMERCIAL CORRIDOR - ROUTE 18			
	B-1 REQUIRED	CCOD REQUIRED	PROVIDED/ PROPOSED
MINIMUM LOT SIZE	10,000 SF (See § 120-58)	30,000 SF	241,415 SF (5.542 ACRES)
MINIMUM LOT WIDTH	100 FT (See § 120-56 and 120-58)	N/A	50 FT (EXISTING NONCONFORMING)
FLOOR AREA RATIO (FAR)	NONE	1.25 (MAX) (WITHIN 1000' OF COMMUTING RAIL STATION)	FAR OF 1.09 (264,104± S.F. ÷ 241,415 S.F.)
LOT FRONTAGE	NONE	NONE	114± FT (Trotter Rd); 600± FT (Main St) ¹
FRONT YARD SETBACK	30 FT (MIN) (EXCLUDING SIGN)	MAX: 70 FT (AVERAGE) MIN: 25 FT (WITH 5' LANDSCAPE AREA)	70 FT. (AVERAGE) 30.0 FT. (MINIMUM)
SIDE YARD SETBACK	10 FT (MIN) (See § 120-59)	10 FT ²	10.0 FT
REAR YARD SETBACK	10 FT (MIN) (See § 120-59)	15 FT ²	81.1 FT
BUILDING COVERAGE	BUILDING & LOT: 50% (MAX)	60% (MAX)	26% (62,625 S.F. ÷ 241,415 S.F.)
LOT COVERAGE (IMPERVIOUS)	BUILDING & LOT: 50% (MAX)	75% (MAX-INCLUDES BUILDING)	71% (172,380 S.F. ÷ 241,415 S.F.) (EXCLUDES WETLAND)
OPEN SPACE	50% (MIN)	15% (MIN)	28% (66,360 S.F. ÷ 241,415 S.F.) (EXCLUDES WETLAND)
BUILDING HEIGHT	6 STORIES / 80 FEET (MAX)	3 STORIES / 45 FEET (MIN) 5 STORIES / 70 FEET (MAX)	5 STORIES w/ BASEMENT LEVEL PARKING: 58.3 FT

1. TROTTER ROAD IS BEING USED TO CALCULATE BUILDING SETBACKS ONLY.
2. A 20-FOOT "NO BUILD" AND "NO PARKING" BUFFER WILL BE PROVIDED ALONG ANY PROPERTY LINE ABUTTING AN EXISTING RESIDENTIAL USE PROPERTY. THE BUILDING IS ALSO LOCATED OUTSIDE OF THE "STEP-UP" BUFFER WHICH IS LOCATED WITHIN THE FIRST 25-FOOT AFTER THE ABOVE MENTIONED 20-FOOT BUFFER.

RESIDENTIAL PARKING REQUIREMENTS

(36) ----- STUDIO AT 1.0 SPACE/UNIT (WITHIN 1000 FT OF COMMUTER RAIL) = 36 SPACES
(113) ----- 1 BEDROOM AT 1.0 SPACES/UNIT (WITHIN 1000 FT OF COMMUTER RAIL) = 113 SPACES
(81) ----- 2 BEDROOM AT 1.25 SPACES/UNIT (WITHIN 100 FT OF COMMUTER RAIL) = 101 SPACES
(7) ----- 3 BEDROOM AT 1.25 SPACES/UNIT (WITHIN 100 FT OF COMMUTER RAIL) = 9 SPACES

TOTAL REQUIRED = 259 SPACES

COMMERCIAL PARKING REQUIREMENTS

COMMERCIAL SPACE (6,121 S.F.) AT 1 SPACE PER 200 SF = 31 SPACES

TOTAL REQUIRED = 31 SPACES

TOTAL PARKING SPACES

TOTAL REQUIRED = 290 SPACES (259 RESIDENTIAL + 31 COMMERCIAL)
TOTAL PROVIDED = 289 SPACES (259 RESIDENTIAL + 30 COMMERCIAL)
TOTAL PROVIDED (INCLUDING SHARED) = 362 (289 TOTAL + 73 SHARED)

*THE COMMERCIAL TOTAL EXCLUDES SHARED PARKING. A TOTAL OF 20 REBUILT SHARED PARKING SPACES ARE PROPOSED, AND AN ADDITIONAL 53 SHARED PARKING SPACES ARE EXISTING ADJACENT TO THE EXISTING CVS AND CAN BE USED BY RIGHT.

ELECTRIC CAR CHARGING STATION REQUIREMENTS

50-100 PARKING SPACES = 2 CHARGING SPACES
EVERY 50 ADDITIONAL SPACES = 1 CHARGING SPACE

TOTAL REQUIRED PARKING ON SITE = 290 SPACES (259 RESIDENTIAL + 31 RETAIL)
TOTAL REQUIRED CHARGING SPACES = 6 SPACES (2 + [190 ÷ 50])
TOTAL PROVIDED CHARGING SPACES = 6 SPACES

BICYCLE STALL REQUIREMENTS

TOTAL UNITS X 15% = 237 UNITS X 15% = 36 SPACES

TOTAL REQUIRED = 36 SPACES
TOTAL PROVIDED = 92+ (80+ INTERIOR SPACES + 12 EXTERIOR SPACES)

THE
RESIDENCES
AT 1500 MAIN

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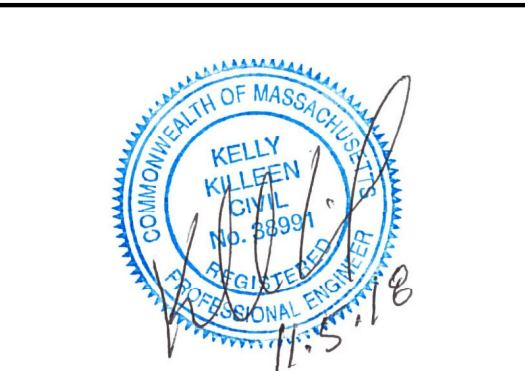
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KEY PLAN



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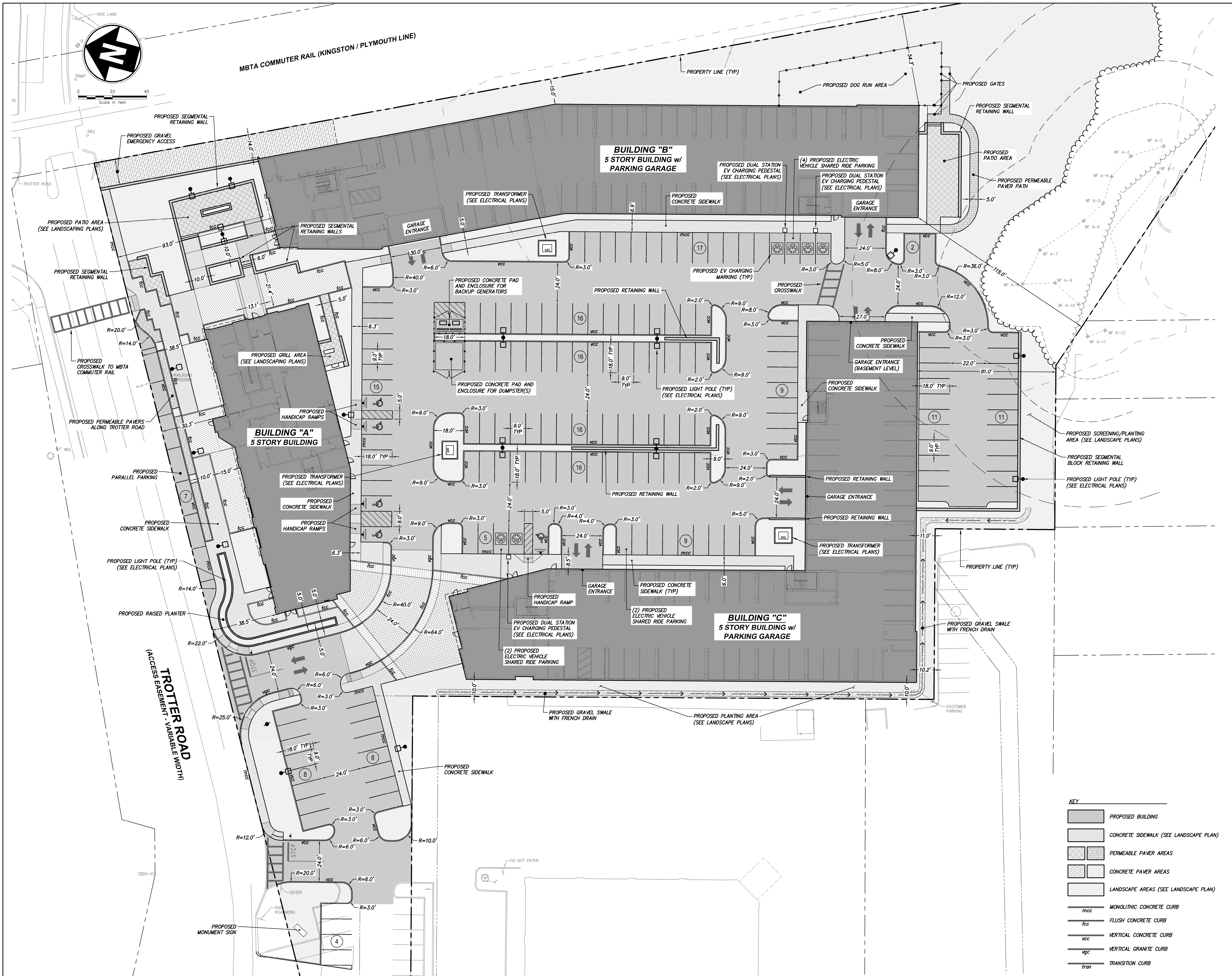
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SCALE UTILE PROJECT NUMBER
1839

OVERALL
SITE PLAN

C-101

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VEITAS AND VEITAS

639 GRANITE ST
LITTLETON, MA 02184
P 781.849.2065
STRUCTURAL

VANASSE & ASSOCIATES

35 NEW ENGLAND BUS CENTER DR.
SUITE 140
ANDOVER, MA 01810
P 978.474.8800
TRAFFIC

McPHAIL ASSOCIATES

2269 MASSACHUSETTS AVE.
CAMBRIDGE, MA 02140
P 617.868.1420
GEOTECH

KEY PLAN



DATE: NOVEMBER 5, 2016
REVISION: B2A SUBMISSION

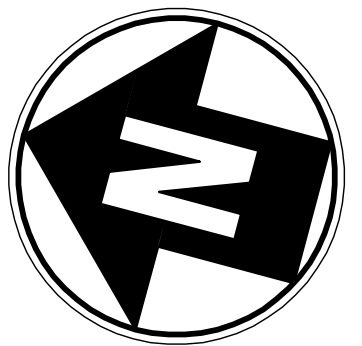
REVISIONS ON SHEET

SCALE: UTILE PROJECT NUMBER 1839

SITE PLAN

C-102

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0 20 40
Scale in feet

MBTA COMMUTER RAIL (KINGSTON / PLYMOUTH LINE)

BUILDING B

S=2.3%

BUILDING A

BUILDING C

BUILDING C

SUBGRADE PARKING

TROTTER ROAD
(ACCESS EASEMENT)
2" PL 60# 19/3

8" CI WATER

18" RCP

12" RCP

12" RCP

12" RCP

12" RCP

12" RCP

12" RCP

12" RCP

12" RCP

12" RCP

12" RCP

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12" RCP

12" RCP

12" RCP

12" RCP

12" RCP

THE RESIDENCES AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190
PROJECT
JOHN M. CORCORAN & CO. LLC
100 GRANDVIEW ROAD, SUITE 203
BRAINTREE, MA 02184
P 781 849.0011
OWNER

utile

ARCHITECTURE + URBAN DESIGN
115 KINGSTON ST
BOSTON, MA 02111
P 617 423.7200 F 617 423.1414
utiledesign.com
ARCHITECT

CHA COMPANIES

141 LONGWATER DRIVE, SUITE 104
NORWELL, MA 02061
P 781 982.5400
CIVIL & LANDSCAPE

BLW ENGINEERS

311 GREAT ROAD
LITTLETON, MA 01460
P 978 486.4301
MEPPF

VEITAS AND VEITAS

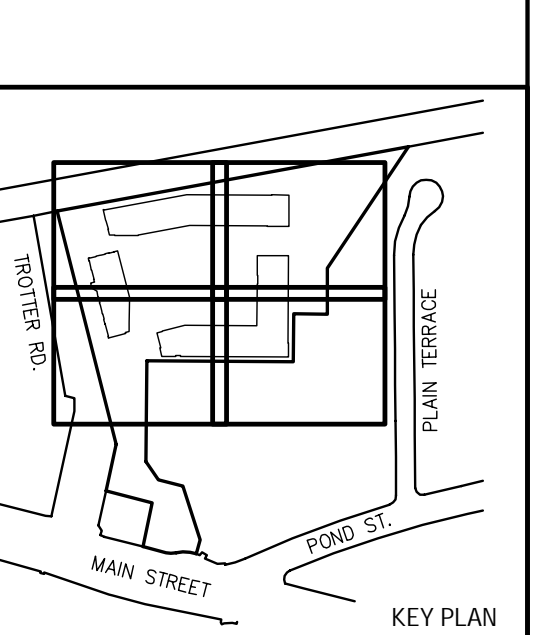
639 GRANITE ST
BRAINTREE, MA 01814
P 978 494.2065
STRUCTURAL

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DATE REVISION
NOVEMBER 5, 2018 BZA SUBMISSION

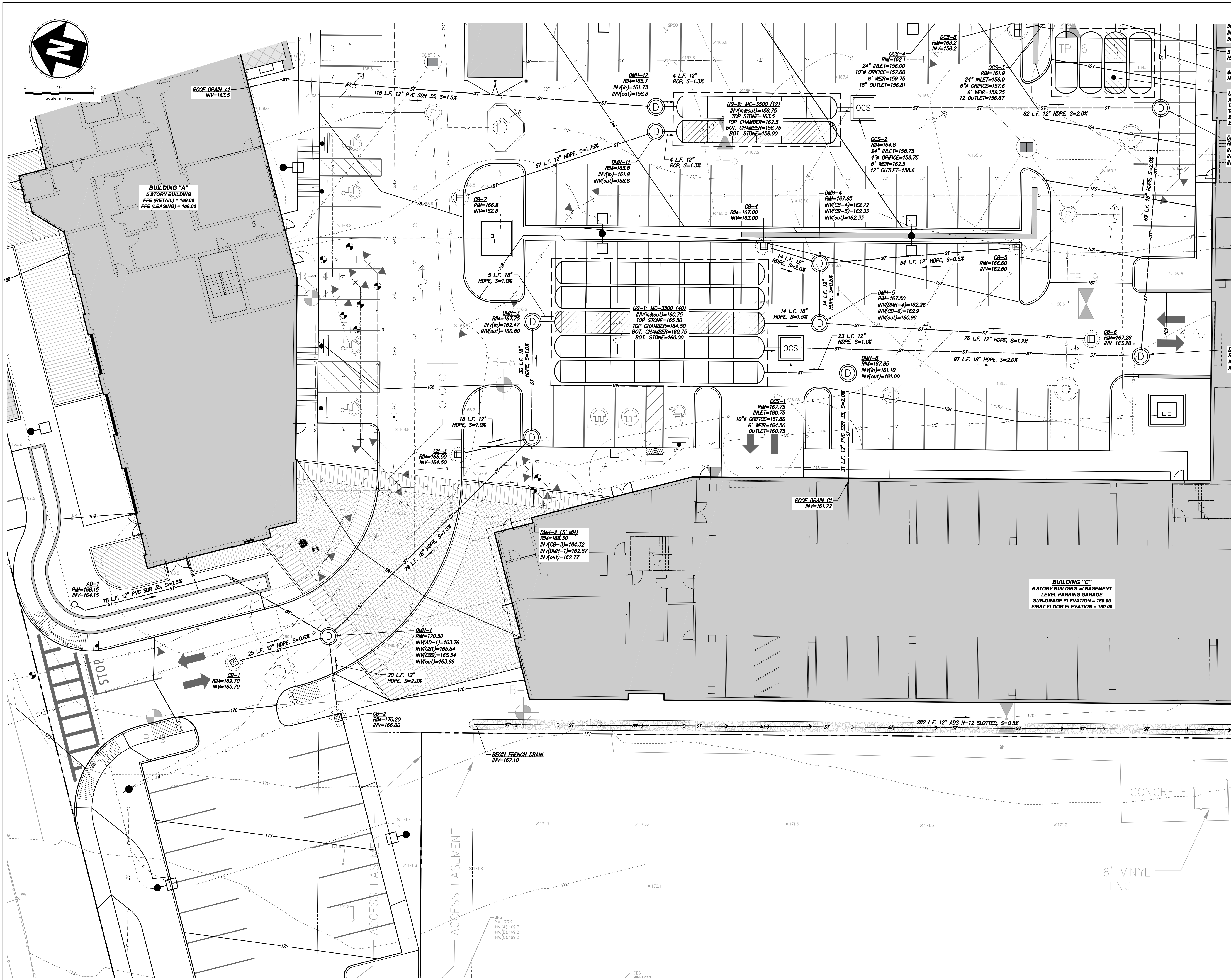
REVISIONS ON SHEET

SCALE UTILE PROJECT NUMBER
1839

GRADING & DRAINAGE PLAN
(SHEET 1 OF 5)

C-201

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THE
RESIDENCES
AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190

PROJECT

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CO. LLC

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P 781.849.0011

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STAMP

DATE

NOVEMBER 5, 2018

REVISION

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REVISIONS ON SHEET

SCALE

UTILITE PROJECT NUMBER

1839

GRADING &
DRAINAGE PLAN
(SHEET 2 OF 5)

C-202

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1500 MAIN STREET
WEYMOUTH, MA 02190

PROJECT

**JOHN M. CORCORAN &
CO. LLC**

100 GRANDVIEW ROAD, SUITE 203
BRAINTREE, MA 02184
P 781.849.0011



0 10 20
Scale in feet

THE RESIDENCES AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190
PROJECT
JOHN M. CORCORAN &
CO. LLC
100 GRANDVIEW ROAD, SUITE 203
BRAINTREE, MA 02184
P 781 849.0011
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CHA COMPANIES

141 LONGWATER DRIVE, SUITE 104
NORWELL, MA 02061
P 781 982.5400
CIVIL & LANDSCAPE

BLW ENGINEERS

311 GREAT ROAD
LITTLETON, MA 01460
P 978 486.4301
MEPPF

VEITAS AND VEITAS

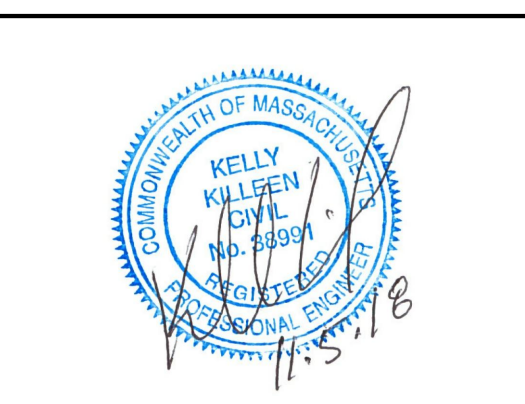
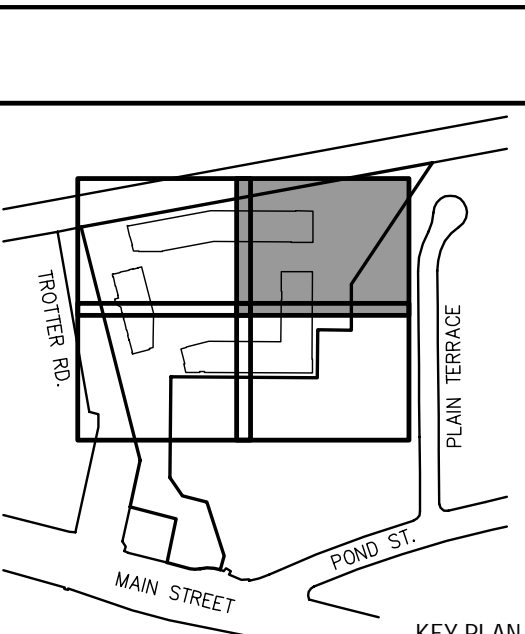
639 GRANITE ST
BRAINTREE, MA 02184
P 781 849.2065
STRUCTURAL

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35 NEW ENGLAND BUS CENTER DR.
SUITE 140
ANDOVER, MA 01810
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TRAFFIC

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CAMBRIDGE, MA 02140
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GEOTECH



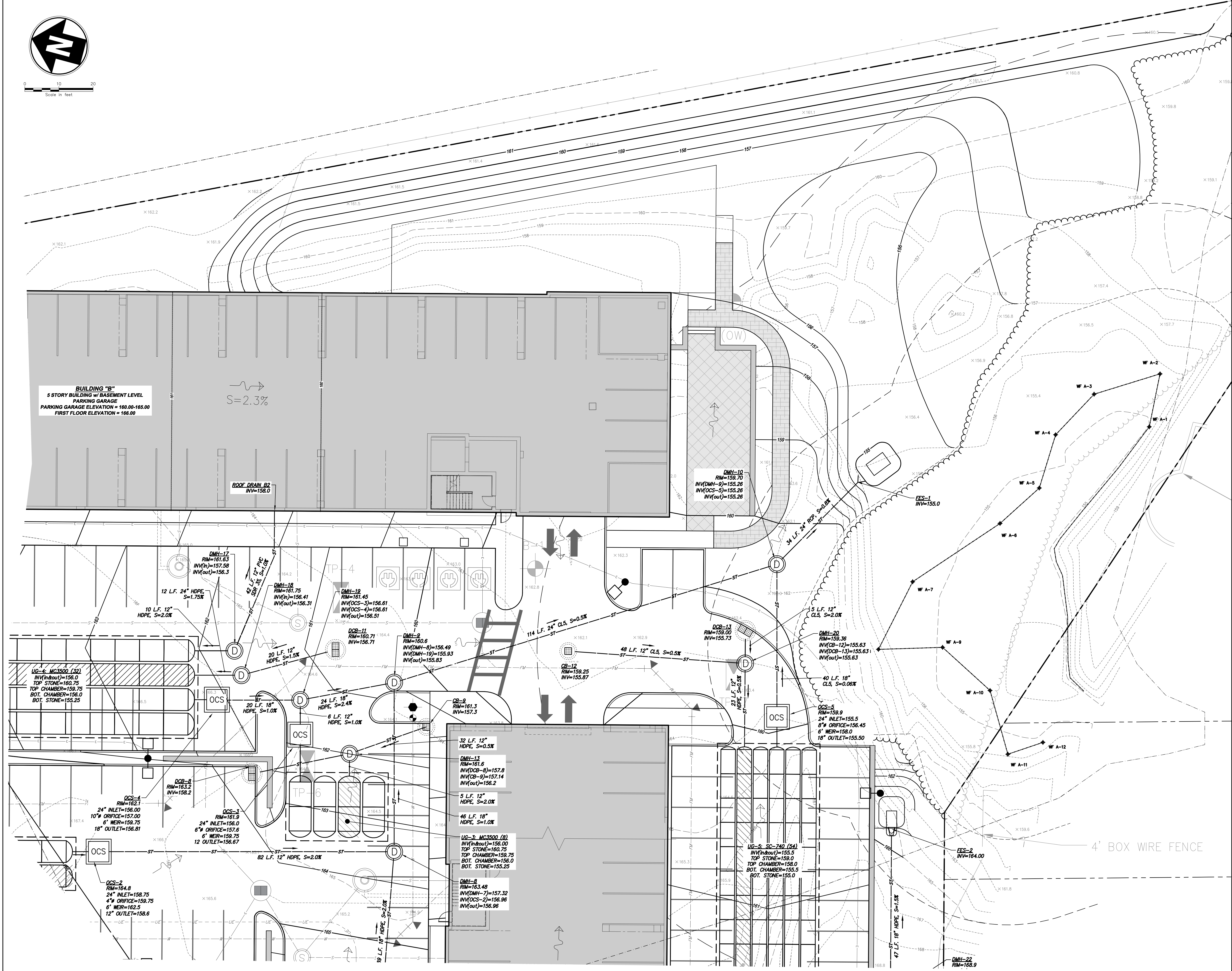
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NOVEMBER 5, 2018 BZA SUBMISSION

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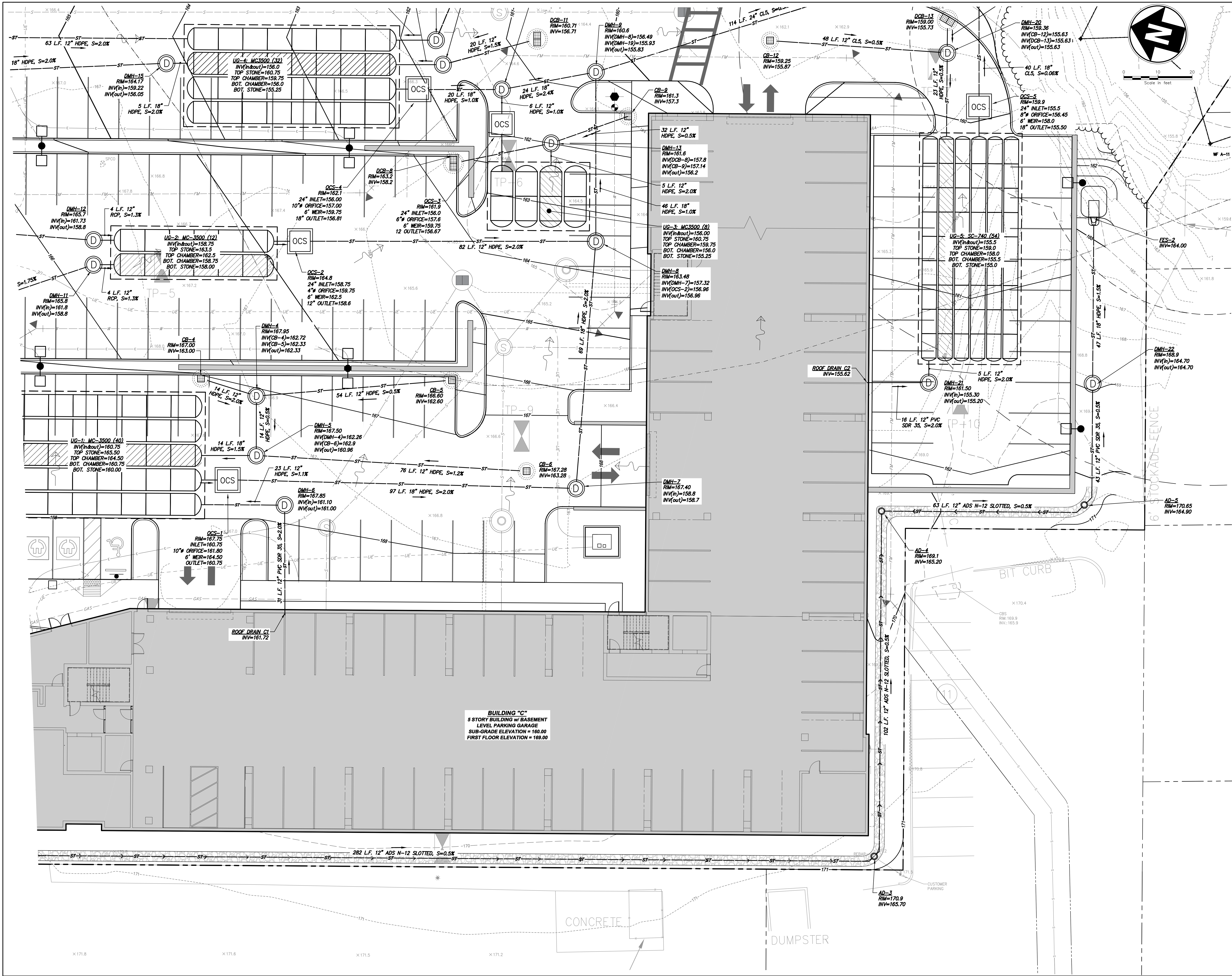
SCALE UTILE PROJECT NUMBER
1839

GRADING & DRAINAGE PLAN (SHEET 4 OF 5)

C-204



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THE RESIDENCES AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190

PROJECT

JOHN M. CORCORAN & CO. LLC

100 GRANDVIEW ROAD, SUITE 203
BRAINTREE, MA 02184
P 781 849 0011

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ARCHITECT

CHA COMPANIES

141 LONGWATER DRIVE, SUITE 104
NORWELL, MA 02061
P 781 982.5400

CIVIL & LANDSCAPE

BLW ENGINEERS

311 GREAT ROAD
LITTLETON, MA 01460
P 978 486.4301

MEPPF

VEITAS AND VEITAS

639 GRANITE ST
BRAINTREE, MA 02184
P 781 494.2065

STRUCTURAL

VANASSE & ASSOCIATES

35 NEW ENGLAND BUS CENTER DR.
SUITE 140
ANDOVER, MA 01810
P 978 474.8800

TRAFFIC

McPHAIL ASSOCIATES

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P 617 868.1420

GEOTECH

STAMP

DATE: NOVEMBER 5, 2018

REVISION: BZA SUBMISSION

REVISIONS ON SHEET

SCALE: 1"=20'

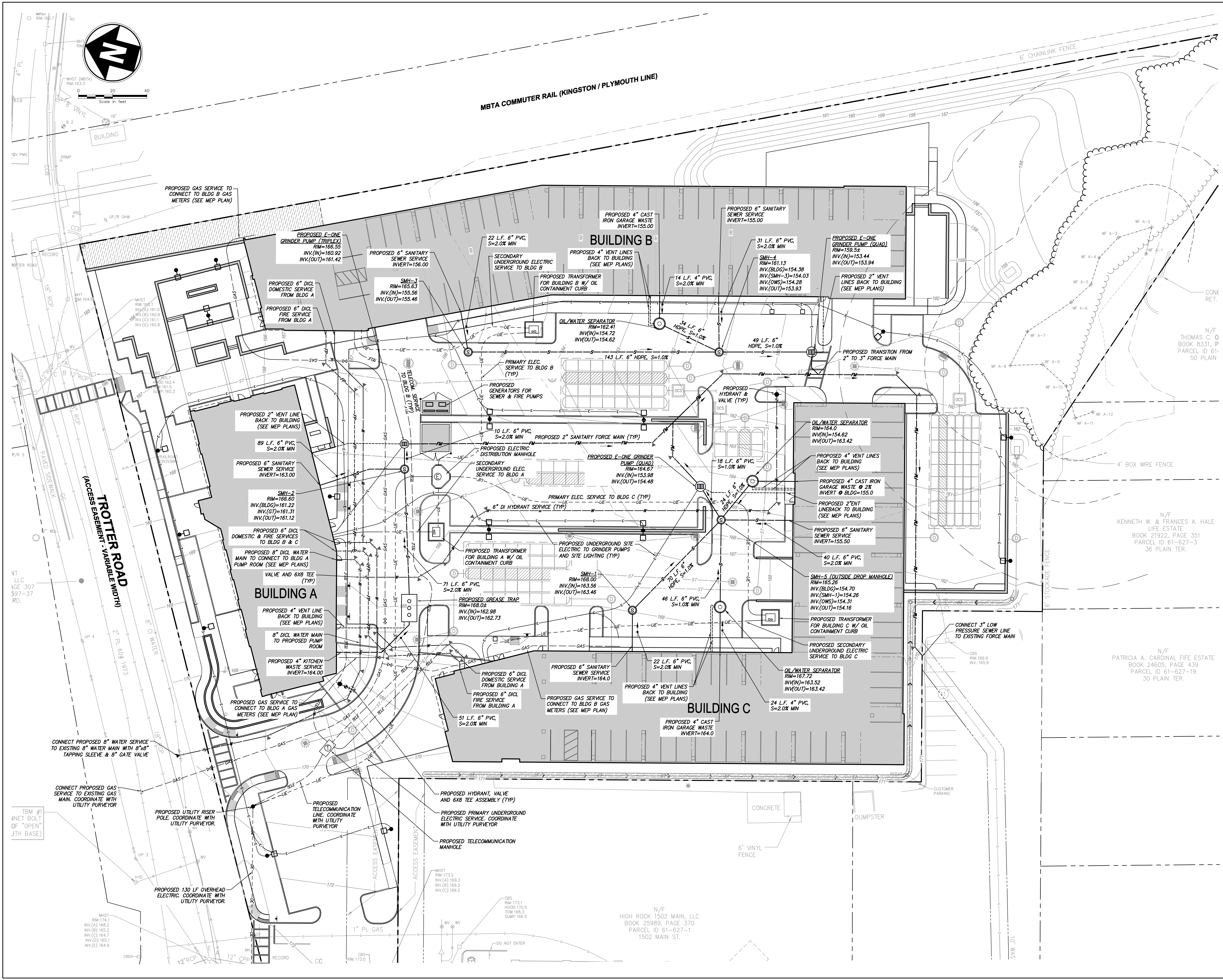
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GRADING & DRAINAGE PLAN (SHEET 5 OF 5)

C-205

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VT LLC
AGE 307
597-37
RD.



THE
RESIDENCES
AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190

PROJECT

JOHN M. CORCORAN &
CO. LLC

100 GRANDVIEW ROAD, SUITE 203
BRAINTREE, MA 02184
P 781.849.0011

OWNER

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ARCHITECT

CHA COMPANIES

141 LONGWATER DRIVE, SUITE 104
NORWELL, MA 02061
P 781.982.5400

CIVIL & LANDSCAPE

BLW ENGINEERS

311 GREAT ROAD
LITTLETON, MA 01460
P 978.486.4301

MEP/P

VEITAS AND VEITAS

639 GRANITE ST
BRAINTREE, MA 02184
P 781.849.2065

STRUCTURAL

VANASSE & ASSOCIATES

35 NEW ENGLAND BUS CENTER DR.
SUITE 140
ANDOVER, MA 01810
P 978.474.8800

TRAFFIC

McPHAIL ASSOCIATES

2269 MASSACHUSETTS AVE.
CAMBRIDGE, MA 02140
P 617.868.1420

GEOTECH

KEY PLAN

STATE OF MASSACHUSETTS

SEAL

KELLY
KILLEN
D.P.E.

REGISTERED PROFESSIONAL ENGINEER

NO. 15178

DATE

REVISION

NOVEMBER 5, 2018

BZA SUBMISSION

REVISIONS ON SHEET

SCALE

UTILE PROJECT NUMBER

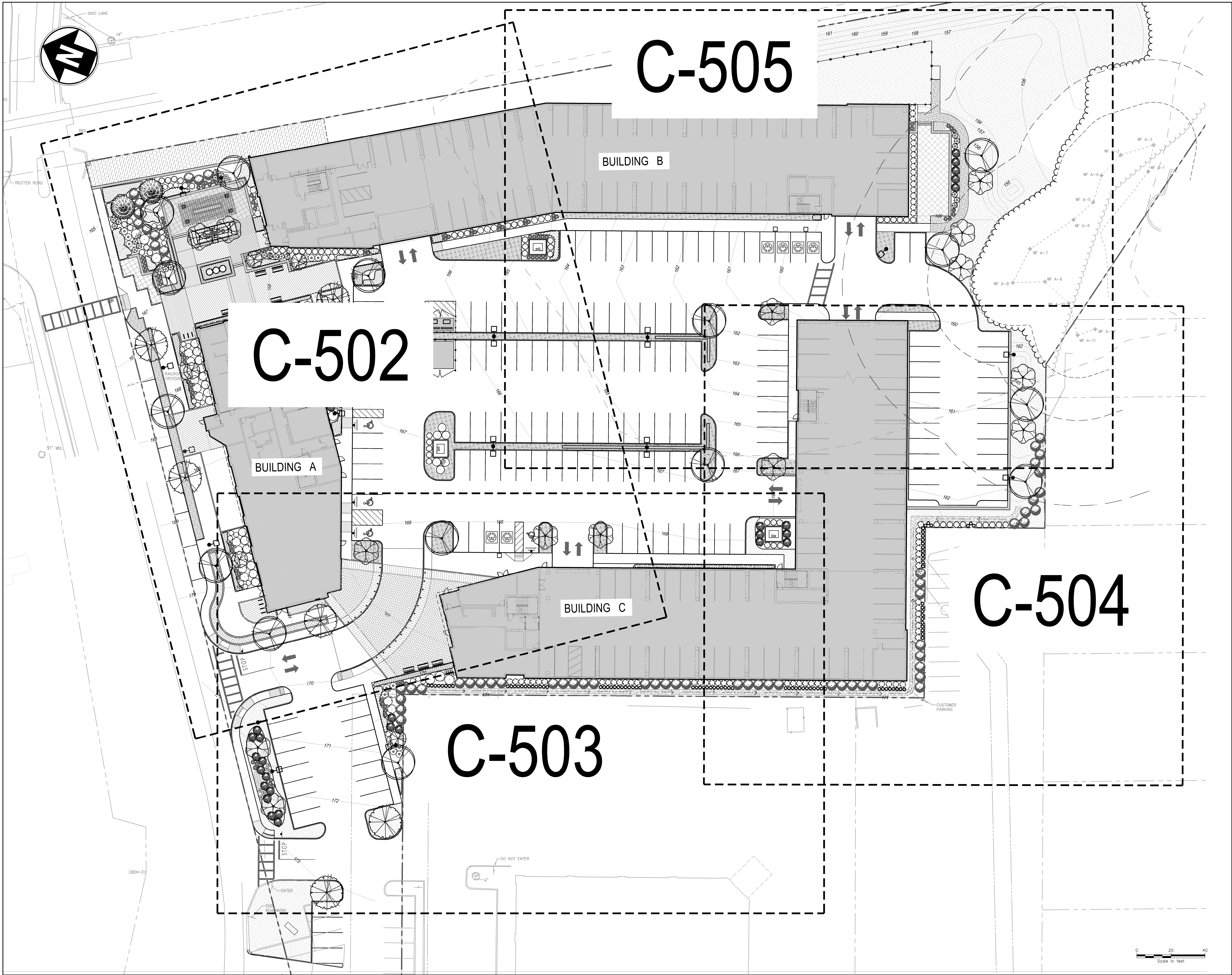
1839

UTILITY PLAN

C-301

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THE RESIDENCES AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190
PROJECT
JOHN M. CORCORAN & CO. LLC
100 GRANDVIEW ROAD, SUITE 203
BRAintree, MA 02184
P 781.849.0011
OWNER

utile

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P 617.423.7200 F 617.423.1414
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ARCHITECT

CHA COMPANIES

141 LONGWATER DRIVE, SUITE 104
NORWELL, MA 02061
P 781.982.5400
CIVIL & LANDSCAPE

BLW ENGINEERS

311 GREAT ROAD
LITTLETON, MA 01460
P 978.486.4301
MEPPF

VEITAS AND VEITAS

639 GRANITE ST
BRAintree, MA 02184
P 781.849.2065
STRUCTURAL

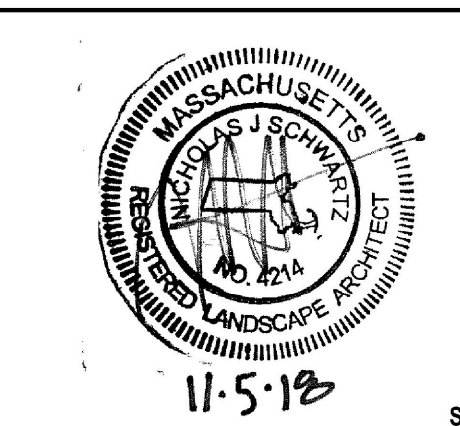
VANASSE & ASSOCIATES

35 NEW ENGLAND BUS CENTER DR.
SUITE 140
ANDOVER, MA 01810
P 978.474.8800
TRAFFIC

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2269 MASSACHUSETTS AVE.
CAMBRIDGE, MA 02140
P 617.868.1420
GEOTECH

KEY PLAN



DATE REVISION
NOVEMBER 5, 2018 BZA SUBMISSION

REVISIONS ON SHEET

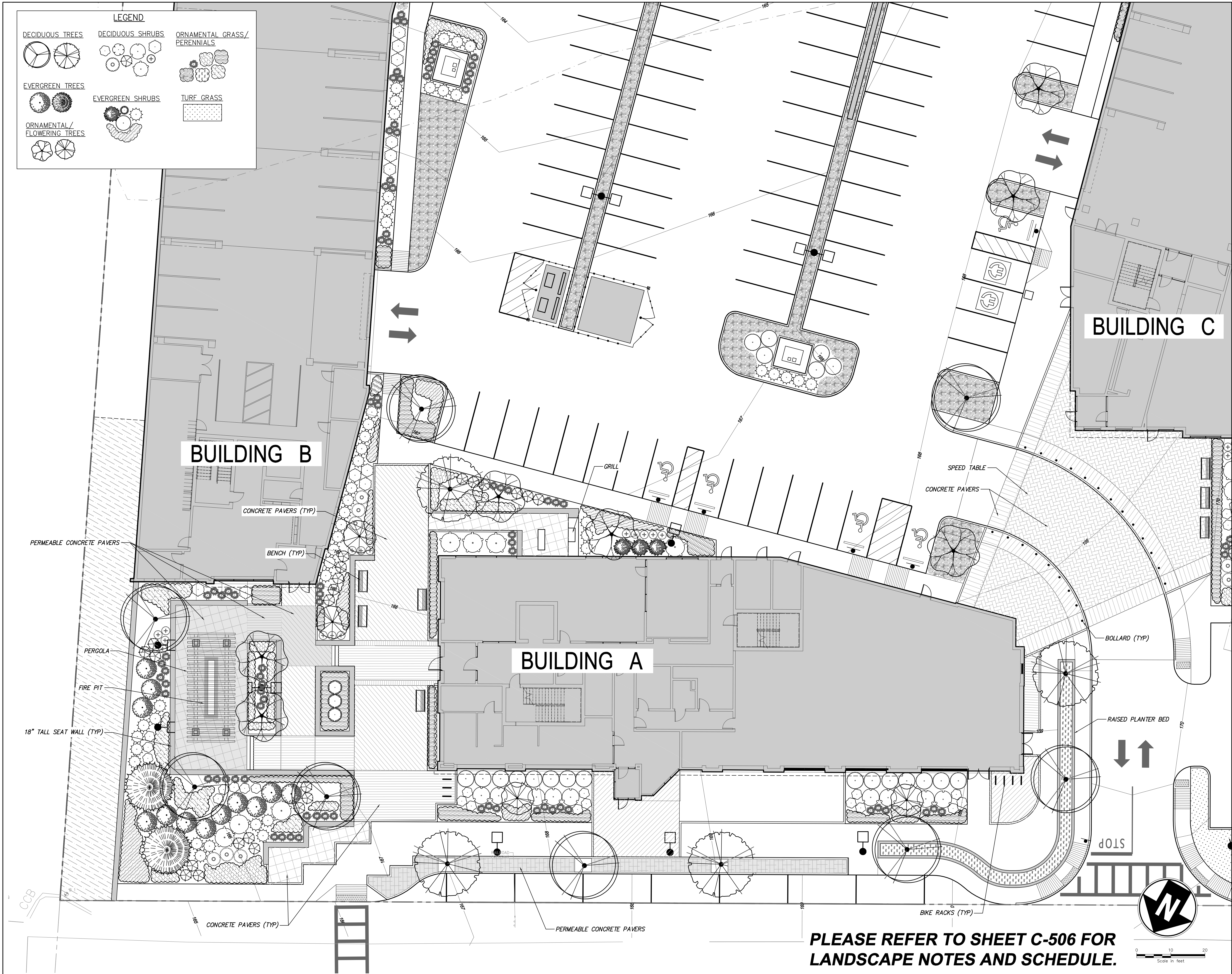
SCALE UTILE PROJECT NUMBER
1839

LANDSCAPE KEY PLAN

C-501

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THE RESIDENCES AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190
PROJECT
JOHN M. CORCORAN & CO. LLC
100 GRANDVIEW ROAD, SUITE 203
BRAINTREE, MA 02184
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NORWELL, MA 02061
P 781 982.5400
CIVIL & LANDSCAPE

BLW ENGINEERS

311 GREAT ROAD
LITTLETON, MA 01460
P 978 486.4301
MEPP

VEITAS AND VEITAS

639 GRANITE ST
BRAINTREE, MA 02184
P 781 849.2065
STRUCTURAL

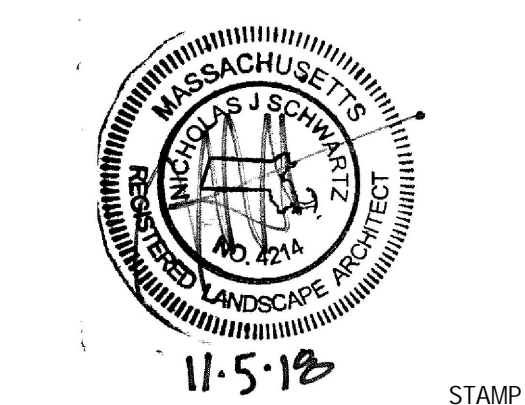
VANASSE & ASSOCIATES

35 NEW ENGLAND BUS CENTER DR.
SUITE 140
ANDOVER, MA 01810
P 978 474.8800
TRAFFIC

McPHAIL ASSOCIATES

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GEOTECH

KEY PLAN



DATE: NOVEMBER 5, 2018
REVISION: BZA SUBMISSION

REVISIONS ON SHEET

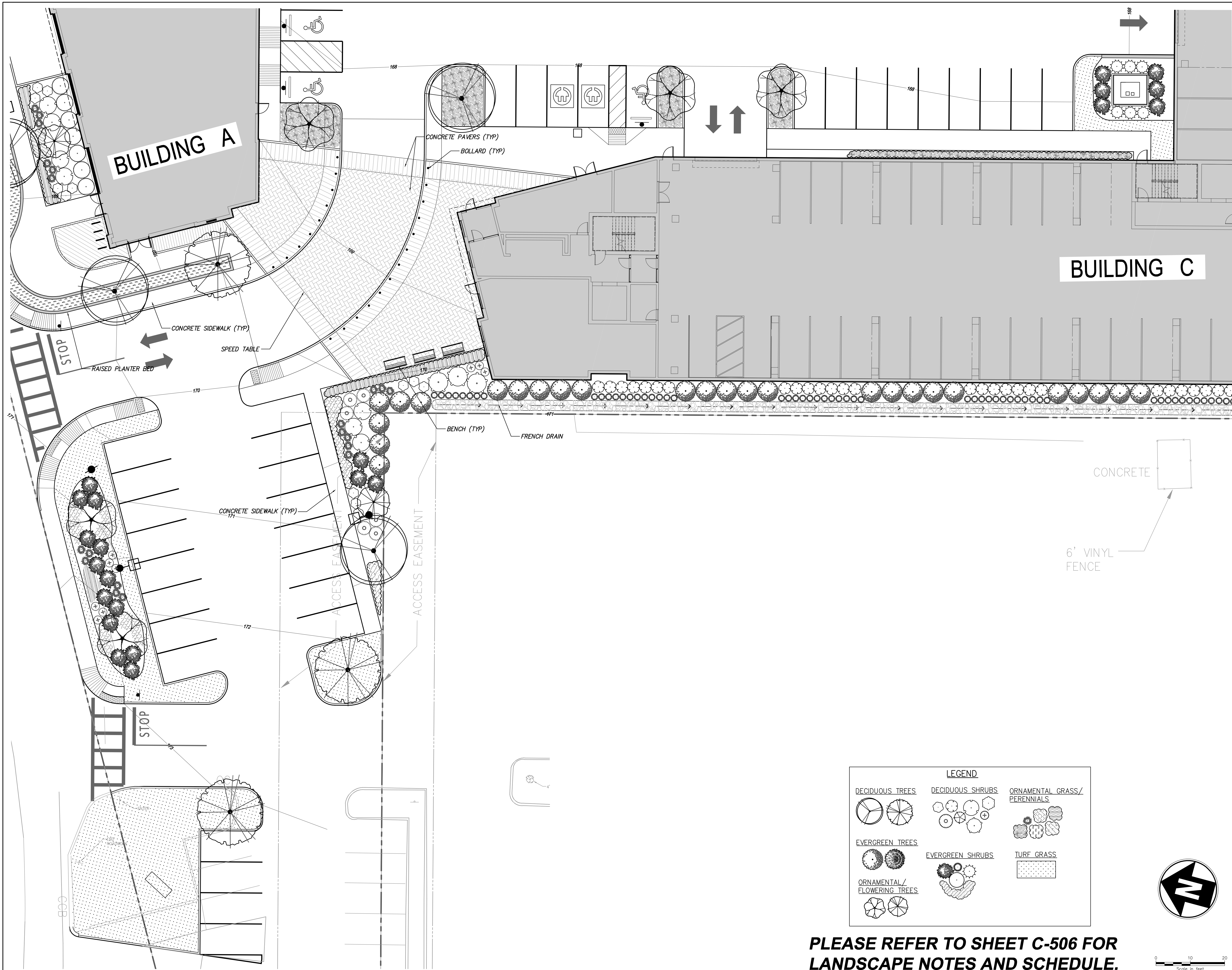
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LANDSCAPE PLAN

C-502

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**PLEASE REFER TO SHEET C-506 FOR
LANDSCAPE NOTES AND SCHEDULE.**

**THE
RESIDENCES
AT 1500 MAIN**
1500 MAIN STREET
WEYMOUTH, MA 02190
PROJECT
JOHN M. CORCORAN &
CO. LLC
100 GRANDVIEW ROAD, SUITE 203
BRAintree, MA 02184
P 781.849.0011
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ARCHITECT

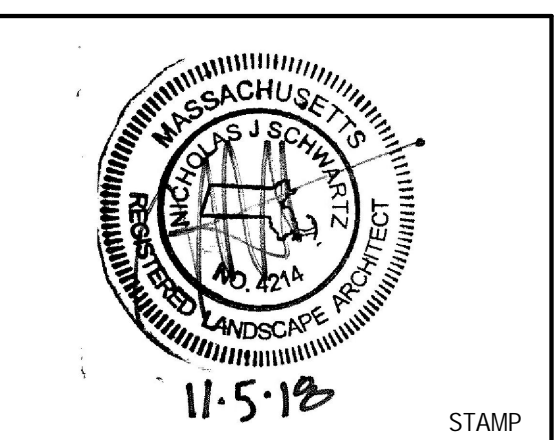
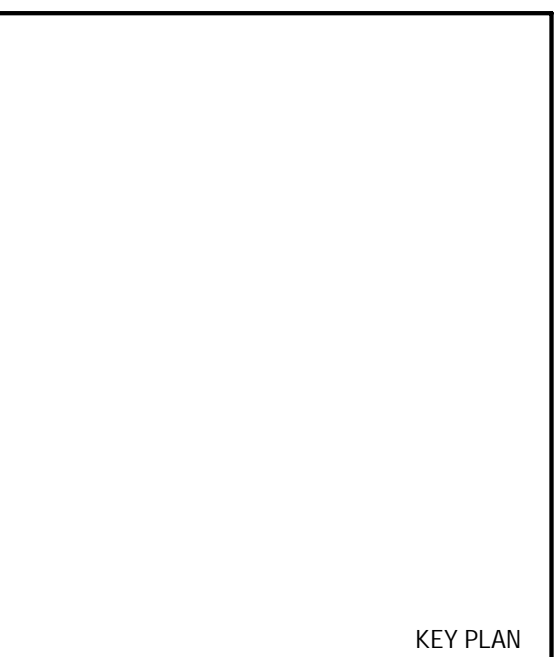
CHA COMPANIES
141 LONGWATER DRIVE, SUITE 104
NORWELL, MA 02061
P 781.982.5400
CIVIL & LANDSCAPE

BLW ENGINEERS
311 GREAT ROAD
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P 978.486.4301
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STRUCTURAL

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P 617.868.1420
GEOTECH



DATE: NOVEMBER 5, 2018
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REVISIONS ON SHEET

SCALE: UTILITE PROJECT NUMBER 1839

LANDSCAPE PLAN

C-503

10/23/2018 2:17:14 PM

1500 MAIN STREET
WEYMOUTH, MA 02190

PROJECT

**JOHN M. CORCORAN &
CO. LLC**

100 GRANDVIEW ROAD, SUITE 203
BRAintree, MA 02184
P 781 849.0011

OWNER

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BOSTON, MA 02111
P 617 423.7200 F 617 423.1414
utiledesign.com ARCHITECT

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NORWELL, MA 02061
P 781 982.5400

311 GREAT ROAD
LITTLETON, MA 01460
P 978 486.4301

639 GRANITE ST
BRAINTREE, MA 02184
P 781 849.2065

35 NEW ENGLAND BUS CENTER DR.
SUITE 140
ANDOVER, MA 01810
P 978 474.8800

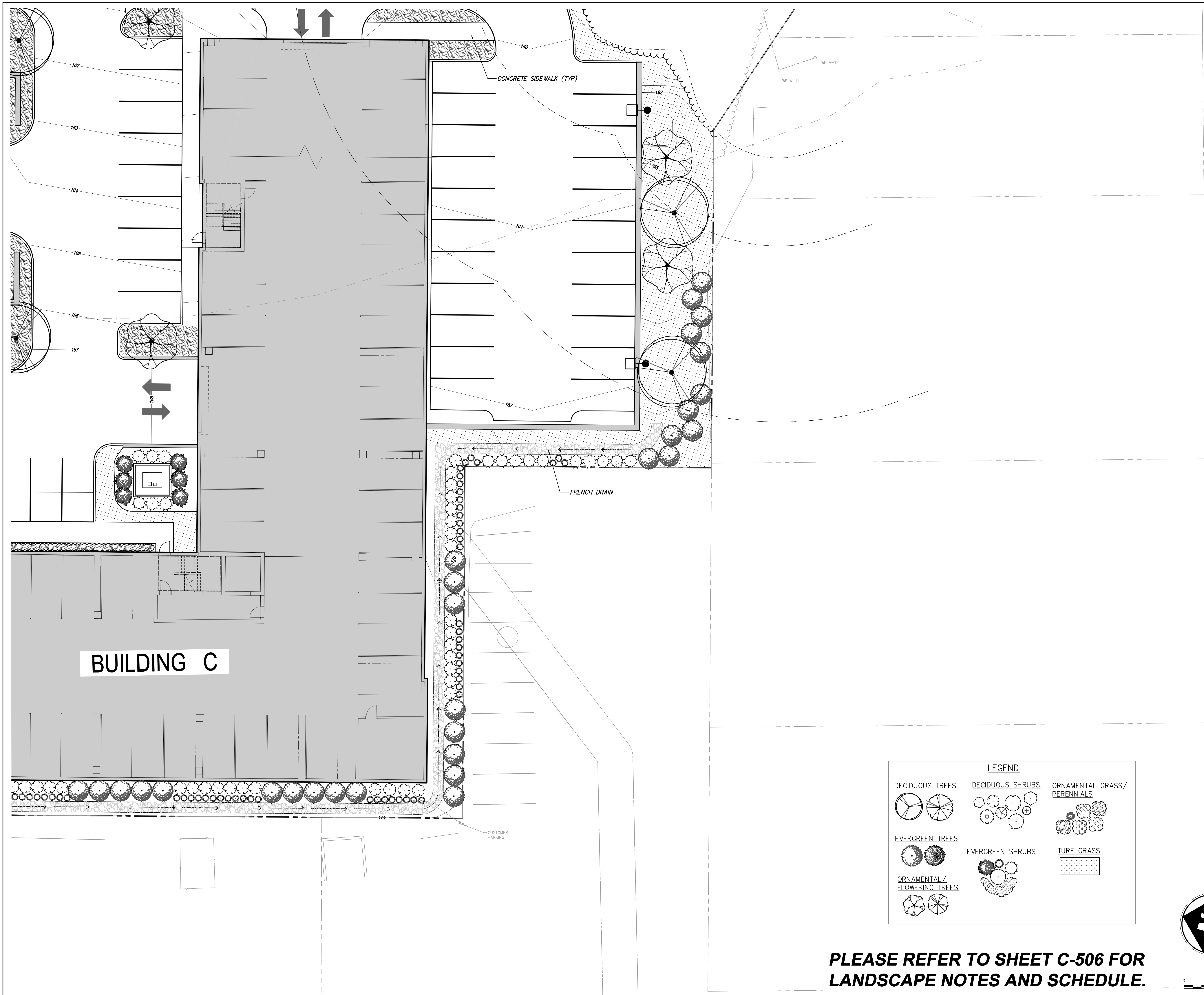
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CAMBRIDGE, MA 02140
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GEOTECH

DATE	REVISION
NOVEMBER 5, 2018	BZA SUBMISSION

SCALE	UTILE PROJECT NUMBER
	1839

C-504



**PLEASE REFER TO SHEET C-506 FOR
LANDSCAPE NOTES AND SCHEDULE.**



A horizontal scale bar with a black and white checkered pattern. It has tick marks at 0, 10, and 20. Below the bar is the text "Scale in feet".

THE
RESIDENCES
AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190
PROJECT
JOHN M. CORCORAN &
CO. LLC
100 GRANDVIEW ROAD, SUITE 203
BRAINTREE, MA 02184
P 781 849.0011
OWNER

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ARCHITECT

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VEITAS AND VEITAS

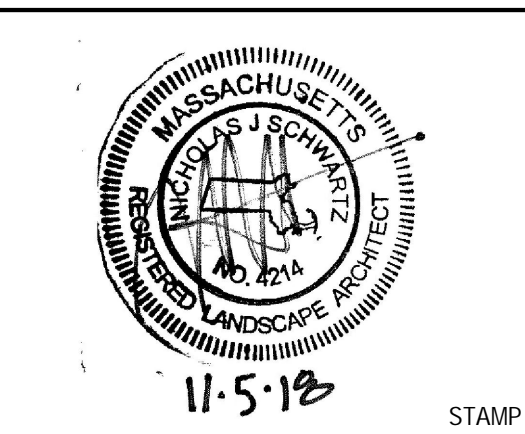
639 GRANITE ST
BRAINTREE, MA 02184
P 781 849.2065
STRUCTURAL

VANASSE & ASSOCIATES

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TRAFFIC

McPHAIL ASSOCIATES

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CAMBRIDGE, MA 02140
P 617 868.1420
GEOTECH



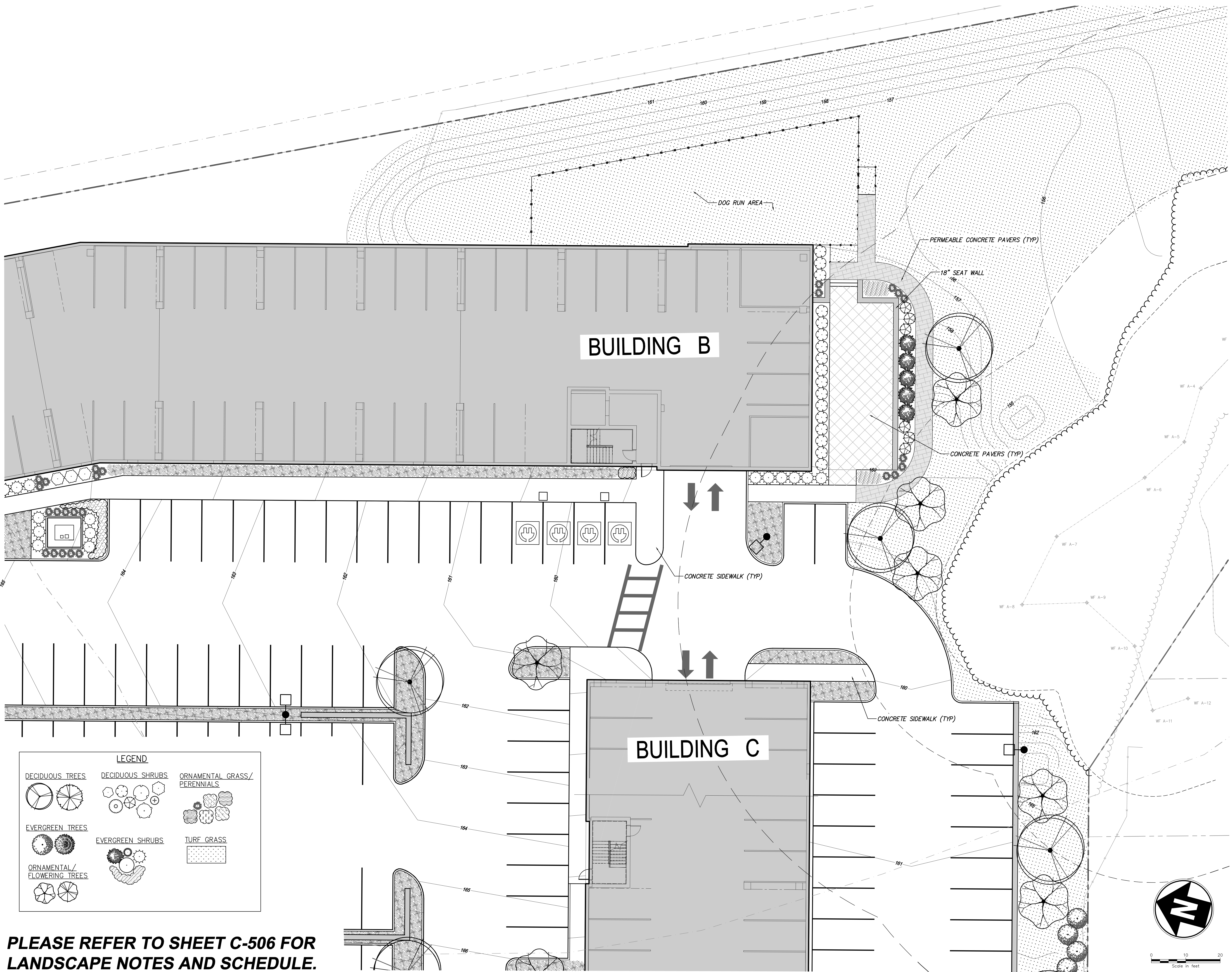
DATE REVISION
NOVEMBER 5, 2018 BZA SUBMISSION

REVISIONS ON SHEET

SCALE UTILE PROJECT NUMBER
1839

LANDSCAPE PLAN

C-505



PLEASE REFER TO SHEET C-506 FOR
LANDSCAPE NOTES AND SCHEDULE.

LANDSCAPE NOTES:

1.

THE CONTRACTOR SHALL SUPPLY PLANT MATERIALS IN QUANTITIES SUFFICIENT TO COMPLETE THE WORK SHOWN. ANY DISCREPANCIES BETWEEN QUANTITIES SHOWN ON THE PLANT SCHEDULE AND THOSE REQUIRED BY THE CONTRACT DRAWINGS SHALL NOT ENTITLE THE CONTRACTOR TO ADDITIONAL REMUNERATION.
2.

THE CONTRACTOR SHALL VERIFY AND VISUALLY INSPECT FINAL SELECTION OF PLANT MATERIALS WITH THE LANDSCAPE ARCHITECT OR OWNER PRIOR TO INSTALLATION.
3.

ALL PLANTING LOCATIONS SHALL BE STAKED OUT AND APPROVED IN THE FIELD BY THE LANDSCAPE ARCHITECT OR OWNER PRIOR TO INSTALLATION.
4.

NO PLANT MATERIAL WILL BE ACCEPTED WHICH DISPLAYS MAJOR IRREGULARITIES OR MECHANICAL DAMAGE. THE OWNER RETAINS THE RIGHT TO REJECT ANY PLANT MATERIAL DEEMED UNFIT.
5.

CONTRACTOR MAY SUBSTITUTE ANY OF THE PLANTINGS LISTED IN THE PLANT SCHEDULE DUE TO AVAILABILITY WITH APPROVAL BY THE LANDSCAPE ARCHITECT OR OWNER PRIOR TO INSTALLATION.
6.

EXISTING VEGETATION SHALL REMAIN UNDISTURBED IN ALL AREAS WHICH ARE NOT PART OF THE PROPOSED CONTRACT LIMIT LINE.
7.

GUARANTEE:

a.

FOR A PERIOD OF TWELVE MONTHS FROM THE DATE THAT THE WORK UNDER THIS CONTRACT IS CERTIFIED AS COMPLETE, THE CONTRACTOR SHALL: 1) GUARANTEE ALL PLANTS AND SEEDED AREAS UNDER THIS CONTRACT; 2) REMOVE AND REPLACE DURING THIS GUARANTEE PERIOD PLANTS WHICH DIE OR ARE IN A BADLY IMPAIRED CONDITION; 3) REPLANT WITH STOCK OF SAME SIZE AND QUALITY AS ORIGINALLY SPECIFIED; 4) GUY AND MAINTAIN AS SPECIFIED HEREIN AT NO ADDITIONAL COST TO THE OWNER.

b.

REPLACEMENTS MADE WITHIN SIX MONTHS AFTER THE BEGINNING OF THE GUARANTEE PERIOD SHALL NOT EXTEND THE GUARANTEE PERIOD OF THESE PARTICULAR PLANTS. THOSE REPLACEMENTS MADE SIX MONTHS OR MORE AFTER THE BEGINNING OF THE GUARANTEE PERIOD SHALL BE MAINTAINED AND GUARANTEED FOR A PERIOD OF SIX MONTHS FROM THE TIME OF PLACEMENT.
8.

MULCH: PROVIDE MINIMUM 3" THICK LAYER OF DARK BROWN SHREDDED BARK MULCH. MULCH SHALL BE USED ONLY AS TREE COLLARS AND IN PLANTING BED AS SHOWN ON THE PLANS.
9.

WATER THOROUGHLY DURING AND IMMEDIATELY AFTER PLANTING.

TOPSOIL NOTES:

1.

ALL AREAS OF THE SITE WHICH ARE DISTURBED AND NOT PLANTED, MULCHED, PAVED, ETC. SHALL BE TOPSOILED AND SEEDED. TOPSOIL TO BE INSTALLED TO A MINIMUM 3" DEPTH IN AREAS OF SEED. SEED SHALL BE FRESH, CLEAN, NEW-CROP SEED MIXED IN WITH SPECIES AND VARIETY CONFORMING TO FEDERAL AND STATE STANDARDS.
2.

PROVIDE AND INSTALL A MULCH ADEQUATE TO PROTECT THE SEEDING DURING ITS GROWING PERIOD. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE THE APPROPRIATE MULCHING TECHNIQUES FOR THE PARTICULAR SITE CONDITIONS AND ACQUIRE APPROVAL OF THE SAME FROM THE OWNER.
3.

TOPSOIL SHALL CONSIST OF FERTILE, FRIABLE, NATURAL LOAM FREE OF SUBSOIL, CLAY LUMPS, BRUSH, TWIGS, ENVIRONMENTAL CONTAMINANTS, STONES OR OTHER DELETERIOUS MATERIALS LARGER THAN 2" IN GREATEST DIMENSION.
4.

PROPOSED TOPSOIL SHALL BE TESTED BY INDEPENDENT TESTING FACILITY WITH TEST RESULTS SUBMITTED TO THE ENGINEER FOR APPROVAL. CONTRACTOR SHALL PAY FOR ALL TESTING. ACCEPTANCE OF TOPSOIL SHALL BE BASED UPON TEST RESULTS. ONE TEST SHALL BE PERFORMED PER 100 C.Y. OF TOPSOIL.

SIEVE

PERCENT PASSING

1/4"

65-100


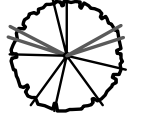
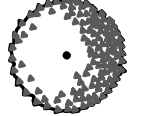
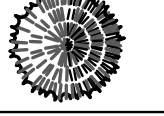
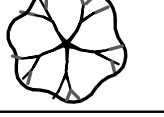
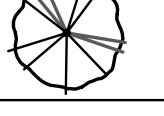
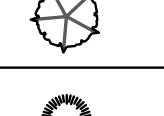
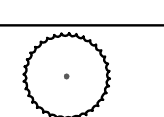
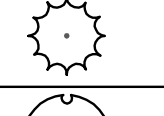
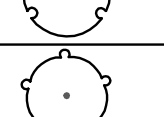
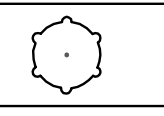

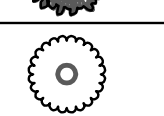
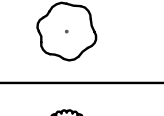
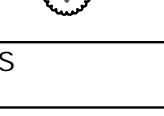


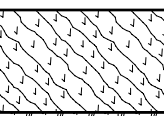
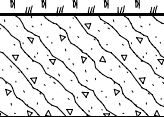
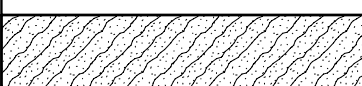
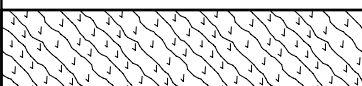




1"

85-100

NO. 200

20-60
5.

NATURAL TOPSOIL MAY BE AMENDED WITH APPROVED MATERIALS, BY APPROVED METHODS, TO MEET THE ABOVE SPECIFICATIONS.

PLANT SCHEDULE	
DECIDUOUS TREES	COMMON NAME
	OCTOBER GLORY MAPLE
	SHADEMASTER LOCUST
EVERGREEN TREES	COMMON NAME
	DARK AMERICAN ARBORVITAE
	WHITE SPRUCE
FLOWERING TREES	COMMON NAME
	"AUTUMN BRILLIANCE" SERVICEBERRY
	KOUSA DOGWOOD
SHRUBS	COMMON NAME
	ARTIC FIRE DOGWOOD
	BLUE ARROW JUNIPER
	GREENWAVE SPREADING YEW
	INKBERRY
	KOREAN SPICE VIBURNUM
	LIMELIGHT HYDRANGEA
	LITTLE LIME HYDRANGEA
	RED SPRITE WINTERBERRY
	SEA GREEN JUNIPER
	SUMMERSWEET
	THE FAIRY ROSE
	VIRGINIA SWEETSPIRE
GRASSES	COMMON NAME
	BLUE SWITCH GRASS
SHRUB AREAS	COMMON NAME
	BLUE CHIP JUNIPER
PERENNIALS & GRASSES	COMMON NAME
	BIG BLUE LILYTURF
	BLUE SEDGE
	COMMON PERIWINKLE
	GRASS/SEdge MIX*
	HAPPY RETURNS DAYLILY

* GRASS/SEdge MIX INCLUDES:
BLUE SEDGE
CREEPING RED FESCUE
LITTLE BUNNY SWITCHGRASS
PRAIRIE MUNCHKIN LITTLE BLUESTEM

THE RESIDENCES AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190

PROJECT

JOHN M. CORCORAN & CO. LLC

100 GRANDVIEW ROAD, SUITE 203
BRAINTREE, MA 02184
P 781 849.0011

OWNER

utile

ARCHITECTURE + URBAN DESIGN

115 KINGSTON ST
BOSTON, MA 02111
P 617 423.7200 F 617 423.1414
utiledesign.com

ARCHITECT

CHA COMPANIES

141 LONGWATER DRIVE, SUITE 104
NORWELL, MA 02061
P 781 982.5400

CIVIL & LANDSCAPE

BLW ENGINEERS

311 GREAT ROAD
LITTLETON, MA 01460
P 978 486.4301

MEPPF

VEITAS AND VEITAS

639 GRANITE ST
BRAINTREE, MA 02184
P 781 849.2065

STRUCTURAL

VANASSE & ASSOCIATES

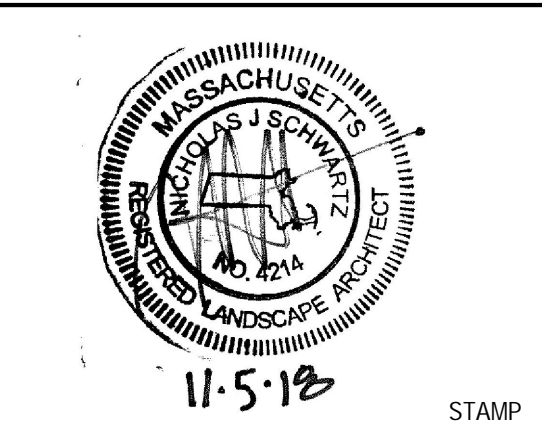
35 NEW ENGLAND BUS CENTER DR.
SUITE 140
ANDOVER, MA 01810
P 978 474.8800

TRAFFIC

McPHAIL ASSOCIATES

2269 MASSACHUSETTS AVE.
CAMBRIDGE, MA 02140
P 617 868.1420

GEOTECH



DATE	REVISION
NOVEMBER 5, 2018	BZA SUBMISSION

REVISIONS ON SHEET

SCALE	UTILE PROJECT NUMBER
	1839

LANDSCAPE SCHEDULE & NOTES

C-506

THE
RESIDENCES
AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190

PROJECT

JOHN M. CORCORAN &
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GEOTECH

KEY PLAN

SEAL

DATE: NOVEMBER 5, 2018

REVISION: BZA SUBMISSION

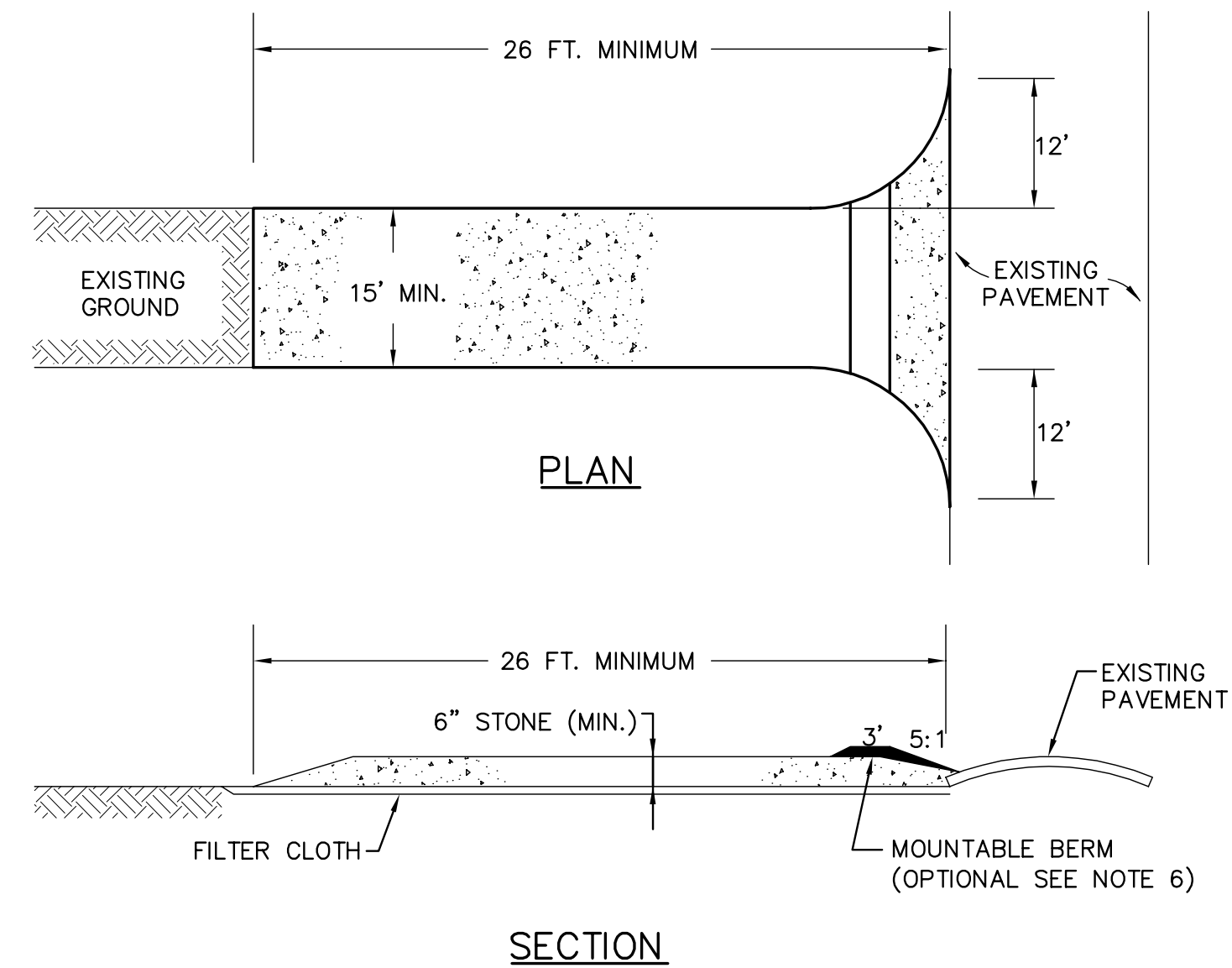
REVISIONS ON SHEET

SCALE: 1"=10'

UTILITE PROJECT NUMBER: 1839

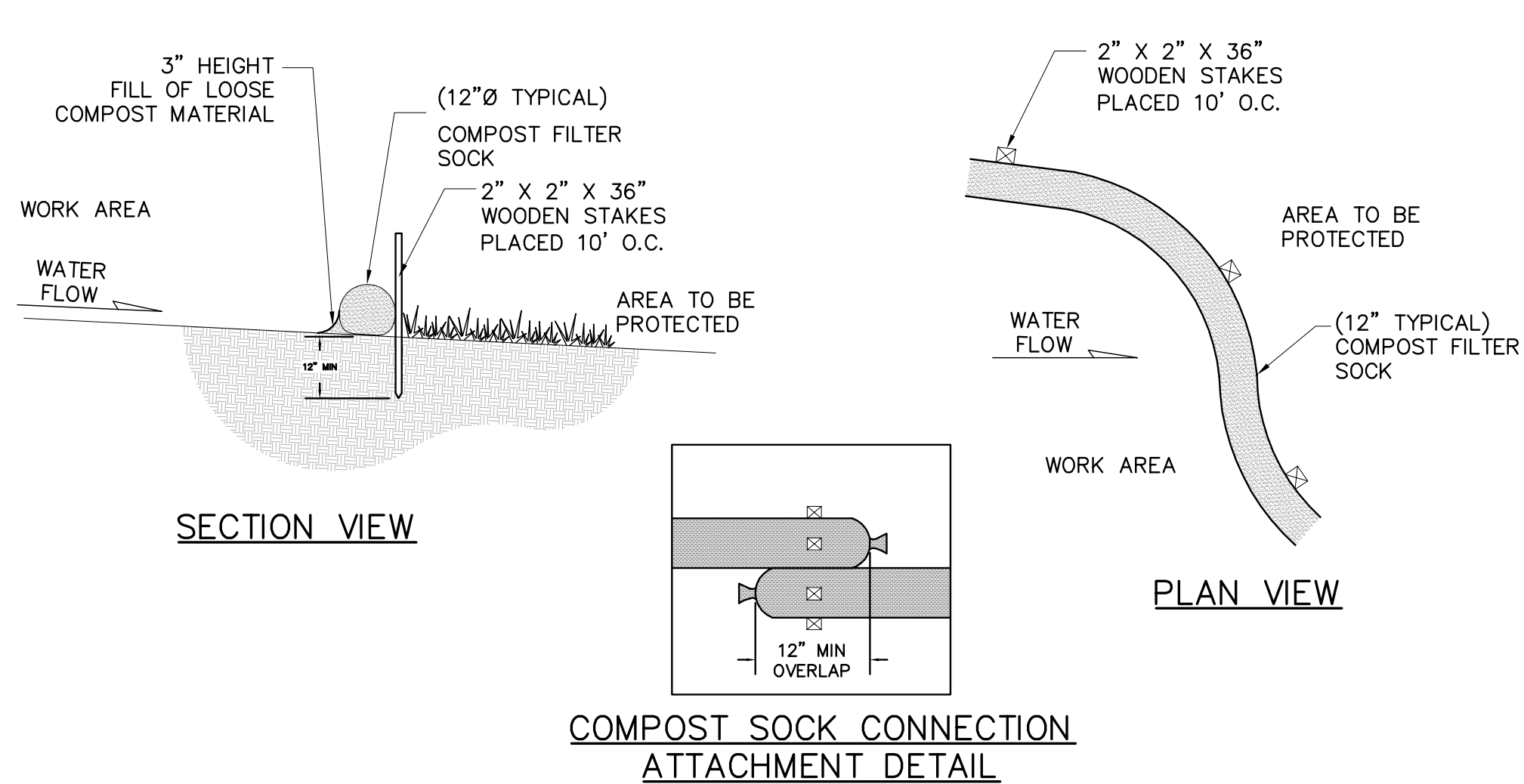
DETAILS - 1

C-601



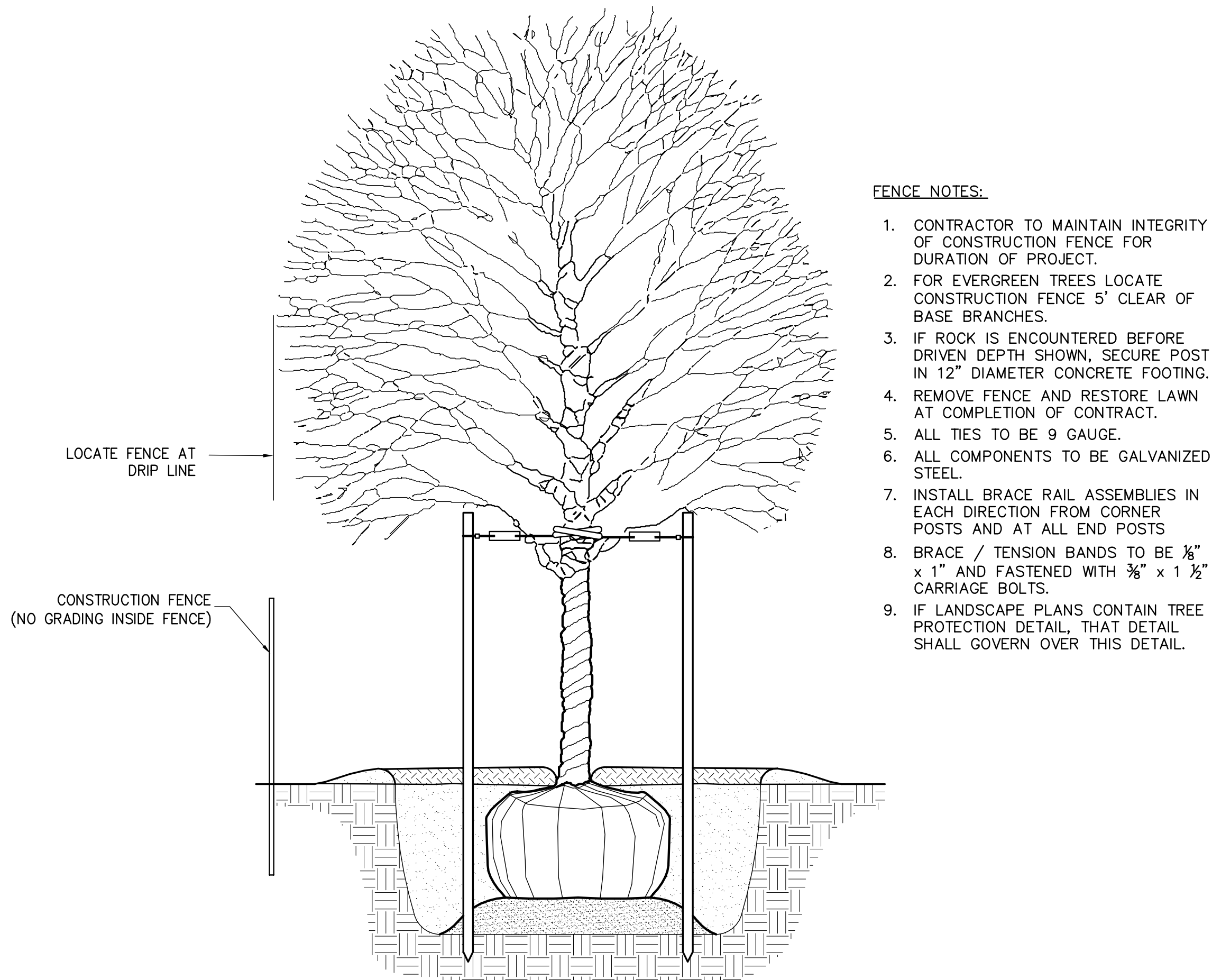
- CONSTRUCTION SPECIFICATIONS:**
- STONE SIZE: USE 2" STONE, OR RECLAIMED OR RECYCLED CONCRETE EQUIVALENT.
 - LENGTH: RECOMMEND GREATER THAN OR EQUAL TO 30 FEET WHERE SOILS ARE SANDS AND GRAVELS AND 100 FEET IN SILTS AND CLAYS.
 - THICKNESS: NOT LESS THAN SIX (6) INCHES.
 - WIDTH: FIFTEEN (15) FOOT MINIMUM, BUT NOT LESS THAN THE FULL WIDTH AT POINTS WHERE INGRESS OF EGRESS OCCURS.
 - FILTER CLOTH: SHALL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING OF STONE.
 - SURFACE WATER: ALL SURFACE WATER FLOWING OR DIVERTED TOWARD CONSTRUCTION ENTRANCES SHALL BE PIPED ACROSS THE ENTRANCE. IF PIPING IS IMPRACTICAL, A MOUNTABLE BERM SHALL BE INSTALLED.
 - MAINTENANCE: THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH SHALL PREVENT TRACKING OF FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND AND REPAIR OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHTS-OF-WAY MUST BE REMOVED IMMEDIATELY AND ON AN ON GOING BASIS.
 - PERIODIC INSPECTION AND NEEDED MAINTENANCE SHALL BE PROVIDED.

1 STABILIZED CONSTRUCTION ENTRANCE
SCALE: NO SCALE

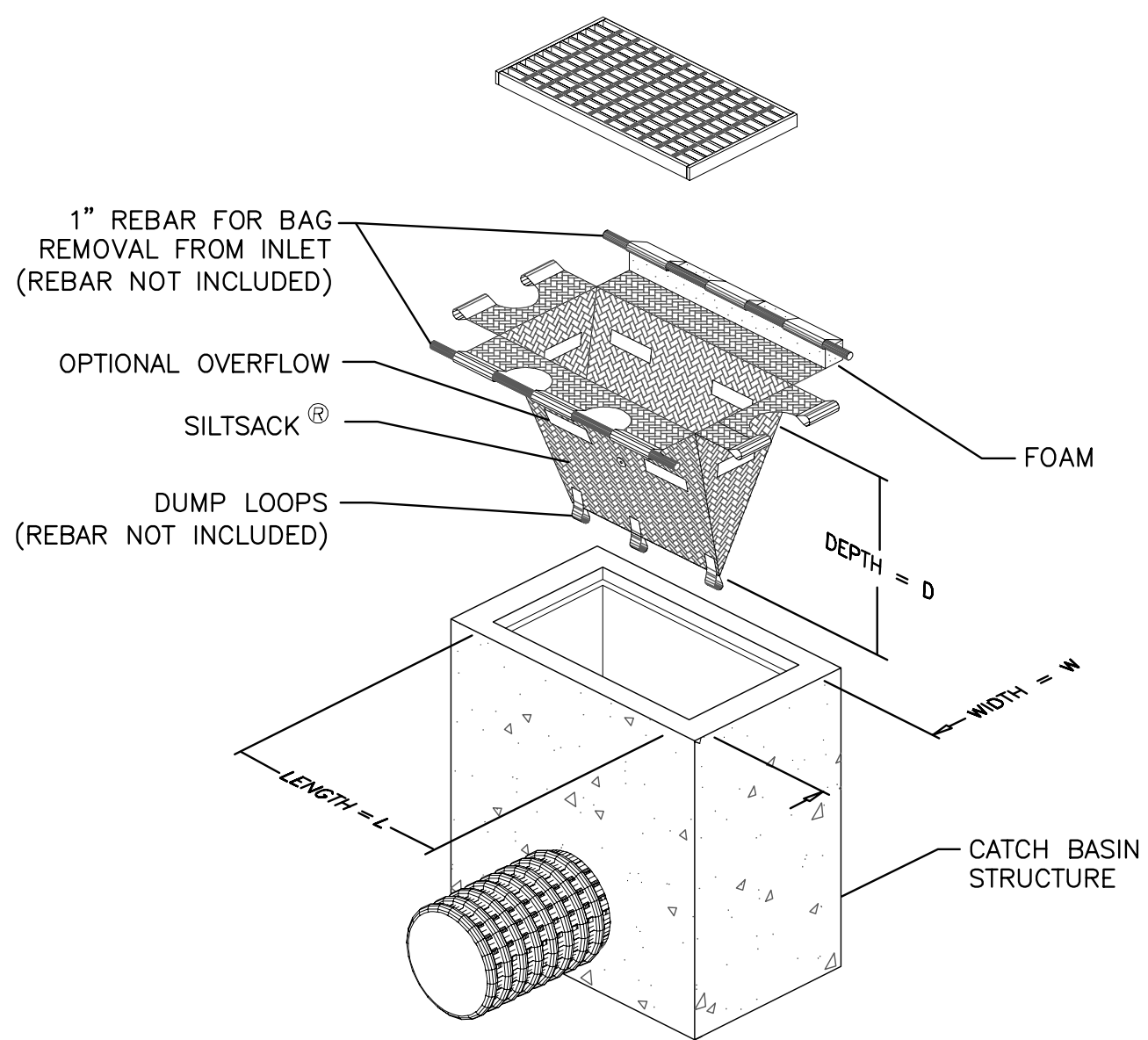


- NOTES:**
- PREFABRICATED COMPOST SOCK SHALL BE FILTREXX SOXX OR APPROVED EQUAL.
 - MATERIAL FOR SOCKS SHALL CONSIST OF SANITIZED MATURE COMPOST, FREE OF VIALBE WEED SEEDS AND FOREIGN DEBRIS SUCH AS GLASS AND PLASTIC. COMPOST SHALL BE IN SHREDDED OR GRANULAR FORM AND FREE FROM HARD LUMPS. IN ADDITION, NO KILN-DRIED WOOD OR CONSTRUCTION DEBRIS SHALL BE ALLOWED. CONTRACTOR SHALL REFER TO MASSDOT SPECIFICATIONS M1.06.0 FOR MATERIAL SPECIFICATIONS.
 - SOCK SHALL CONSIST OF JUTE MESH OR OTHER APPROVED BIODEGRADABLE MATERIAL.
- PRACTICE: COMPOST FILTER SOCK.**
- A COMPOST FILTER SOCK IS A TYPE OF CONTAINED COMPOST FILTER BERM CONSISTING OF A MESH TUBE FILLED WITH COMPOST MATERIAL THAT IS PLACED PERPENDICULAR TO SHEET FLOW RUNOFF TO RETAIN SEDIMENT FROM DISTURBED AREAS. THE COMPOST FILTER SOCK ACTS AS A FILTER TO RETAIN SEDIMENT AND OTHER POLLUTANTS (E.G., SUSPENDED SOLIDS, NUTRIENTS) WHILE ALLOWING THE WATER TO FLOW THROUGH IT. COMPOST QUALITY MUST MEET AASHTO 2010 SPECIFICATIONS.
- INSTALLATIONS REQUIREMENTS:**
- ONCE THE FILTER SOCK IS FILLED AND PUT IN PLACE, IT SHOULD BE ANCHORED TO THE SLOPE BY STAKES ALONG THE DOWNHILL SIDE OF THE SOCK AT 10' SPACING ON CENTER (O.C.) THE ENDS OF THE FILTER SOCK SHOULD BE DIRECTED UPSLOPE, TO PREVENT STORMWATER FROM RUNNING AROUND THE END OF THE TUBE.
- MAINTENANCE REQUIREMENTS:**
- SOCK MUST BE INSPECTED FOR SEDIMENT ACCUMULATION. IF THERE IS EXCESSIVE PONDING BEHIND THE FILTER SOCK OR ACCUMULATED SEDIMENT REACHES THE TOP OF THE SOCK, AN ADDITIONAL SOCK SHOULD BE ADDED ON TOP OR INFRONT OF THE EXISTING SOCK IN THESE AREAS. AN ADEQUATE RESERVE OF SOCKS MUST BE KEPT ON SITE AT ALL TIMES FOR EMERGENCY AND/OR ROUTINE REPLACEMENT. SOCKS SHALL BE REMOVED ONLY AFTER EXPOSED SOILS IN THE CONTRIBUTING DRAINAGE AREA ACHIEVE FINAL STABILIZATION. SEDIMENT ACCUMULATION MUST BE REMOVED ONCE IT HAS REACHED 1/2 OF THE EXPOSED HEIGHT OF THE SOCK.

2 12-INCH COMPOST FILTER SOCK
SCALE: NO SCALE

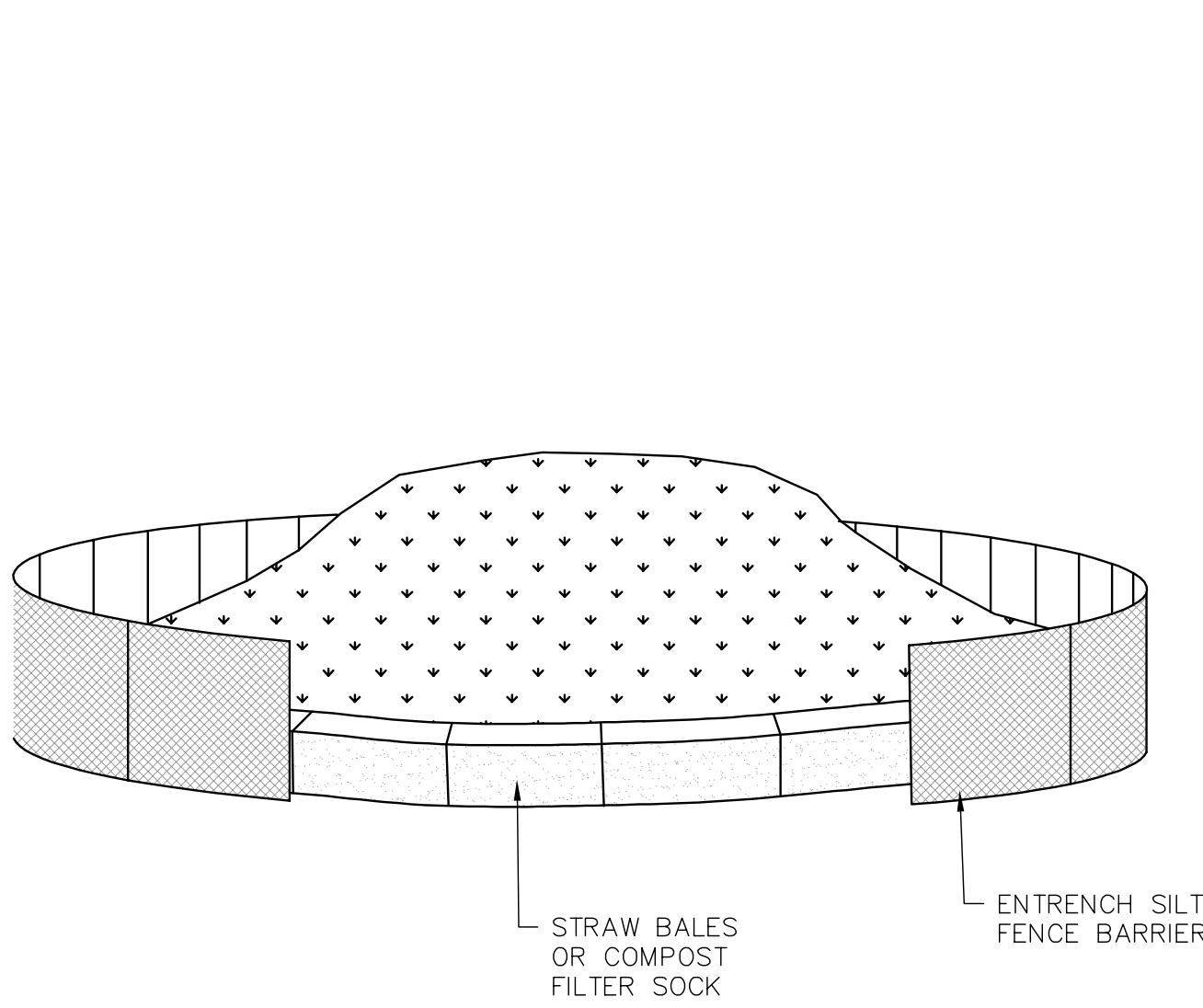


3 TREE PROTECTION DETAIL
SCALE: NO SCALE



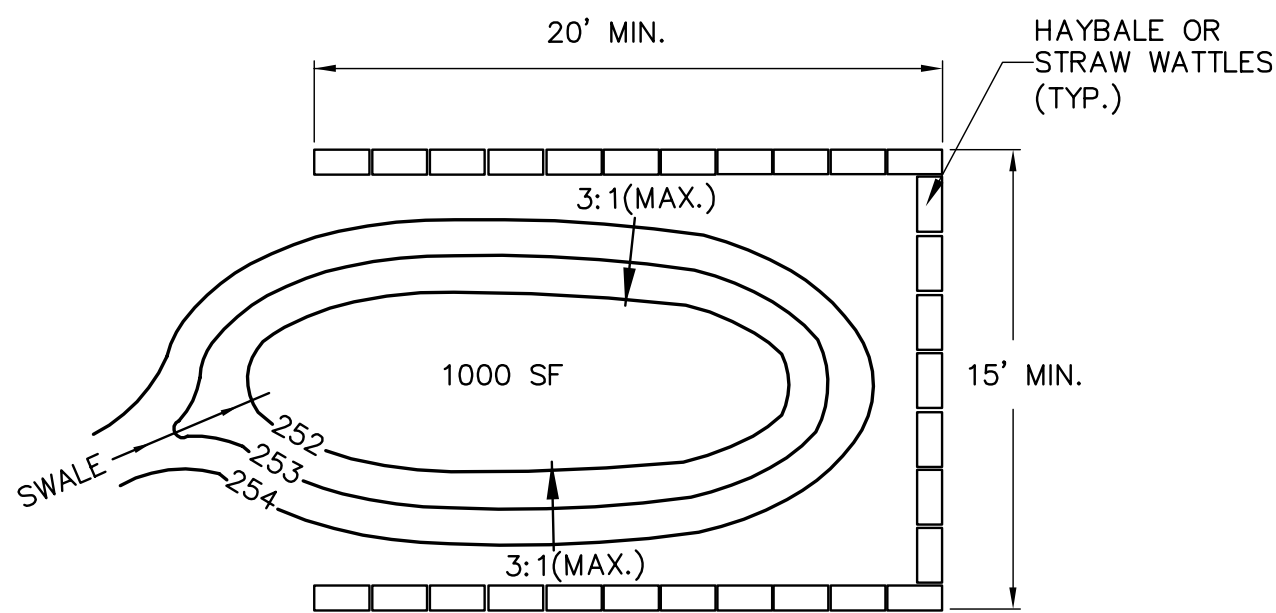
- NOTES:**
- REMOVE ACCUMULATED SEDIMENT WHEN CAPACITY IS REDUCED BY HALF OR PER MANUFACTURER'S RECOMMENDATIONS.
 - CATCH BASIN INSERT TO BE SILTSACK® BY ACF ENVIRONMENTAL, PHONE: 1-800-448-3636, acfenvironmental.com OR EQUIVALENT.
 - INSTALL ON ALL EXISTING CATCH BASINS WITHIN MAIN STREET AND ON SITE. INSTALL ON ALL PROPOSED CATCH BASINS AFTER STRUCTURE INSTALLATION.

4 FILTER SACK CATCH BASIN INSERT DETAIL
SCALE: NO SCALE



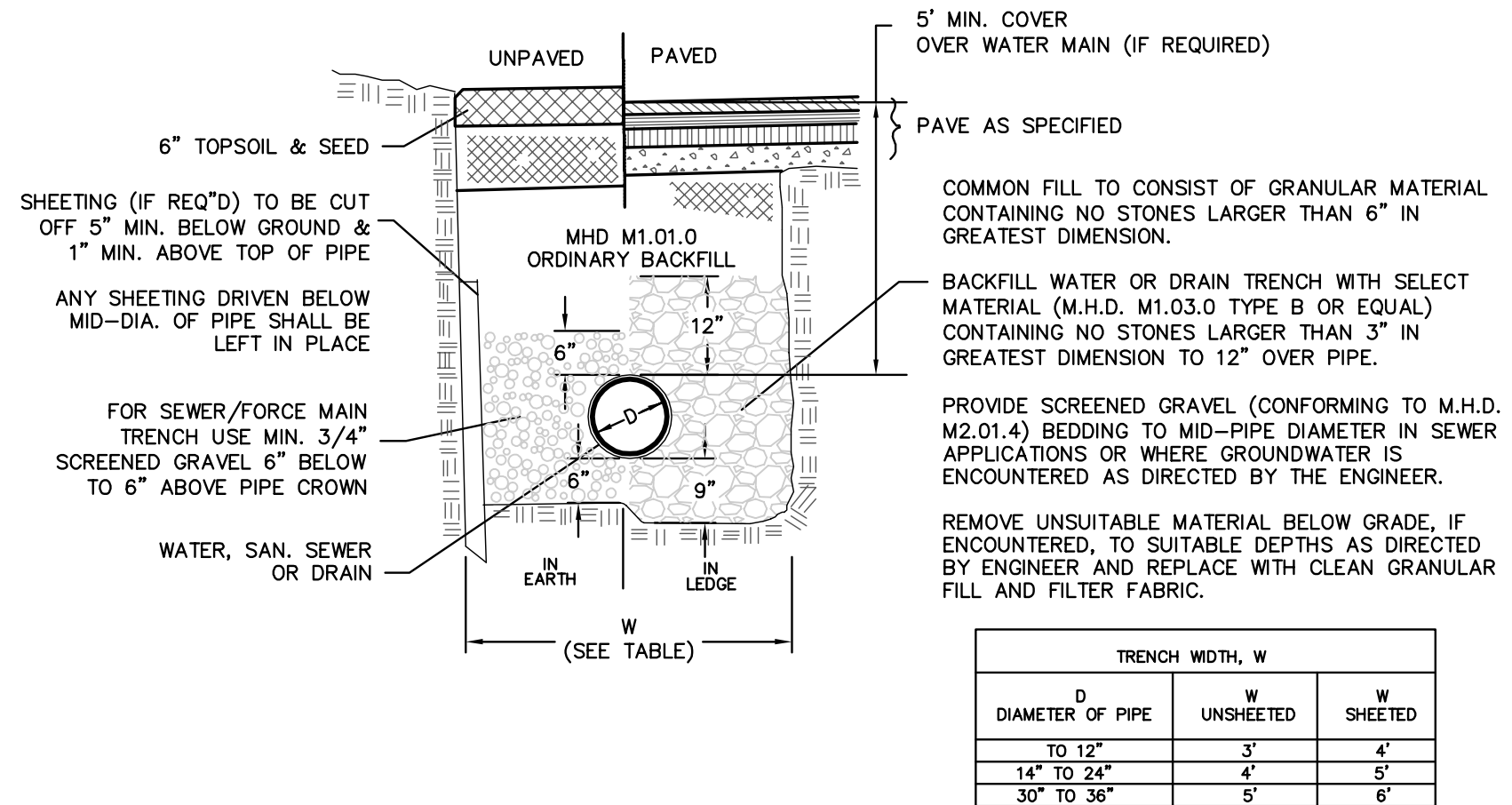
- INSTALLATION NOTES:**
- AREA CHOSEN FOR STOCKPILING OPERATIONS SHALL BE DRY AND STABLE.
 - MAXIMUM SLOPE OF STOCKPILE SHALL BE 2:1.
 - UPON COMPLETION OF STOCKPILING, EACH PILE SHALL BE SURROUNDED WITH STRAW BALES, THEN STABILIZED WITH VEGETATION, OR COVERED.
 - SEE SPECIFICATIONS FOR INSTALLATION OF STRAW BALES.

5 SOIL STOCKPILE
SCALE: NO SCALE



- SEDIMENT BASINS AND TRAPS SHALL BE SIZED IN ACCORDANCE WITH EPA NPDES GUIDELINES.
- SEDIMENT TRAPS ARE UTILIZED FOR DRAINAGE AREAS SMALLER THAN 5 ACRES. THE SEDIMENT TRAP SHOULD HAVE A MINIMUM VOLUME BASED ON 1/2 INCH OF STORAGE FOR EACH ACRE OF DRAINAGE AREA. THIS VOLUME EQUATES TO 1800 CUBIC FEET OF STORAGE OR 67 CUBIC YARDS FOR EACH ACRE OF DRAINAGE AREA.
- SEDIMENT BASIN ARE UTILIZED FOR DRAINAGE AREAS FROM 5 TO 100 ACRES. THE TEMPORARY SEDIMENT BASIN SHOULD HAVE A MINIMUM VOLUME OF 3,600 CUBIC FEET FOR EACH ACRE OF DRAINAGE AREA.
- LOCATION DICTATED BY SEQUENCE OF CONSTRUCTION. CONTRACTOR TO PROVIDE WHERE NECESSARY TO FILTER RUNOFF FROM CONSTRUCTION AREAS PRIOR TO DISCHARGE.

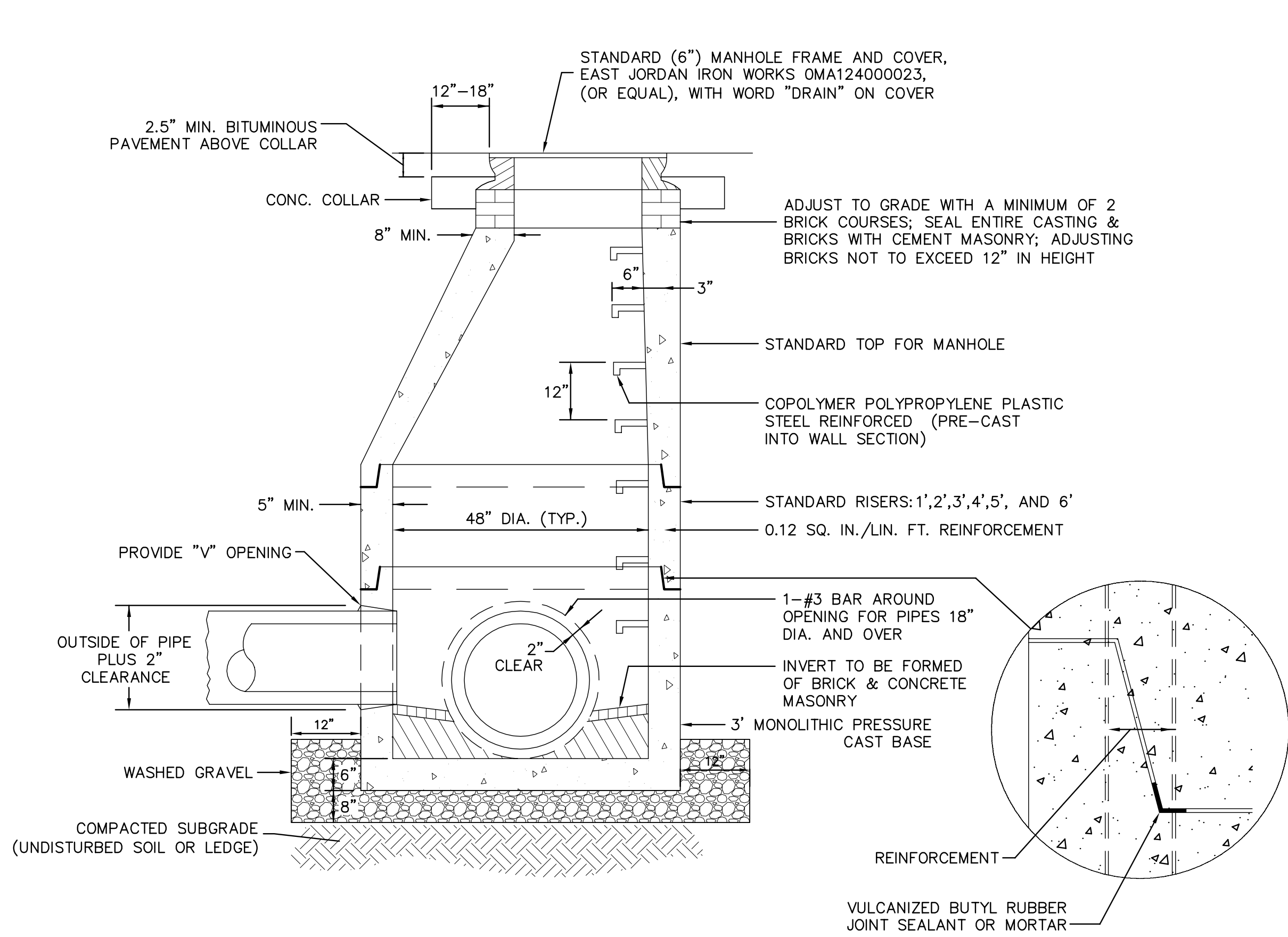
6 TEMPORARY SEDIMENT BASIN
SCALE: NO SCALE



- NOTES:**
- ALL TRENCH CONSTRUCTION TO CONFORM TO APPLICABLE FEDERAL, STATE AND LOCAL REGULATIONS.
 - COMPACT FILL AND TAMP PIPE TO 95% MAX. DRY DENSITY IN 8" LIFTS UNLESS OTHERWISE SPECIFIED.
 - INSTALL DUCTILE IRON WATER PIPE IN ACCORDANCE WITH ANSI A21.51 (AWWA C151) LAYING CONDITION TYPE 2. BACKFILL TO CONFORM TO MHD M1.03.0 GRAVEL BURROW TYPE c TO 12" ABOVE PIPE CROWN OR AS DIRECTED BY MANUFACTURER OR ENGINEER.
 - MATERIALS FOR SEWER BEDDING, HAUNCHING, AND BACKFILL TO CONFORM TO CLASSES I, II, OR III AS DESCRIBED IN ASTM D 2321 AND TR-16 GUIDES FOR THE DESIGN OF WASTEWATER TREATMENT WORKS.
 - BC TO 12" ABOVE PIPE CROWN OR AS DIRECTED BY MANUFACTURER OR ENGINEER.
 - PROVIDE MINIMUM 5' FT. COVER OVER WATER MAIN AS MEASURED FROM BOTTOM OF CURB LINE. INSULATE WATER MAIN IN ACCORDANCE WITH M.H.D. SECTION 301 WATER SYSTEMS IN AREAS PRONE TO FROST ACTION AND/OR LESS THAN 5' MIN. COVER.

7 TYPICAL TRENCH SECTION
SCALE: NO SCALE

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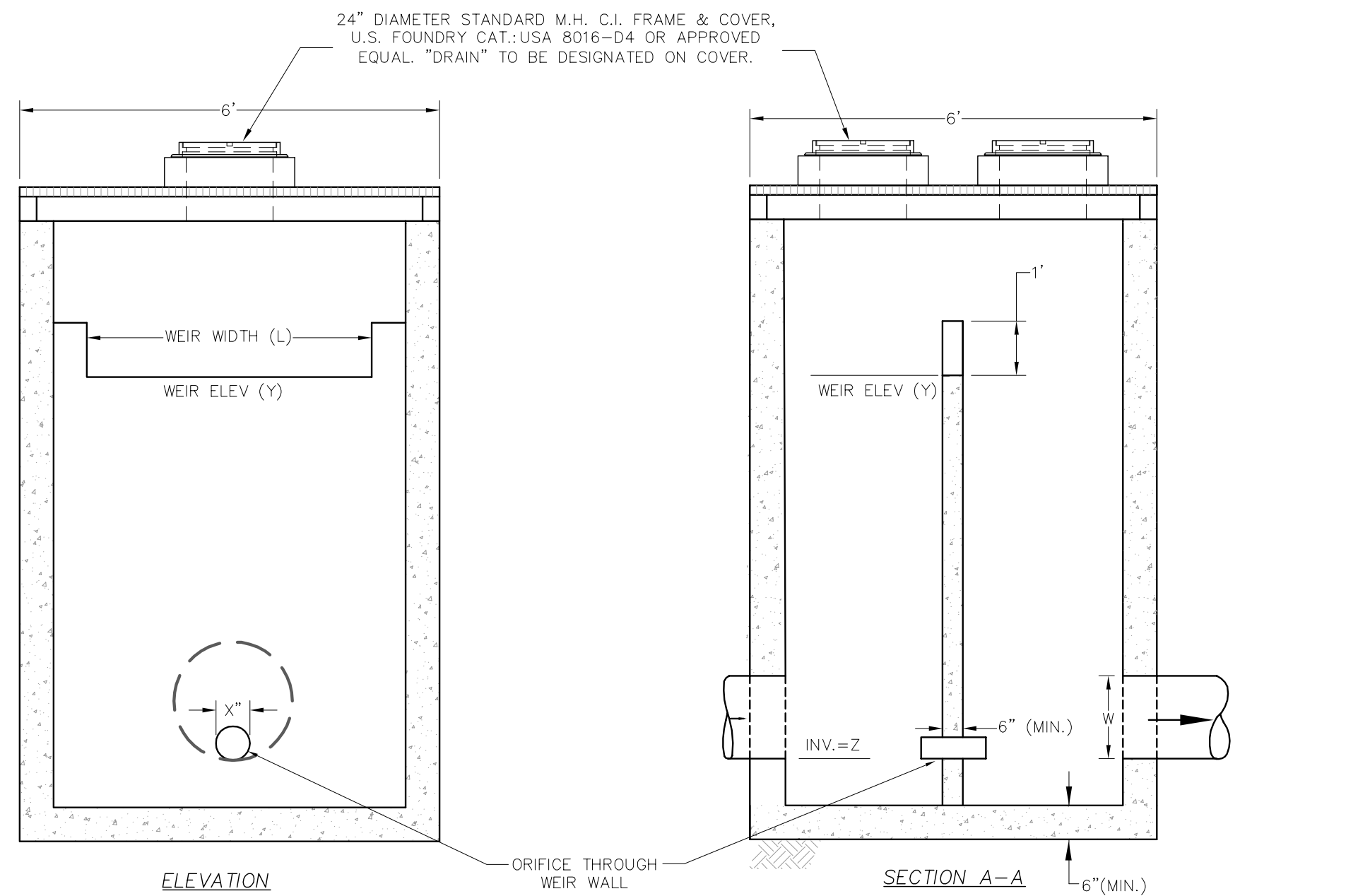


NOTE:

1. CONTRACTOR TO CONFIRM ALL SIZES IN SHOP DRAWING PHASE.
2. CONTRACTOR SHALL PROVIDE SIGNED & SEALED BUOYANCY CALCULATIONS FOR ALL PRECAST STRUCTURES WITH SHOP DRAWINGS SUBMITTED TO CHA. CALCULATIONS SHALL ASSUME THE HIGHER OF FEMA FLOODPLAIN ELEVATION OR SEASONAL HIGH GROUNDWATER ELEVATION FOR FOR GROUNDWATER DEPTH AT EACH STRUCTURE.

5 STANDARD PRECAST DRAIN MANHOLE

SCALE: NO SCALE



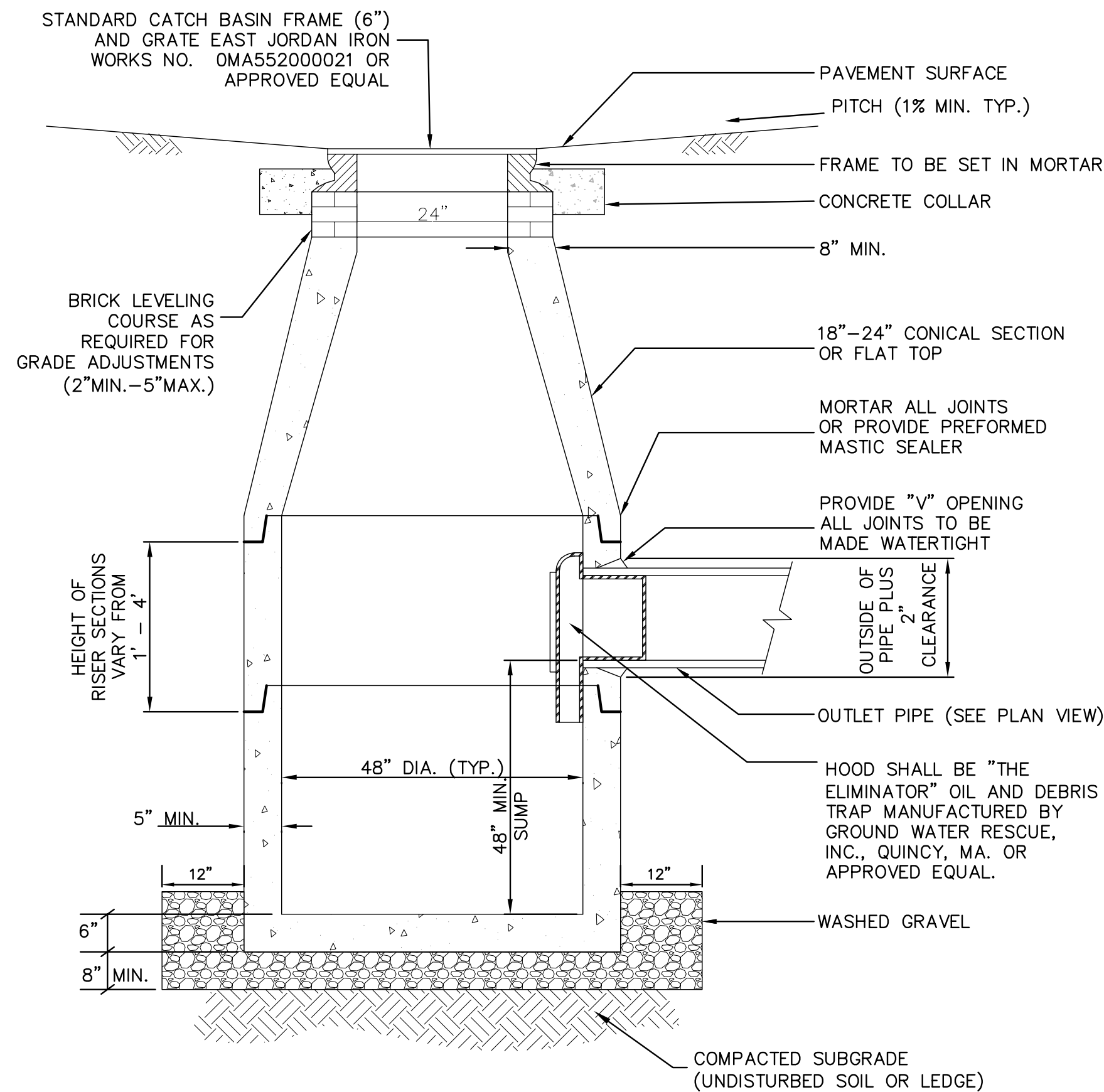
OUTLET CONTROL STRUCTURE (OCS) TABLE				
NAME	ORIFICE DIA (")	ORIFICE INV	WEIR WIDTH (L)	WEIR ELEV
OCS #1	10"	161.80	6'	164.50
OCS #2	4"	159.75	6'	162.50
OCS #3	6"	157.60	6'	159.75
OCS #4	10"	157.00	6'	159.75
OCS #5	8"	156.45	6'	158.00

NOTES:

1. SEE PLAN FOR INDIVIDUAL STRUCTURE PIPE ORIENTATION.
2. CONTRACTOR SHALL PROVIDE SIGNED & SEALED BUOYANCY CALCULATIONS FOR ALL PRECAST STRUCTURES WITH SHOP DRAWINGS SUBMITTED TO CHA. CALCULATIONS SHALL ASSUME THE HIGHER OF FEMA FLOODPLAIN ELEVATION OR SEASONAL HIGH GROUNDWATER ELEVATION FOR FOR GROUNDWATER DEPTH AT EACH STRUCTURE.
3. OUTLET STRUCTURE TO BE PRECAST CONCRETE MANUFACTURED BY ROTONDO & SONS, INC. OR APPROVED EQUAL.

4 TYPICAL OUTLET CONTROL STRUCTURE

SCALE: NO SCALE

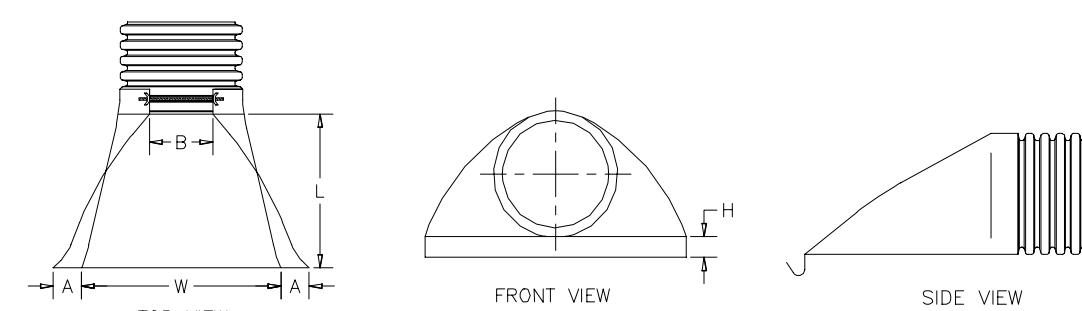


NOTE:

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2 PRECAST CONCRETE SINGLE GRATE CATCH BASIN

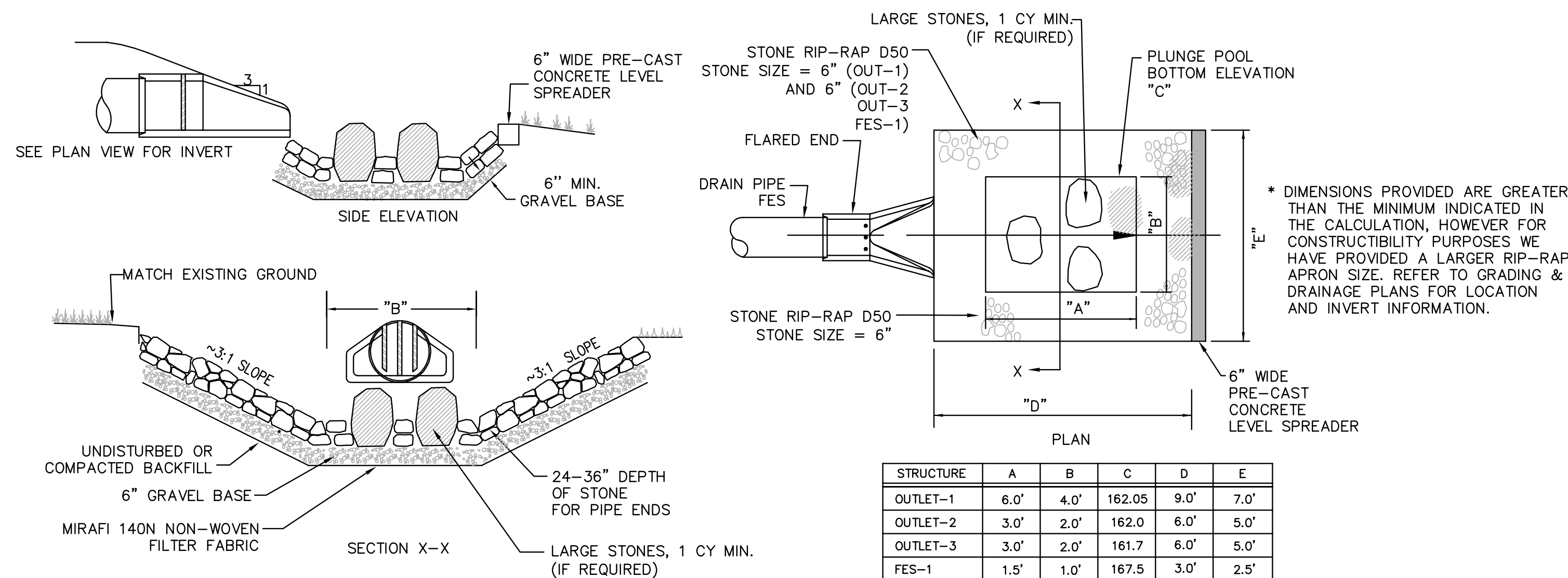
SCALE: NO SCALE



PIPE DIAMETER, (Inches)						
DIAMETER	12	15	18	24	30	36
A	6.5	6.5	7.5	7.5	7.5	7.5
B (max)	10.0	10.0	15.0	18.0	22.0	25.0
H	6.5	6.5	6.5	6.5	8.6	8.6
L	25.0	25.0	32.0	36.0	58.0	58.0
W	29.0	29.0	35.0	45.0	63.0	63.0

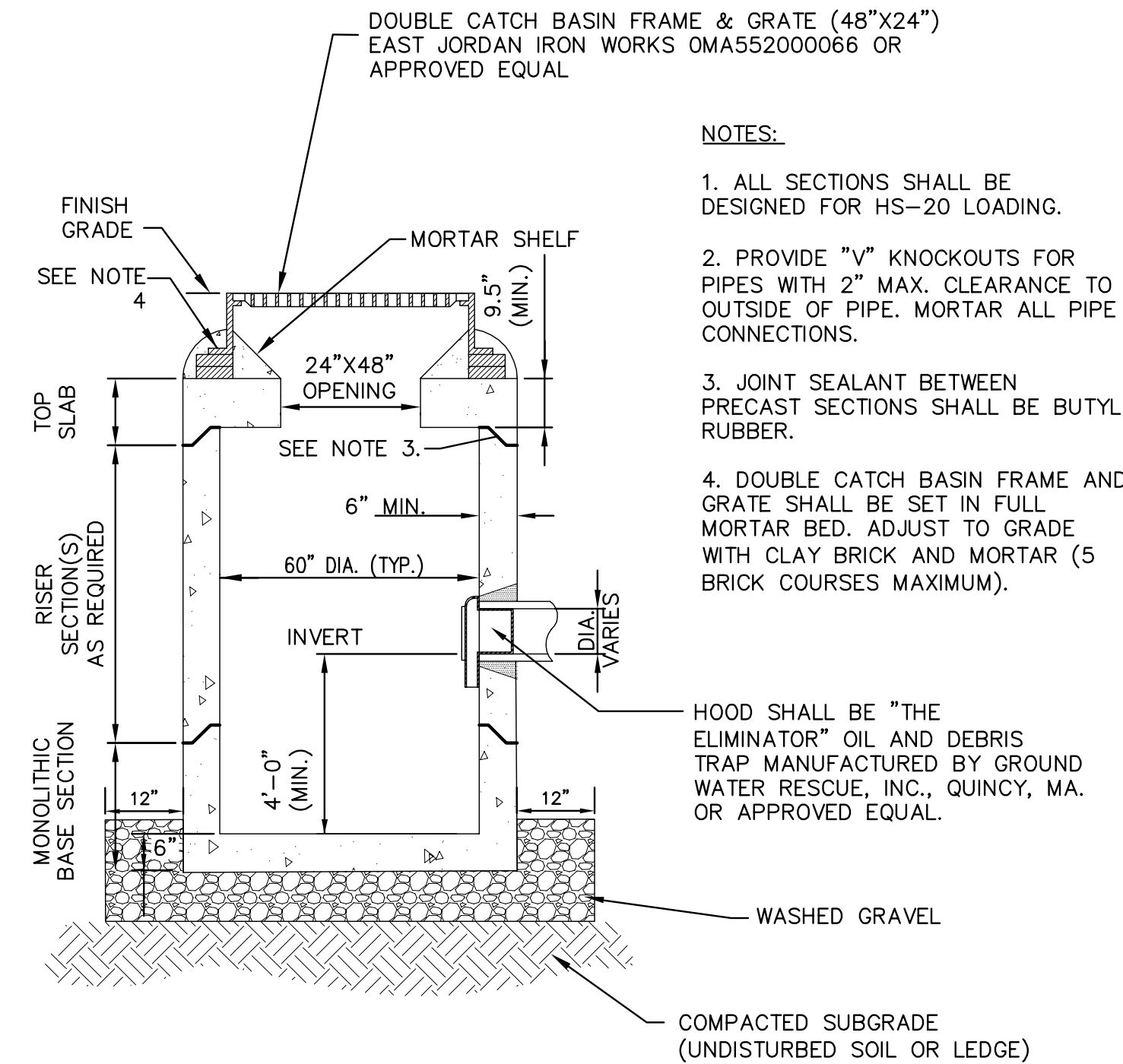
5 ADS HDPE FLARED END SECTION

SCALE: NO SCALE



6 LEVEL SPREADER/PLUNGE POOL/ENERGY DISSIPATER

SCALE: NO SCALE



NOTE:

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2. CONTRACTOR SHALL PROVIDE SIGNED & SEALED BUOYANCY CALCULATIONS FOR ALL PRECAST STRUCTURES WITH SHOP DRAWINGS SUBMITTED TO CHA. CALCULATIONS SHALL ASSUME SEASONAL HIGH GROUNDWATER ELEVATION FOR FOR GROUNDWATER DEPTH AT EACH STRUCTURE WITH A 1.5 FACTOR OF SAFETY APPLIED TO THE UPLIFT.

3 PRECAST DOUBLE GRATE CATCH BASIN

SCALE: NO SCALE

THE RESIDENCES AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190

PROJECT

JOHN M. CORCORAN & CO. LLC

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GEOTECH

KEY PLAN



STAMP

DATE: NOVEMBER 5, 2018
REVISION: BZA SUBMISSION

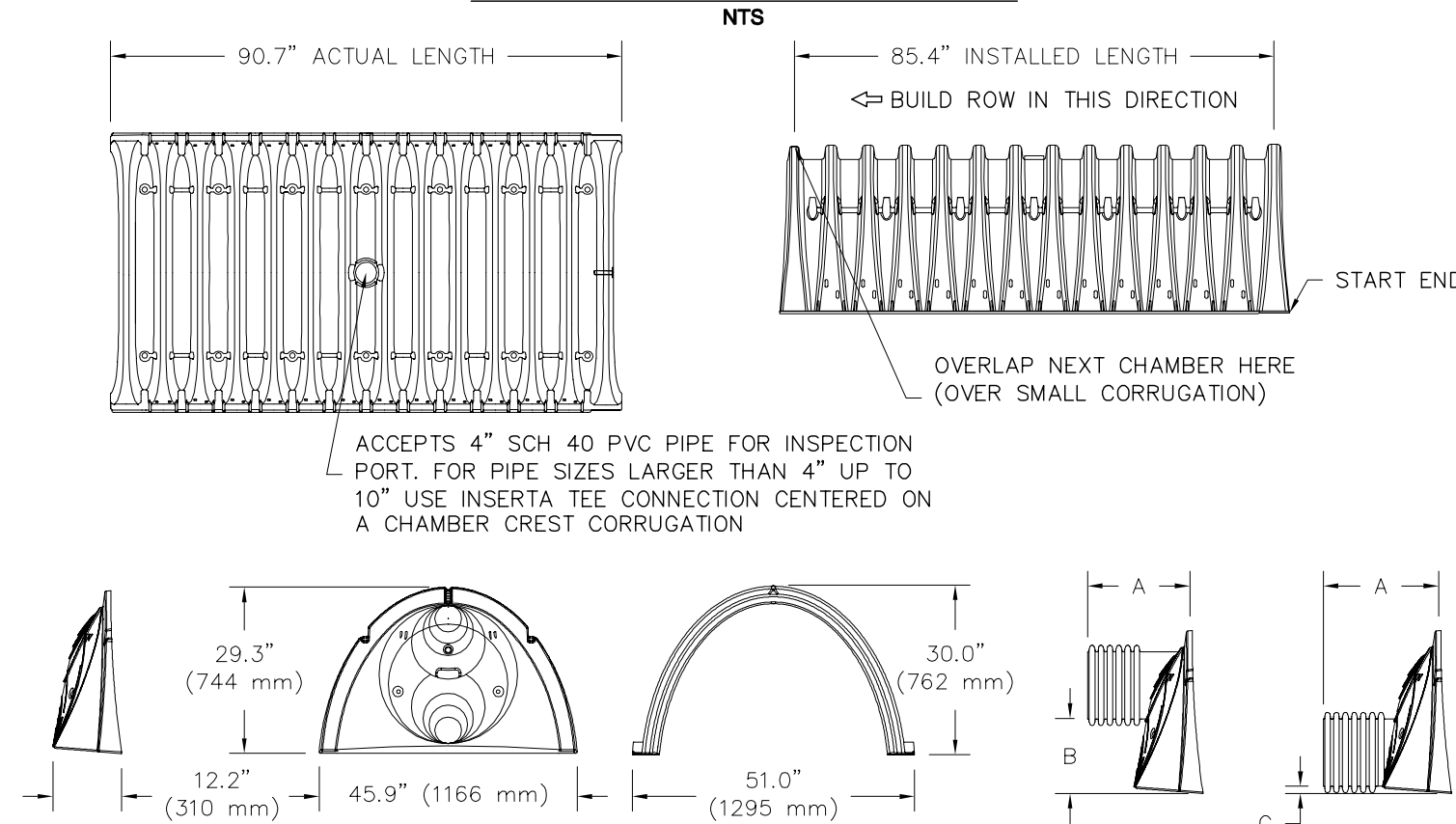
REVISIONS ON SHEET

SCALE: UTILITE PROJECT NUMBER 1839

DETAILS - 2

C-602

SC-740 TECHNICAL SPECIFICATION



NOMINAL CHAMBER SPECIFICATIONS
SIZE (W X H X INSTALLED LENGTH)
CHAMBER STORAGE
MINIMUM INSTALLED STORAGE*
WEIGHT

*ASSUMES 6" STONE ABOVE, BELOW, AND BETWEEN CHAMBERS

STUBS AT BOTTOM OF END CAP FOR PART NUMBERS ENDING WITH "B"
STUBS AT TOP OF END CAP FOR PART NUMBERS ENDING WITH "T"

PART #	STUB	A	B	C
SC740EPE06T / SC740EPE06TC	6"	10.9"	18.5" (470 mm)	---
SC740EPE08B / SC740EPE08BTC	8"	12.2"	---	0.5"
SC740EPE08T / SC740EPE08TTC	8"	12.2"	16.5"	---
SC740EPE08B / SC740EPE08BTC	8"	12.2"	---	0.6"
SC740EPE10T / SC740EPE10TTC	10"	13.4"	14.5"	---
SC740EPE10B / SC740EPE10BTC	10"	13.4"	---	0.7"
SC740EPE12T / SC740EPE12TTC	12"	14.7"	12.5"	---
SC740EPE12B / SC740EPE12BTC	12"	14.7"	---	1.2"
SC740EPE15T / SC740EPE15TTC	15"	18.4"	9.0"	---
SC740EPE15B / SC740EPE15BTC	15"	18.4"	---	1.3"
SC740EPE18T / SC740EPE18TTC	18"	19.7"	5.0"	---
SC740EPE18B / SC740EPE18BTC	18"	19.7"	---	1.6"
SC740EPE24B*	24"	18.5"	---	0.1"

ALL STUBS, EXCEPT FOR THE SC740EPE24B ARE PLACED AT BOTTOM OF END CAP SUCH THAT THE OUTSIDE DIAMETER OF THE STUB IS FLUSH WITH THE BOTTOM OF THE END CAP. FOR ADDITIONAL INFORMATION CONTACT STORMTECH AT 1-888-892-2694.

* FOR THE SC740EPE24B THE 24" STUB LIES BELOW THE BOTTOM OF THE END CAP APPROXIMATELY 1.75". BACKFILL MATERIAL SHOULD BE REMOVED FROM BELOW THE N-12 STUB SO THAT THE FITTING SITS LEVEL.

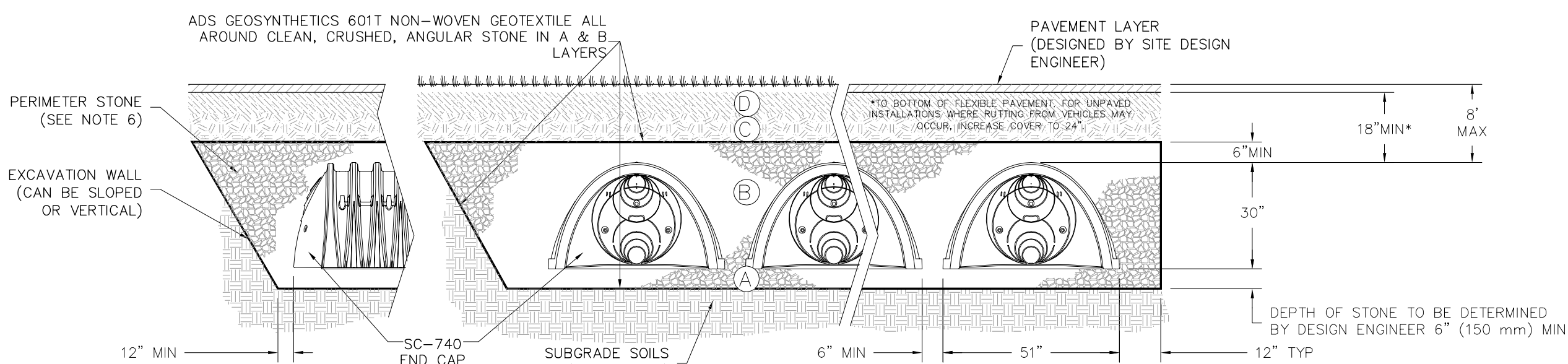
- NOTES:
- ALL DIMENSIONS ARE NOMINAL.
 - CONTRACTOR TO SUBMIT SHOP DRAWINGS FOR APPROVAL PRIOR TO CONSTRUCTION.

1 STORMTECH SC-740 CHAMBER UNIT SYSTEMS
SCALE: NO SCALE

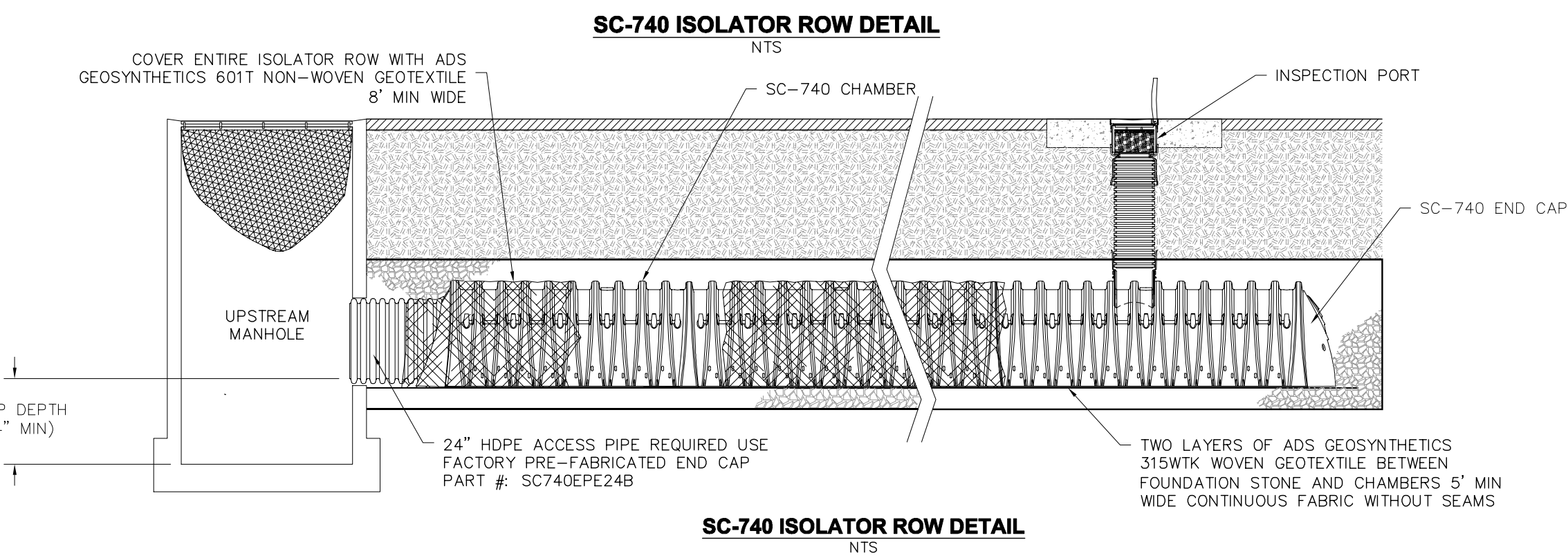
ACCEPTABLE FILL MATERIALS: STORMTECH SC-740 AND MC-3500 CHAMBER SYSTEMS

MATERIAL LOCATION	DESCRIPTION	AASHTO MATERIAL CLASSIFICATIONS	COMPACTION / DENSITY REQUIREMENT
D	FINAL FILL: FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'D' LAYER.	N/A	PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS.
C	INITIAL FILL: FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE (B LAYER) TO 18" (450 mm) ABOVE THE TOP OF THE CHAMBER. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER.	AASHTO M145* A-1, A-2-4, A-3 OR AASHTO M43* 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	BEGIN COMPACTIONS AFTER 12" (300 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 6" (150 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL-GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS. ROLLER GROSS VEHICLE WEIGHT NOT TO EXCEED 12,000 lbs (53 kN). DYNAMIC FORCE NOT TO EXCEED 20,000 lbs (89 kN).
B	EMBEDMENT STONE: FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE (A LAYER) TO THE 'C' LAYER ABOVE.	AASHTO M43* 3, 357, 4, 467, 5, 56, 57	PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE. 1 *
A	FOUNDATION STONE: FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER.	AASHTO M43* 3, 357, 4, 467, 5, 56, 57	PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE. 1 *

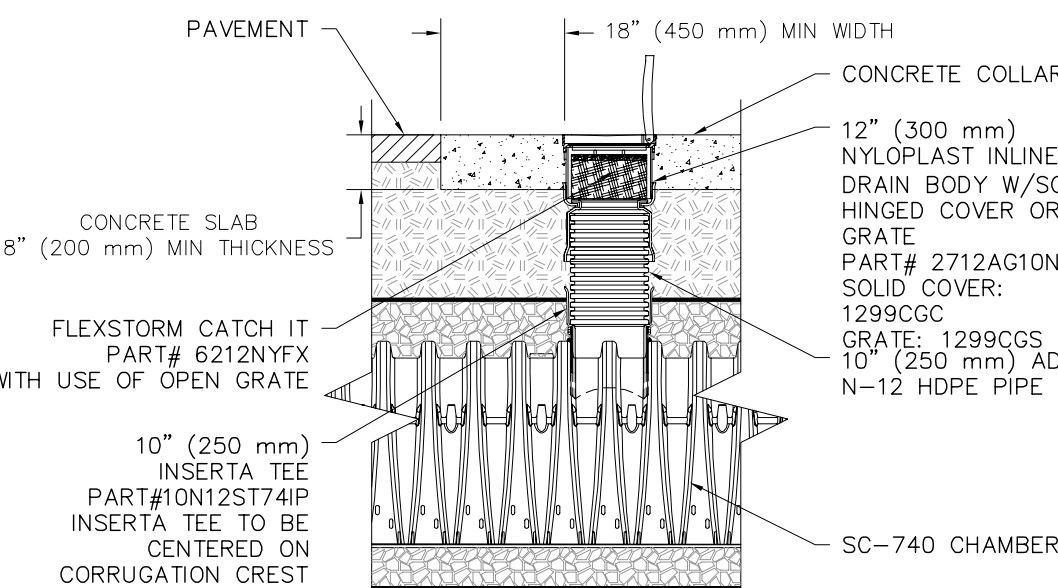
- PLEASE NOTE:
- THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".
 - STORMTECH COMPACTION REQUIREMENTS ARE MET FOR 'A' LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 6" (150 mm) (MAX) LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR.
 - WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION, FOR STANDARD DESIGN LOAD CONDITIONS, A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTION EQUIPMENT. FOR SPECIAL LOAD DESIGNS, CONTACT STORMTECH FOR COMPACTION REQUIREMENTS.



- NOTES:**
- SC-740 CHAMBERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F2418 "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS", OR ASTM F2922 "STANDARD SPECIFICATION FOR POLYETHYLENE (PE) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
 - SC-740 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
 - "ACCEPTABLE FILL MATERIALS" TABLE ABOVE PROVIDES MATERIAL LOCATIONS, DESCRIPTIONS, GRADATIONS, AND COMPACTION REQUIREMENTS FOR FOUNDATION, EMBEDMENT, AND FILL MATERIALS.
 - THE "SITE DESIGN ENGINEER" REFERS TO THE ENGINEER RESPONSIBLE FOR THE DESIGN AND LAYOUT OF THE STORMTECH CHAMBERS FOR THIS PROJECT.
 - THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS.
 - PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
 - ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEER'S DISCRETION.
 - STORMTECH SC-740 UNITS ARE H20 RATED FOR VEHICLE LOADS.



- INSPECTION & MAINTENANCE**
- STEP 1) INSPECT ISOLATOR ROW FOR SEDIMENT
- INSPECTION PORTS (IF PRESENT)
 - REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN
 - REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED
 - USING A FLASHLIGHT AND STADIA ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG
 - LOWER A CAMERA INTO ISOLATOR ROW FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL)
 - IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
 - ALL ISOLATOR ROWS
 - REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW
 - USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW THROUGH OUTLET PIPE
 - MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY
 2. FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE
 3. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- STEP 2) CLEAN OUT ISOLATOR ROW USING THE JETVAC PROCESS
- A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45° (1.1 m) OR MORE IS PREFERRED
 - APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN
 - VACUUM STRUCTURE SUMP AS REQUIRED
- STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.
- STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.



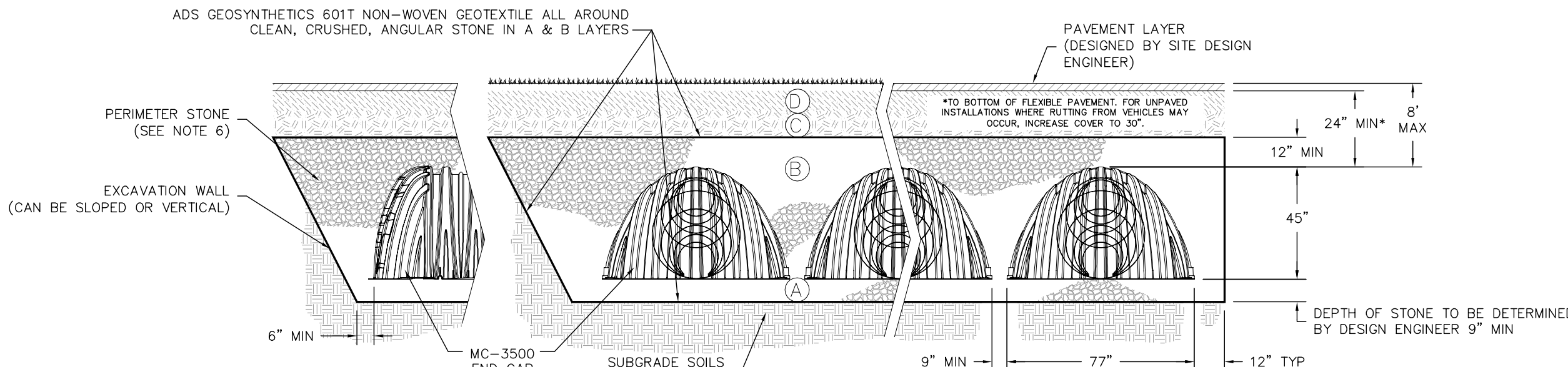
- NOTES**
- INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.
 - CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.

2 STORMTECH SC-740 CHAMBER ISOLATOR ROW
SCALE: NO SCALE

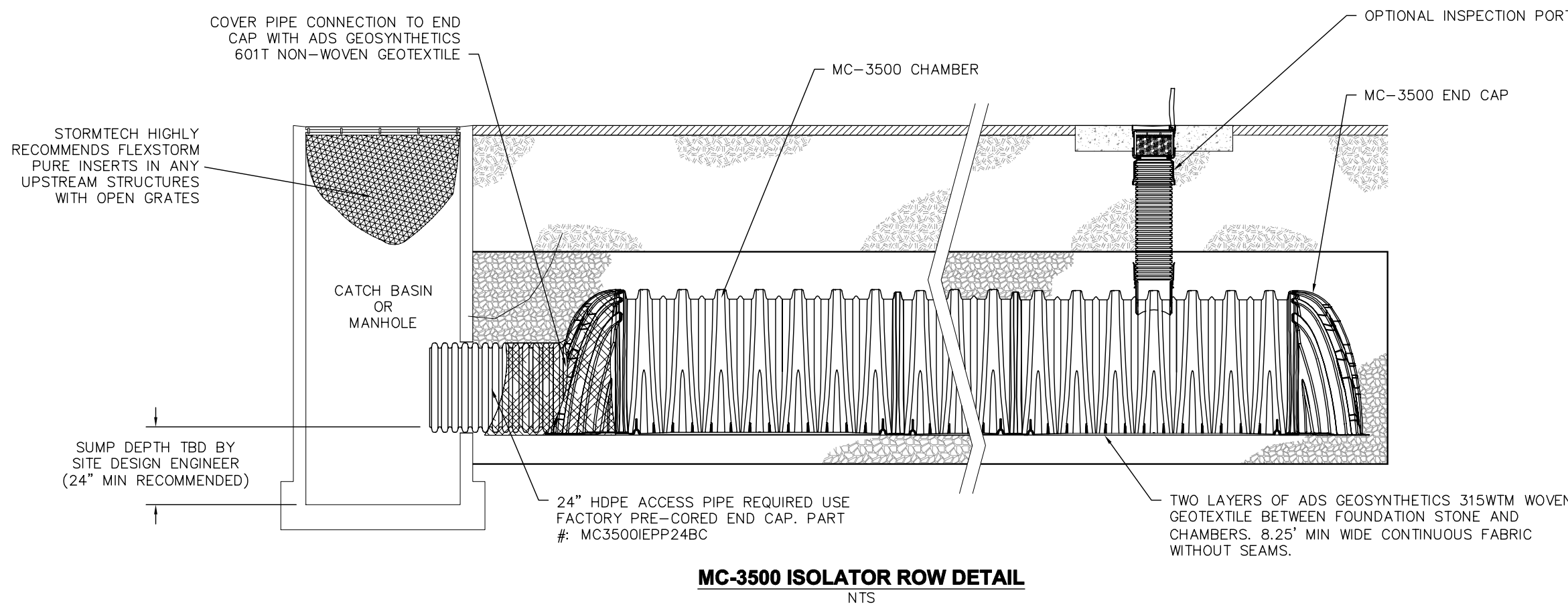
ACCEPTABLE FILL MATERIALS: STORMTECH MC-3500 CHAMBER SYSTEMS

MATERIAL LOCATION	DESCRIPTION	AASHTO MATERIAL CLASSIFICATIONS	COMPACTION / DENSITY REQUIREMENT
D	FINAL FILL: FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'D' LAYER.	N/A	PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS.
C	INITIAL FILL: FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE (B LAYER) TO 24" (600 mm) ABOVE THE TOP OF THE CHAMBER. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER.	AASHTO M145* A-1, A-2-4, A-3 OR AASHTO M43* 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	BEGIN COMPACTIONS AFTER 24" (600 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 12" (300 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL-GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS.
B	EMBEDMENT STONE: FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE (A LAYER) TO THE 'C' LAYER ABOVE.	AASHTO M43* 3, 4	NO COMPACTION REQUIRED.
A	FOUNDATION STONE: FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER.	AASHTO M43* 3, 4	PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE. 1 *

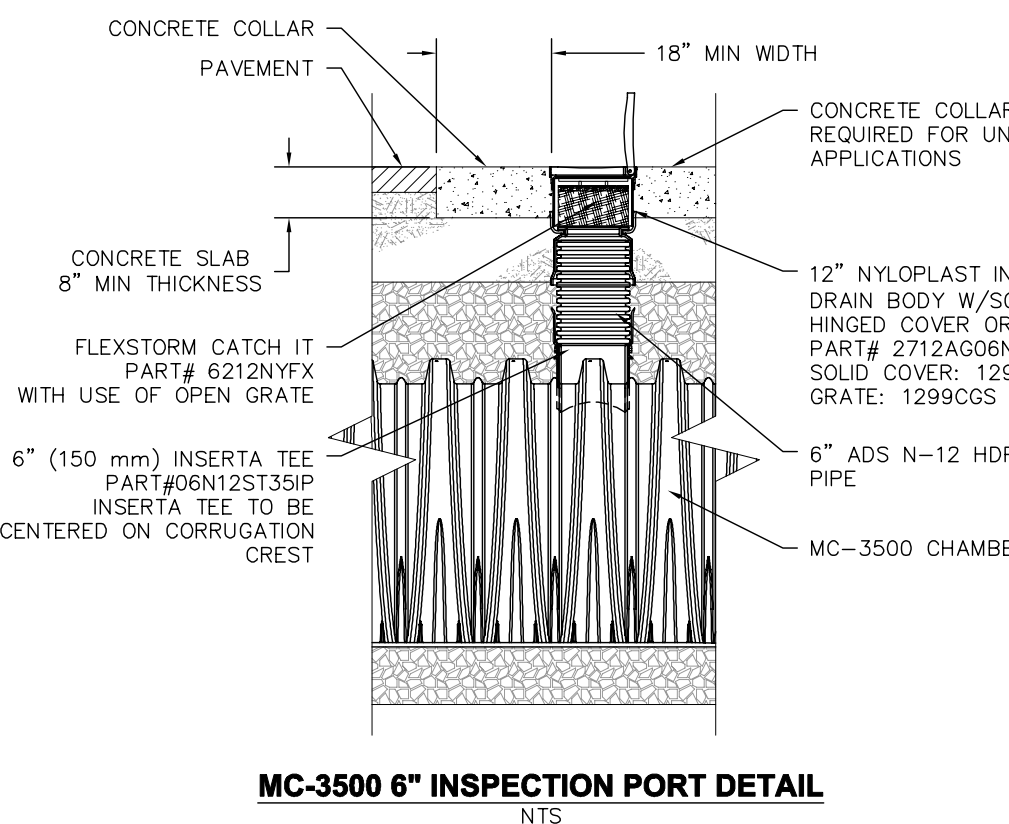
- PLEASE NOTE:
- THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".
 - STORMTECH COMPACTION REQUIREMENTS ARE MET FOR 'A' LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 9" (MAX) LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR.
 - WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION, FOR STANDARD DESIGN LOAD CONDITIONS, A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTION EQUIPMENT. FOR SPECIAL LOAD DESIGNS, CONTACT STORMTECH FOR COMPACTION REQUIREMENTS.



- NOTES:**
- MC-3500 CHAMBERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F2418 "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
 - MC-3500 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
 - "ACCEPTABLE FILL MATERIALS" TABLE ABOVE PROVIDES MATERIAL LOCATIONS, DESCRIPTIONS, GRADATIONS, AND COMPACTION REQUIREMENTS FOR FOUNDATION, EMBEDMENT, AND FILL MATERIALS.
 - THE "SITE DESIGN ENGINEER" REFERS TO THE ENGINEER RESPONSIBLE FOR THE DESIGN AND LAYOUT OF THE STORMTECH CHAMBERS FOR THIS PROJECT.
 - THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS.
 - PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
 - ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEER'S DISCRETION.

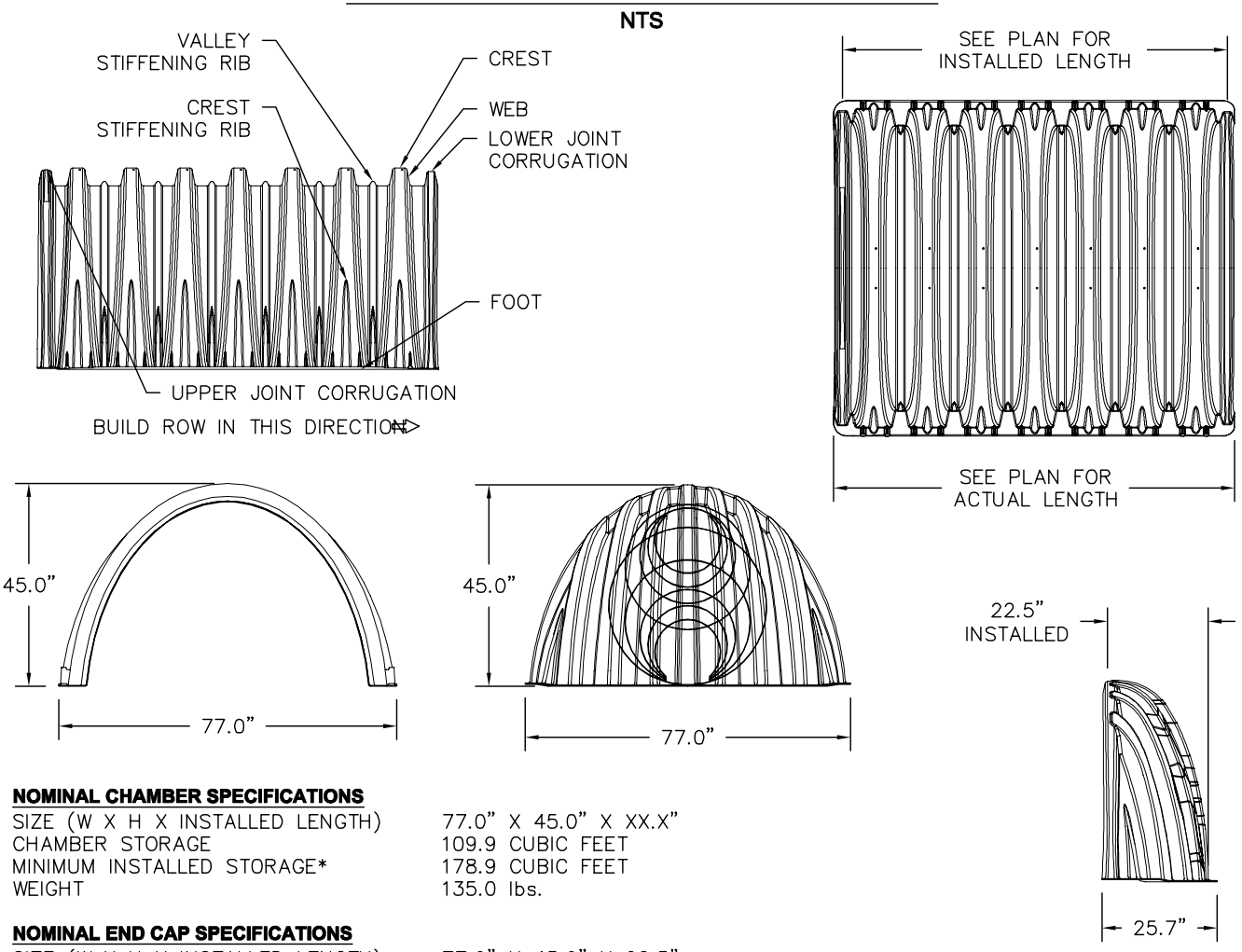


- INSPECTION & MAINTENANCE**
- STEP 1) INSPECT ISOLATOR ROW FOR SEDIMENT
- INSPECTION PORTS
 - REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN.
 - REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED.
 - USING A FLASHLIGHT AND STADIA ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG.
 - LOWER A CAMERA INTO ISOLATOR ROW FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL).
 - IF SEDIMENT IS AT, OR ABOVE, 3" PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
 - ALL ISOLATOR ROWS
 - REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW
 - USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW THROUGH OUTLET PIPE. MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY. FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE.
 - IF SEDIMENT IS AT, OR ABOVE, 3" PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- STEP 2) CLEAN OUT ISOLATOR ROW USING THE JETVAC PROCESS
- A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45° OR MORE IS PREFERRED.
 - APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN.
 - VACUUM STRUCTURE SUMP AS REQUIRED.
- STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.
- STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.



- NOTES**
- INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.
 - CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.

MC-3500 TECHNICAL SPECIFICATION



NOMINAL CHAMBER SPECIFICATIONS
SIZE (W X H X INSTALLED LENGTH)
CHAMBER STORAGE
MINIMUM INSTALLED STORAGE*
WEIGHT

NOMINAL END CAP SPECIFICATIONS
SIZE (W X H X INSTALLED LENGTH)
END CAP STORAGE
MINIMUM INSTALLED STORAGE*
WEIGHT

*ASSUMES 12" STONE ABOVE, 9" STONE FOUNDATION AND BETWEEN CHAMBERS, 12" STONE PERIMETER IN FRONT OF END CAPS AND 40% STONE POROSITY

PART #	STUB	B	C
MC3500EP06T	6"	33.21"	---
MC3500EPP06B	6"	---	0.66"
MC3500EPP08T	8"	31.16"	---
MC3500EPP08B	8"	---	0.81"
MC3500EPP10T	10"	29.04"	---
MC3500EPP10B	10"	---	0.93"
MC3500EPP12T	12"	26.36"	---
MC3500EPP12B	12"	---	1.35"
MC3500EPP15T	15"	23.39"	---
MC3500EPP15B	15"	---	1.50"
MC3500EPP18T	18"	20.03"	---
MC3500EPP18B	18"	---	1.77"
MC3500EPP24T	24"	14.48"	---
MC3500EPP24B	24"	---	2.06"
MC3500EPP30B	30"	---	---

NOTE: ALL DIMENSIONS ARE NOMINAL

CUSTOM PRECURED INVERTS ARE AVAILABLE UPON REQUEST. INVENTORIED MANIFOLDS INCLUDE 12-24" SIZE ON SIZE AND 15-48" ECCENTRIC MANIFOLDS. CUSTOM INVERT LOCATIONS ON THE MC-3500 END CAP CUT IN THE FIELD ARE NOT RECOMMENDED FOR PIPE SIZES GREATER THAN 10". THE INVERT LOCATION IN COLUMN 'B' ARE THE HIGHEST POSSIBLE FOR THE PIPE SIZE.

3 STORMTECH MC-3500 CHAMBER UNIT SYSTEMS
SCALE: NO SCALE

4 STORMTECH MC-3500 CHAMBER ISOLATOR ROW
SCALE: NO SCALE

THE RESIDENCES AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190
PROJECT
JOHN M. CORCORAN & CO. LLC
100 GRANDVIEW ROAD, SUITE 203
BRAINTREE, MA 02184
P 781 849.0011
OWNER

utile

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BOSTON, MA 02111
P 617 423.7200 F 617 423.1414
utilidesign.com
ARCHITECT

CHA COMPANIES

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NORWELL, MA 02061
P 781 982.5400
CIVIL & LANDSCAPE

BLW ENGINEERS

311 GREAT ROAD
LITTLETON, MA 01460
P 978 486.4301
MEPPF

VEITAS AND VEITAS

639 GRANITE ST
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P 781 849.2065
STRUCTURAL

VANASSE & ASSOCIATES

35 NEW ENGLAND BUS CENTER DR.
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P 978 474.8800
TRAFFIC

McPHAIL ASSOCIATES

2269 MASSACHUSETTS AVE.
CAMBRIDGE, MA 02140
P 617 868.1420
GEOTECH

KEY PLAN

REVISIONS ON SHEET

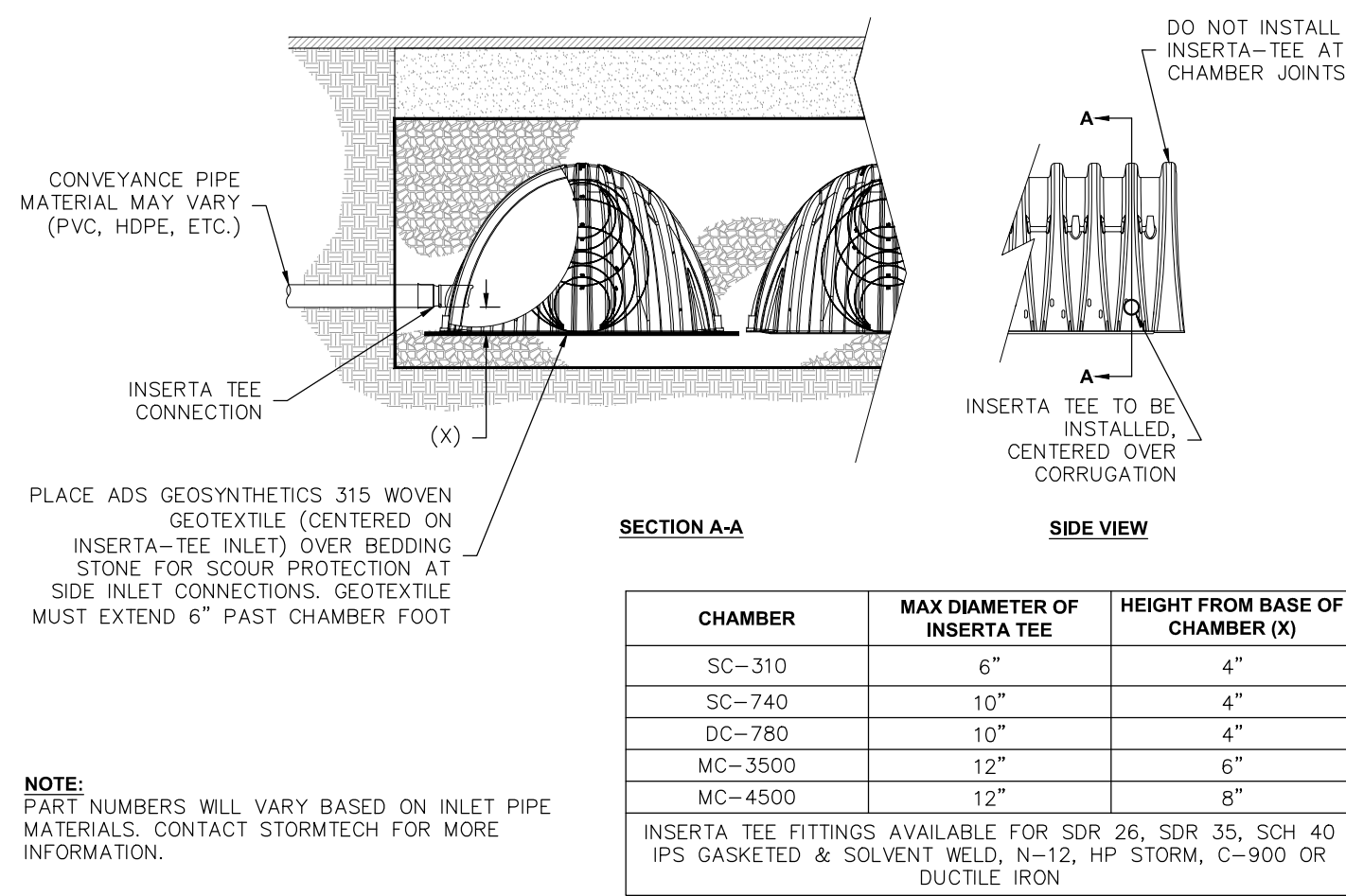
DATE	REVISION
NOVEMBER 5, 2018	BZA SUBMISSION

STAMP

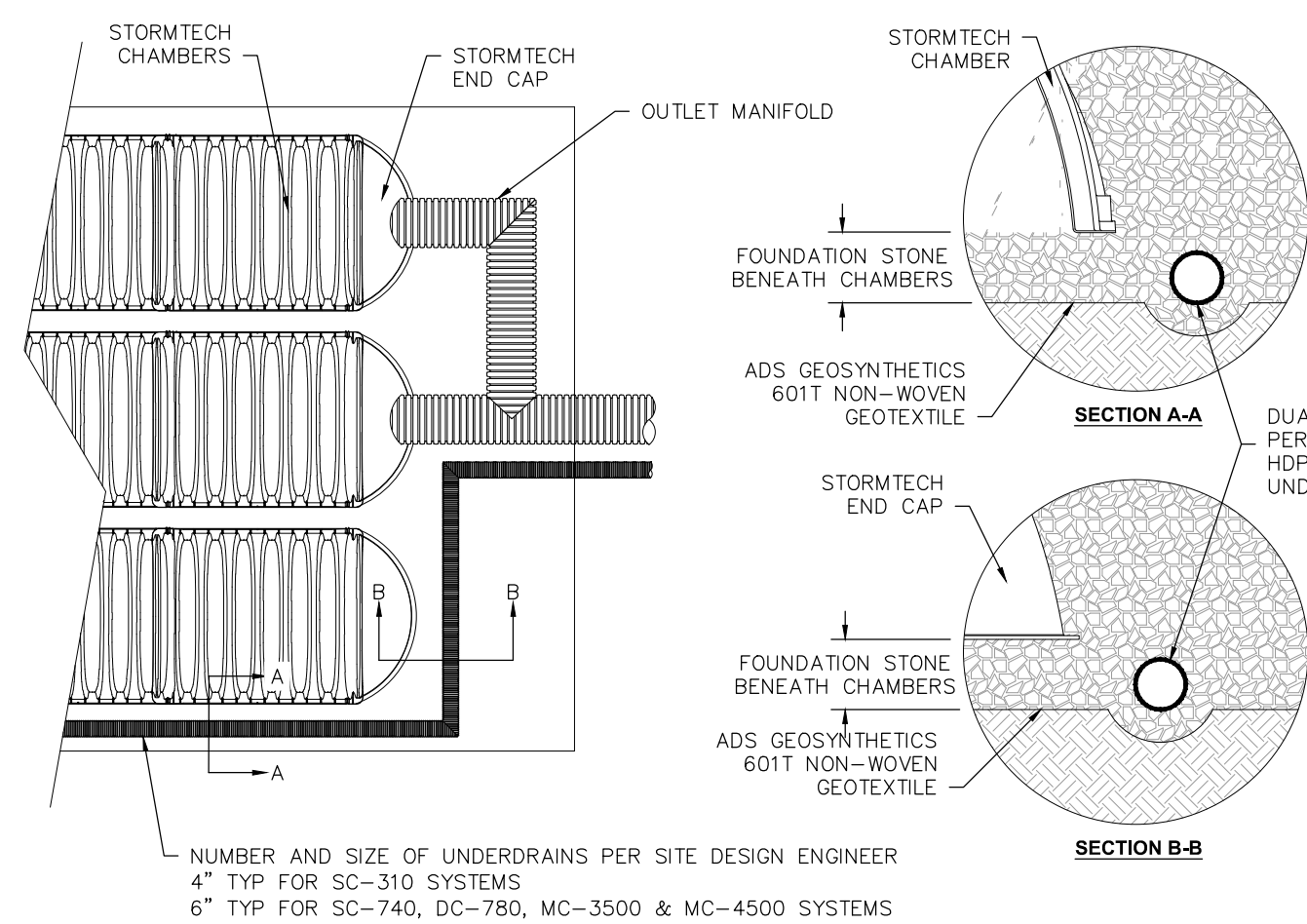
SCALE UTILITE PROJECT NUMBER 1839

DETAILS - 3

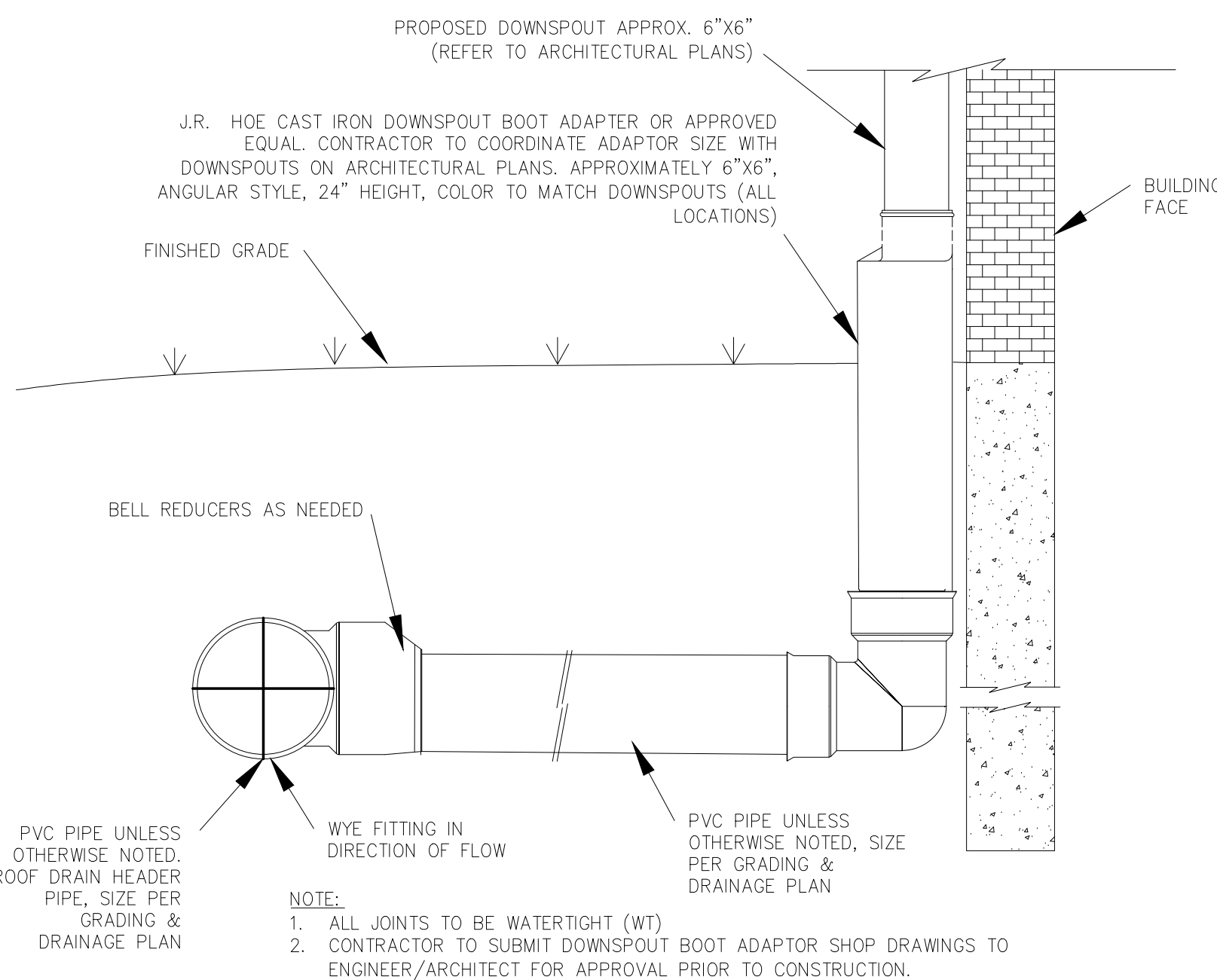
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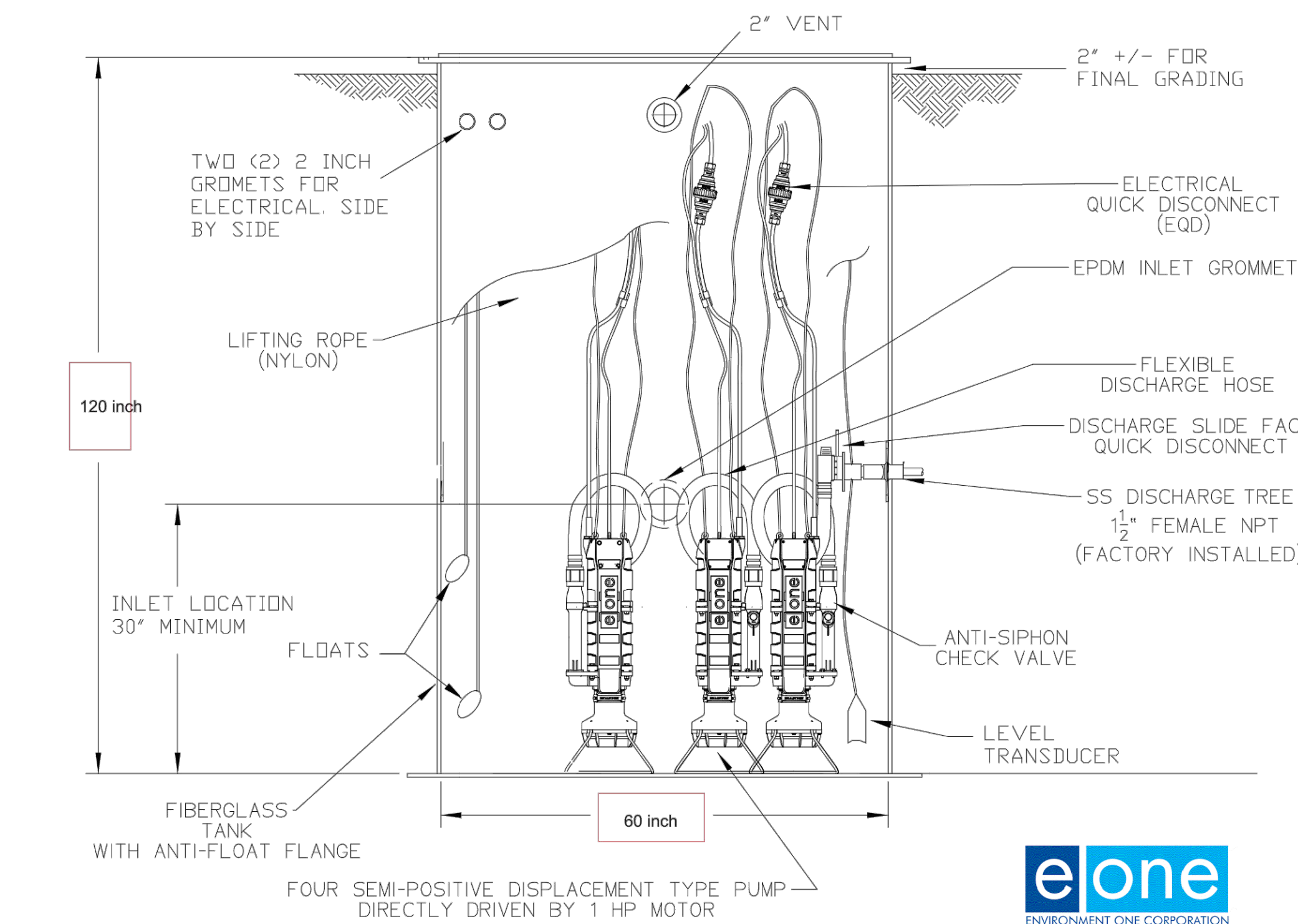
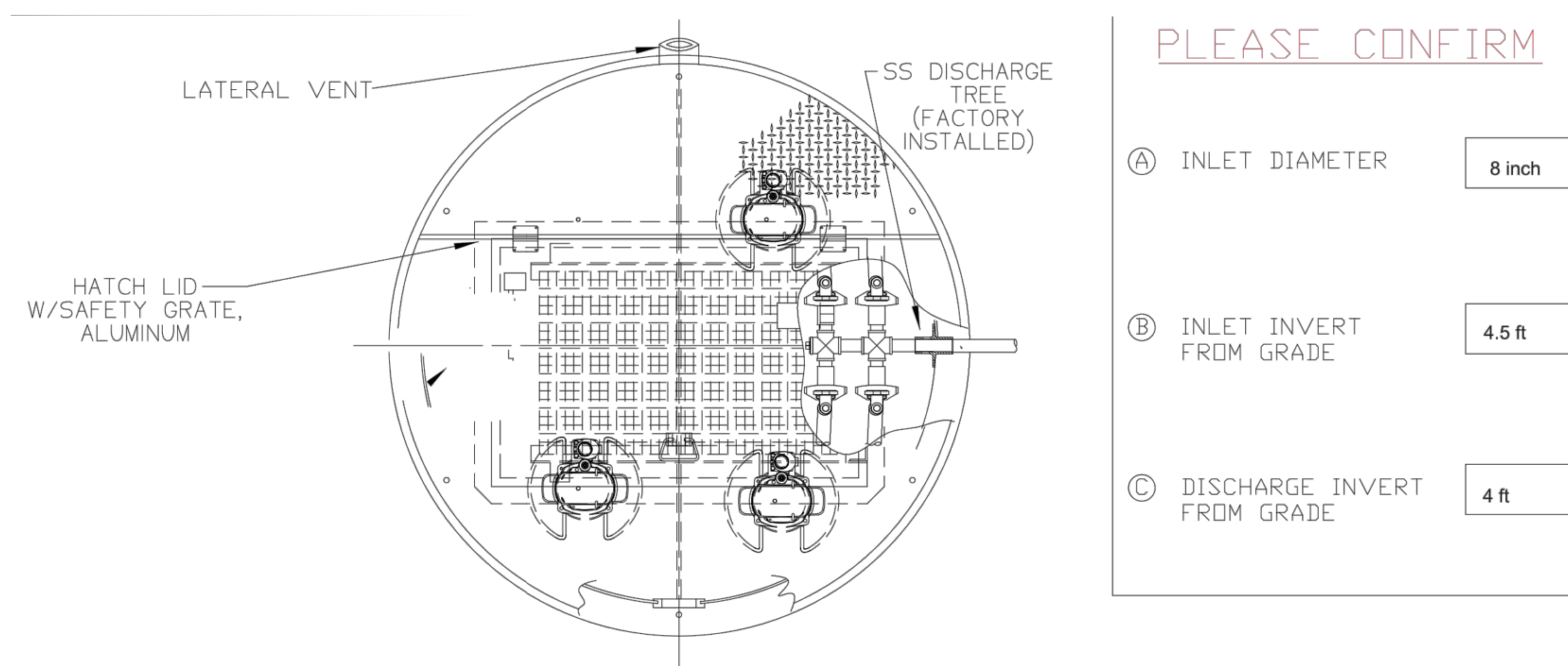
3 STORMTECH INSERTA TEE DETAIL
SCALE: NO SCALE



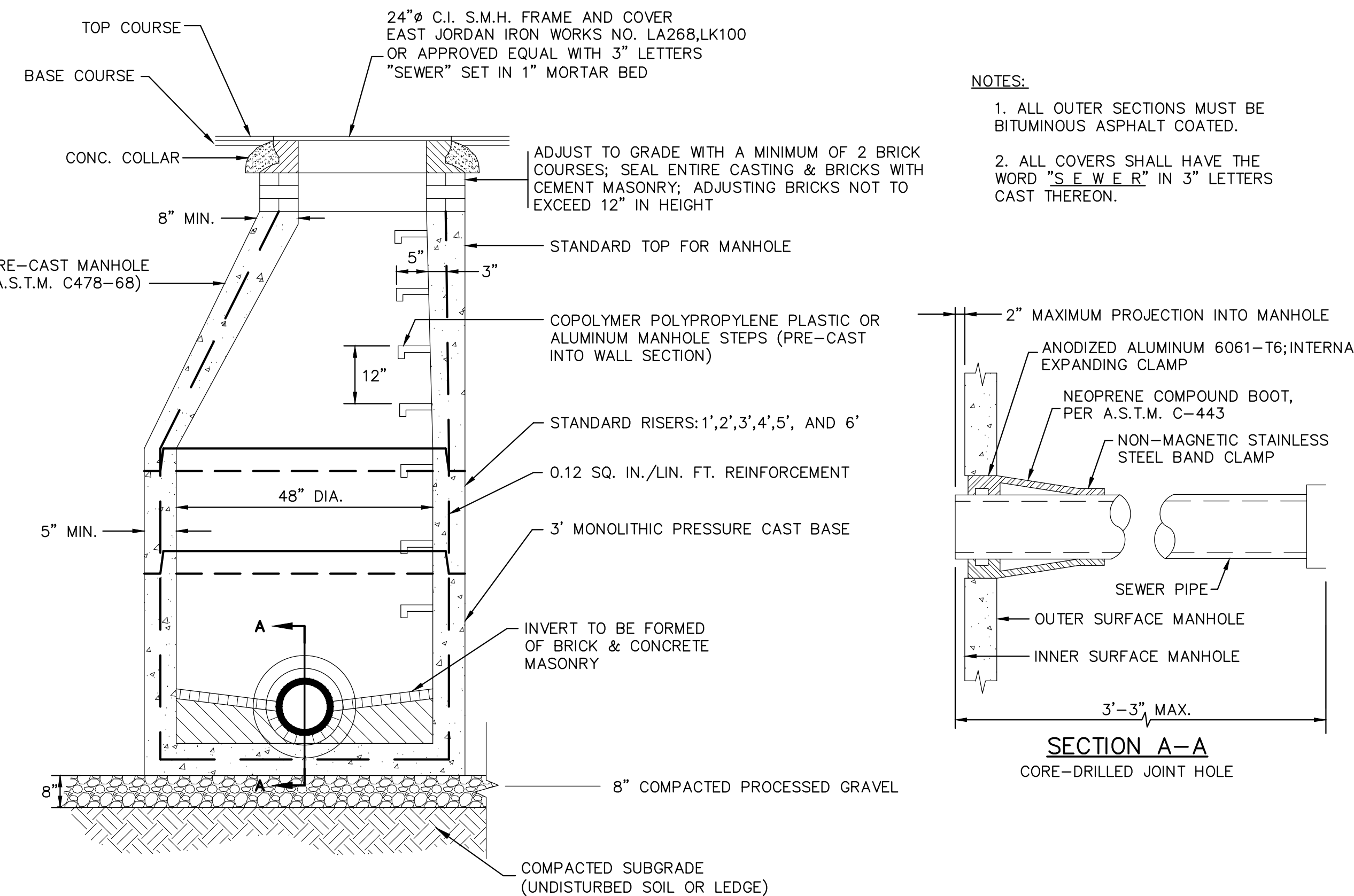
4 STORMTECH UNDERDRAIN DETAIL
SCALE: NO SCALE



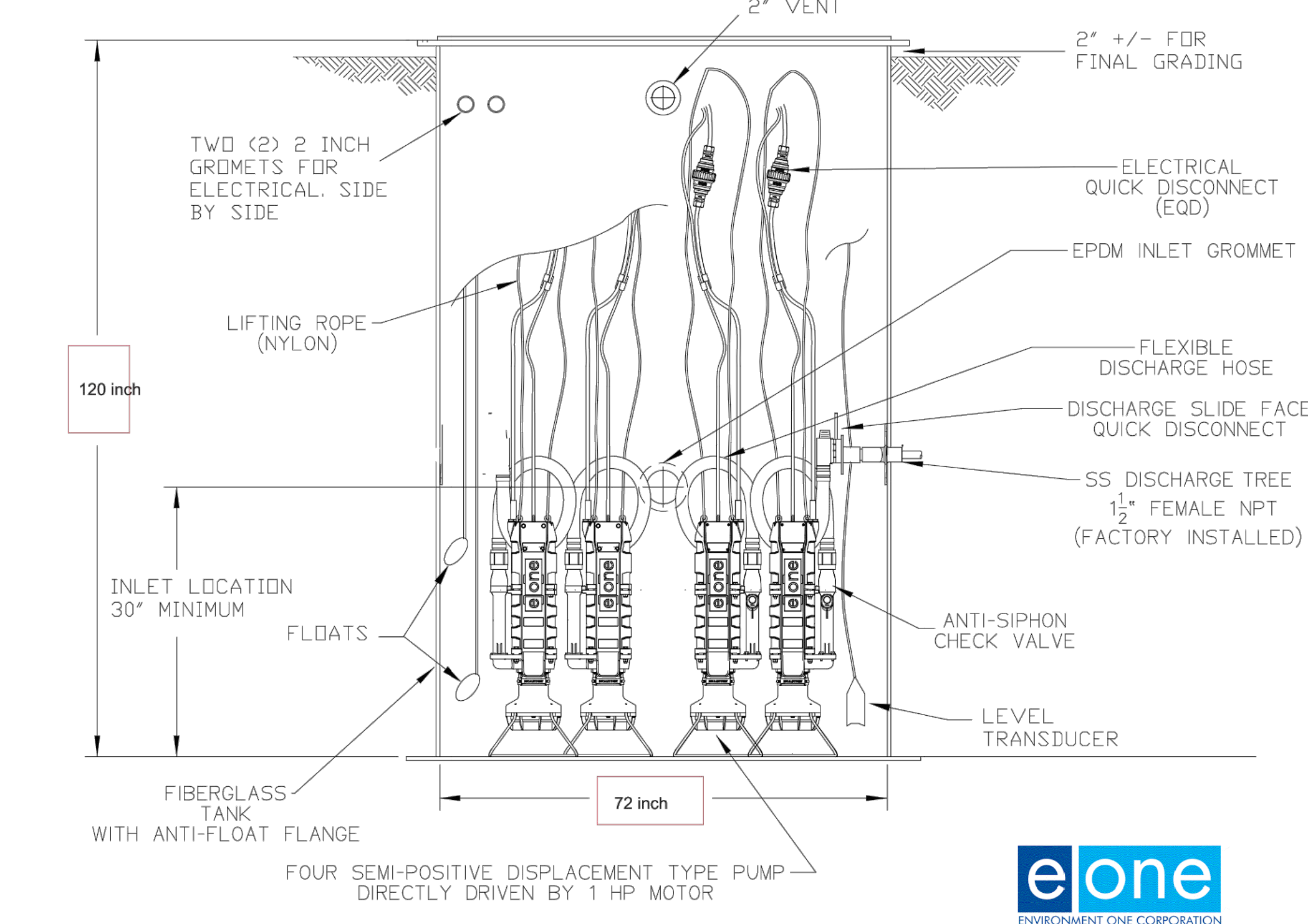
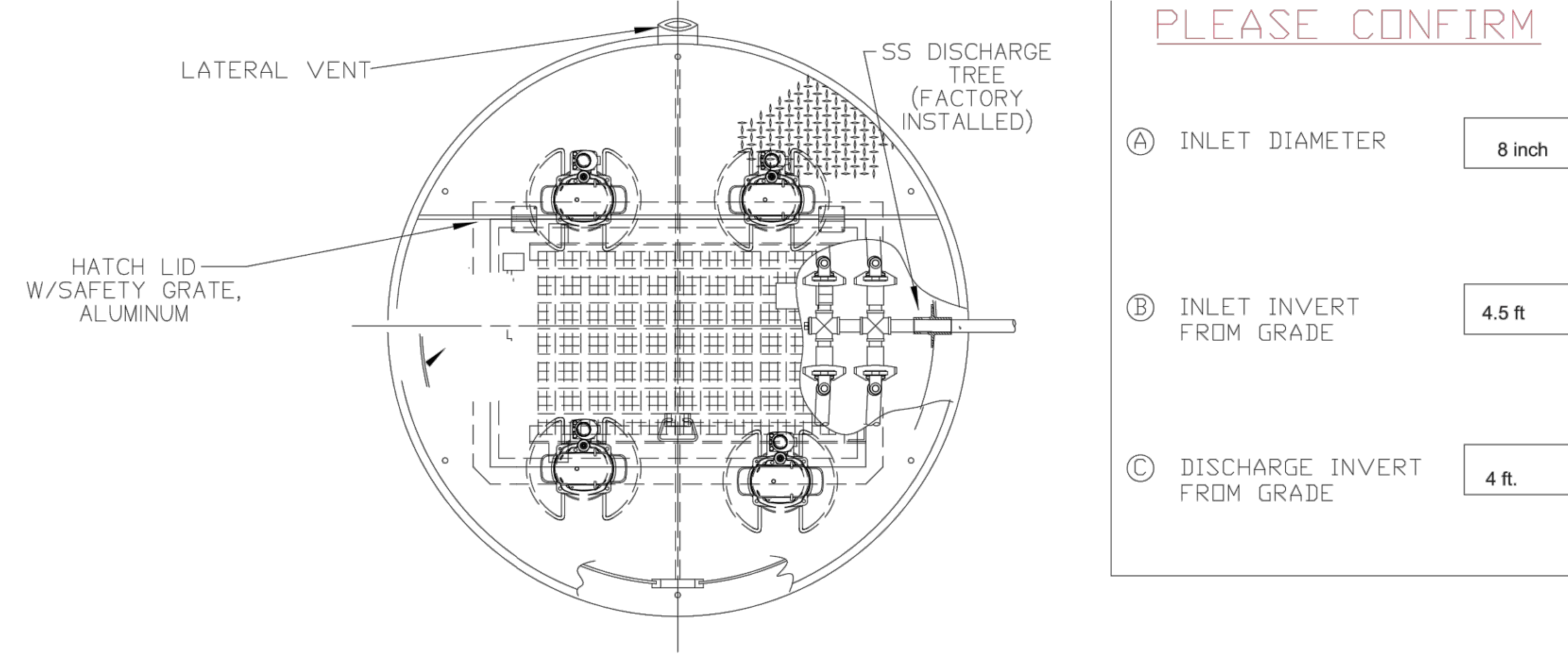
11 ROOF DRAIN CONNECTION
SCALE: NO SCALE



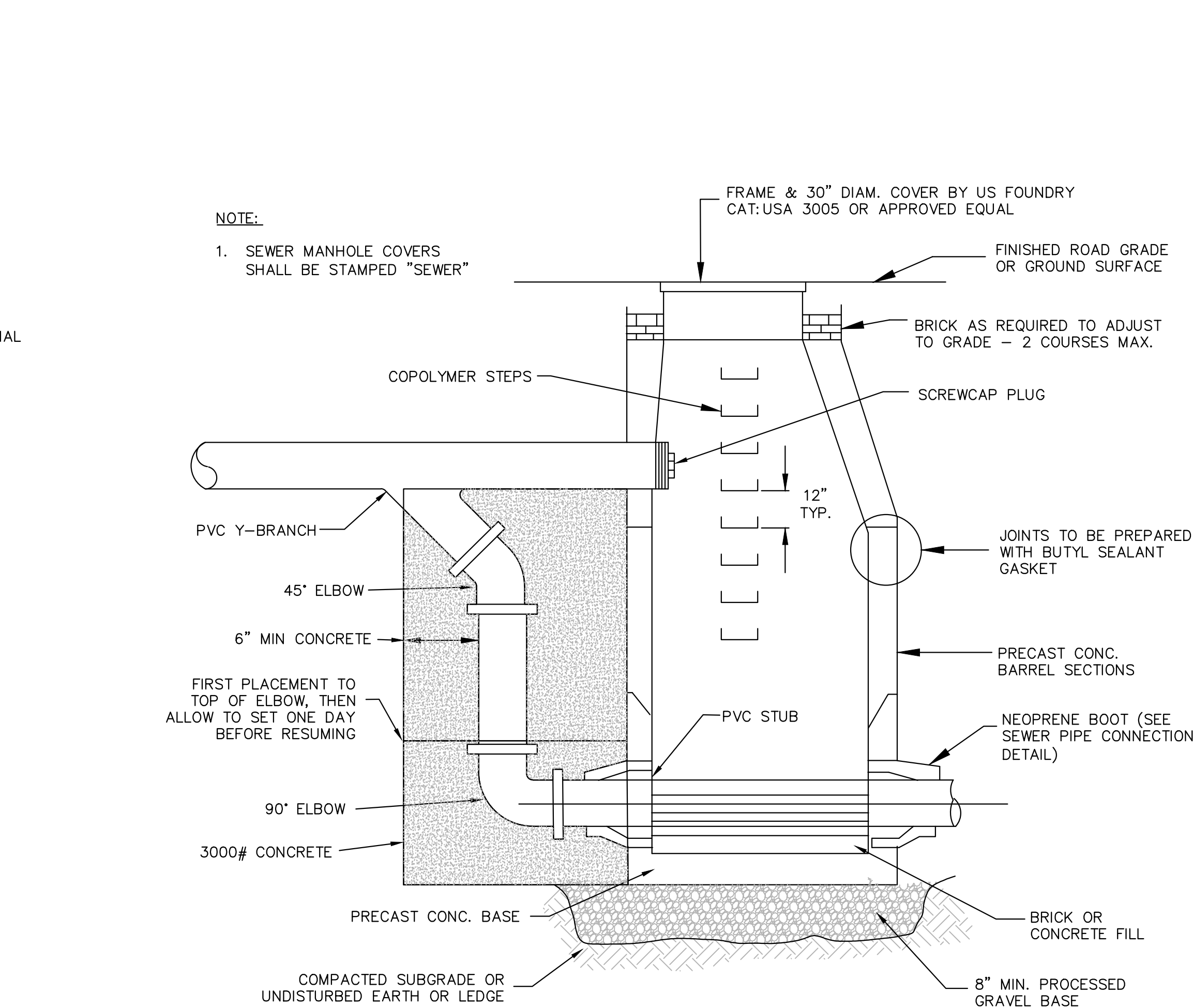
3 E-ONE® TRIPLEX FIBERGLASS GRINDER PUMP
SCALE: NO SCALE



5 STANDARD PRE-CAST SEWER MANHOLE
SCALE: NO SCALE



4 E-ONE® QUADRIPLEX FIBERGLASS GRINDER PUMP
SCALE: NO SCALE



6 DROP SEWER CONNECTION DETAIL
SCALE: NO SCALE

THE RESIDENCES AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190

PROJECT
JOHN M. CORCORAN & CO. LLC

100 GRANDVIEW ROAD, SUITE 203
BRAINTREE, MA 02184
P 781.849.0011

OWNER

utile

ARCHITECTURE + URBAN DESIGN

115 KINGSTON ST
BOSTON, MA 02111
P 617.423.7200 F 617.423.1414
utiledesign.com

ARCHITECT

CHA COMPANIES

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NORWELL, MA 02061
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CIVIL & LANDSCAPE

BLW ENGINEERS

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MEFPF

VEITAS AND VEITAS

639 GRANITE ST
BRAINTREE, MA 01814
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STRUCTURAL

VANASSE & ASSOCIATES

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P 978.474.8800

TRAFFIC

McPHAIL ASSOCIATES

2269 MASSACHUSETTS AVE.
CAMBRIDGE, MA 02140
P 617.868.1420

GEOTECH

KEY PLAN



STAMP

DATE REVISION
NOVEMBER 5, 2018 B2A SUBMISSION

REVISIONS ON SHEET

SCALE UTILE PROJECT NUMBER
1839

DETAILS - 4

C-604

1500 MAIN STREET
WEYMOUTH, MA 02190

PROJECT

**JOHN M. CORCORAN &
CO. LLC**

100 GRANDVIEW ROAD, SUITE 203
BRAINTREE, MA 02184
P 781.849.0011

OWNER

ARCHITECTURE + URBAN DESIGN

ARCHITECT

141 LONGWATER DRIVE, SUITE 104
NORWELL, MA 02061
P 781 982.5400

CIVIL & LANDSCAPE

MEPFR

CTURAL

TRAFFIC

EOTECH

Y PLAN



REVISIONS ON SHEET

DETAILS - 5

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NOTES:

- ## 1 GREASE TRAP DETAIL

SCALE: NO SCALE



SCALE: NO SCALE

- NOTES:

- 6" SECTION B-B 6"
OPPOSITE WALLS ARE MIRROR IMAGES



OPPOSITE WALLS ARE MIRROR IMAGES.

SCALE: NO SCALE



BASIN TO BE LOCATED OUTSIDE OF BUILDING
WHERE POSSIBLE,COVER TO HAVE A CENTER HOLE.

A TIGHT COVER MUST BE USED IF BASIN IS LOCATED INSIDE OF BUILDING.

OPENING SHALL BE NOT LESS THAN 24" DIA.

THE CATCH BASIN SHALL BE SO LOCATED AND
CONSTRUCTED THAT SURFACE WATER SHALL BE EXCLUDED.

INLET PIPE SHALL BE AT LEAST FOUR
INCHES ABOVE NORMAL WATER LINE.

WHERE SUBJECT TO FROST OR CRUSHING CONDITIONS,
OUTLET SHALL BE AT LEAST THREE FEET BELOW THE
SURFACE.

THE NEW CATCH BASIN MUST BE FILLED WITH CLEAN WATER BEFORE USING, AND AFTER BEING EMPTIED FOR PERIODIC CLEANING.

ALL OIL AND GASOLINE MUST BE REMOVED BEFORE
CLEANING OUT THE BASIN, AND MUST NOT BE
DISCHARGED INTO THE SEWER THROUGH OTHER FIXTURES.

SPECIFICATIONS FOR COVERING SPECIAL CASES OR CONDITIONS, SHALL BE APPROVED BY THE LOCAL AUTHORITIES, AND THE AUTHORITIES OF THE M.W.R.A.

WROUGHT IRON STEPS SHALL BE SPACED ABOUT 18" APART.

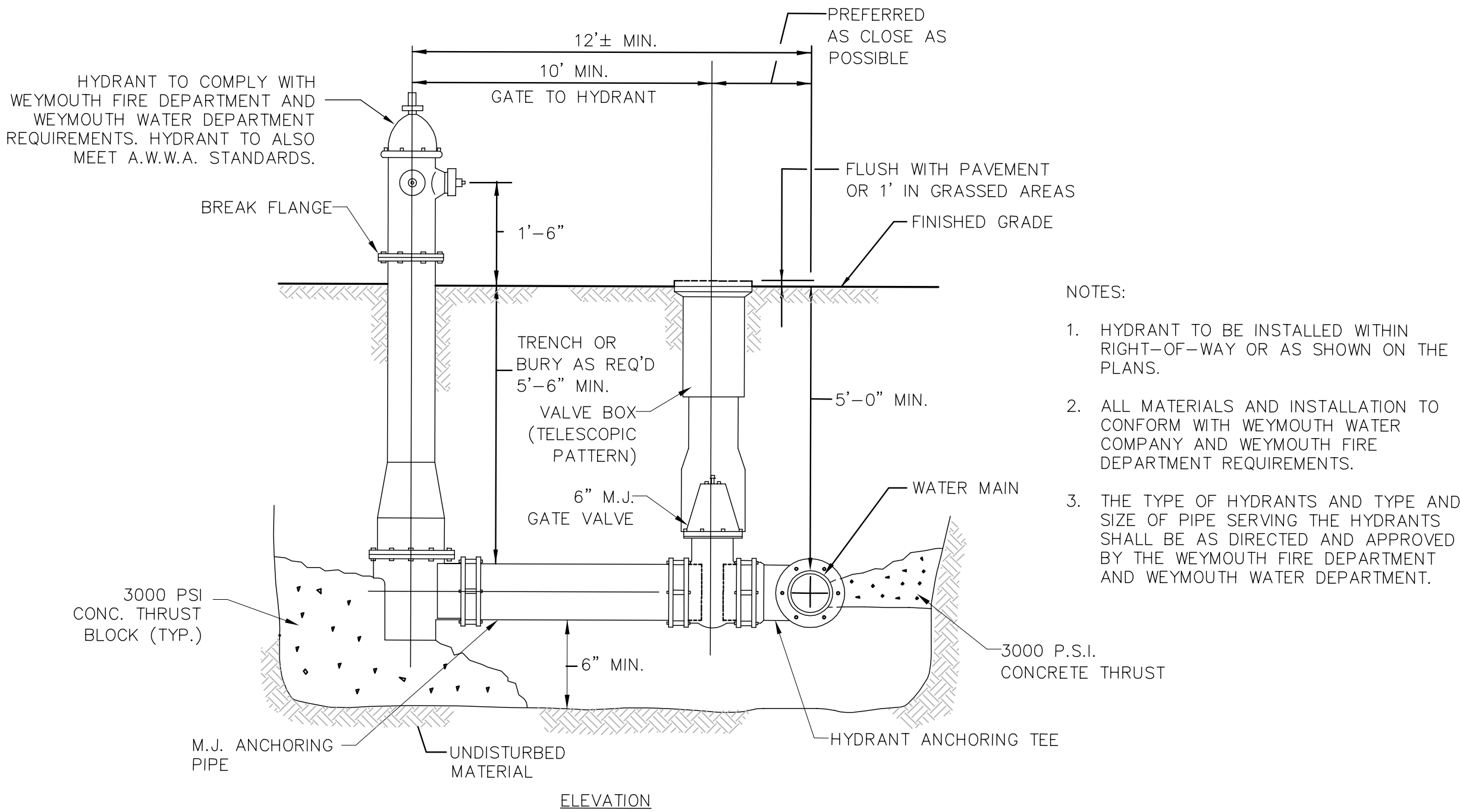
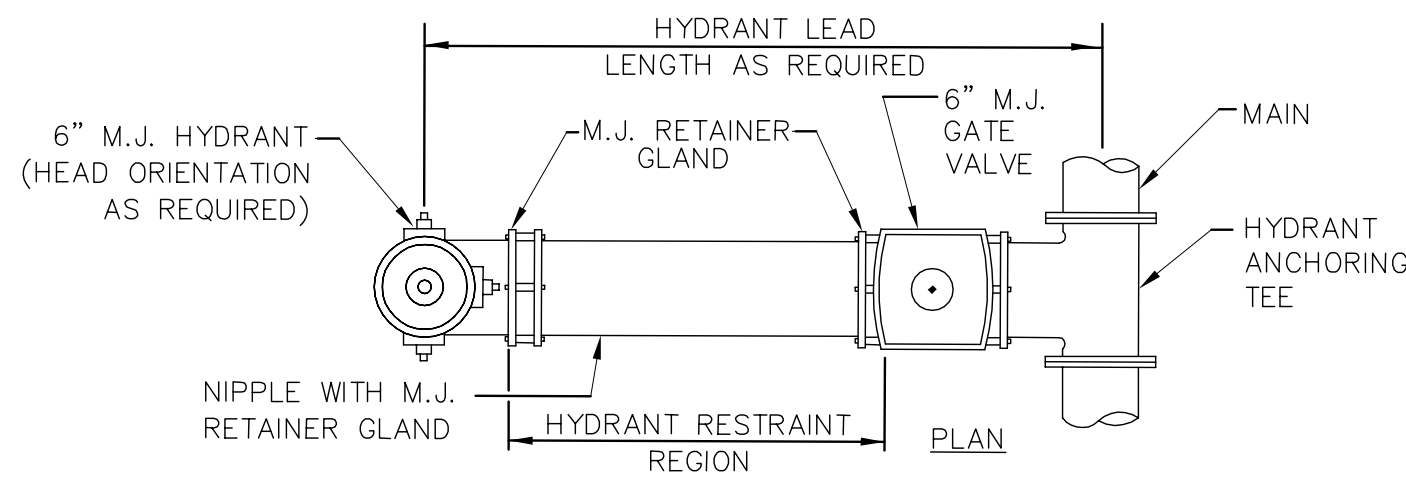
BOTH VENTS SHALL BE EXTENDED INDEPENDENTLY 18" ABOVE THE ROOF, OR AS APPROVED BY THE LOCAL AUTHORITIES. AND THE AUTHORITIES OF THE M.W.R.A.

OUTLET PIPE TO BE 45 DEGREE ANGLE.



3	3 WAY DISTRIBUTION ELECTRIC MANHOLE DETAIL
---	--

SCALE: NO SCALE

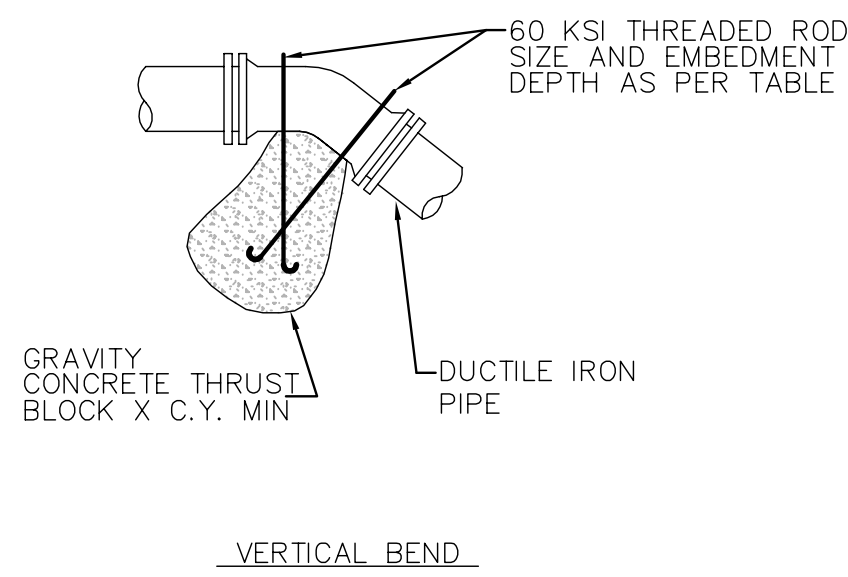


1 TYPICAL HYDRANT & VALVE ASSEMBLY DETAIL
SCALE: NO SCALE

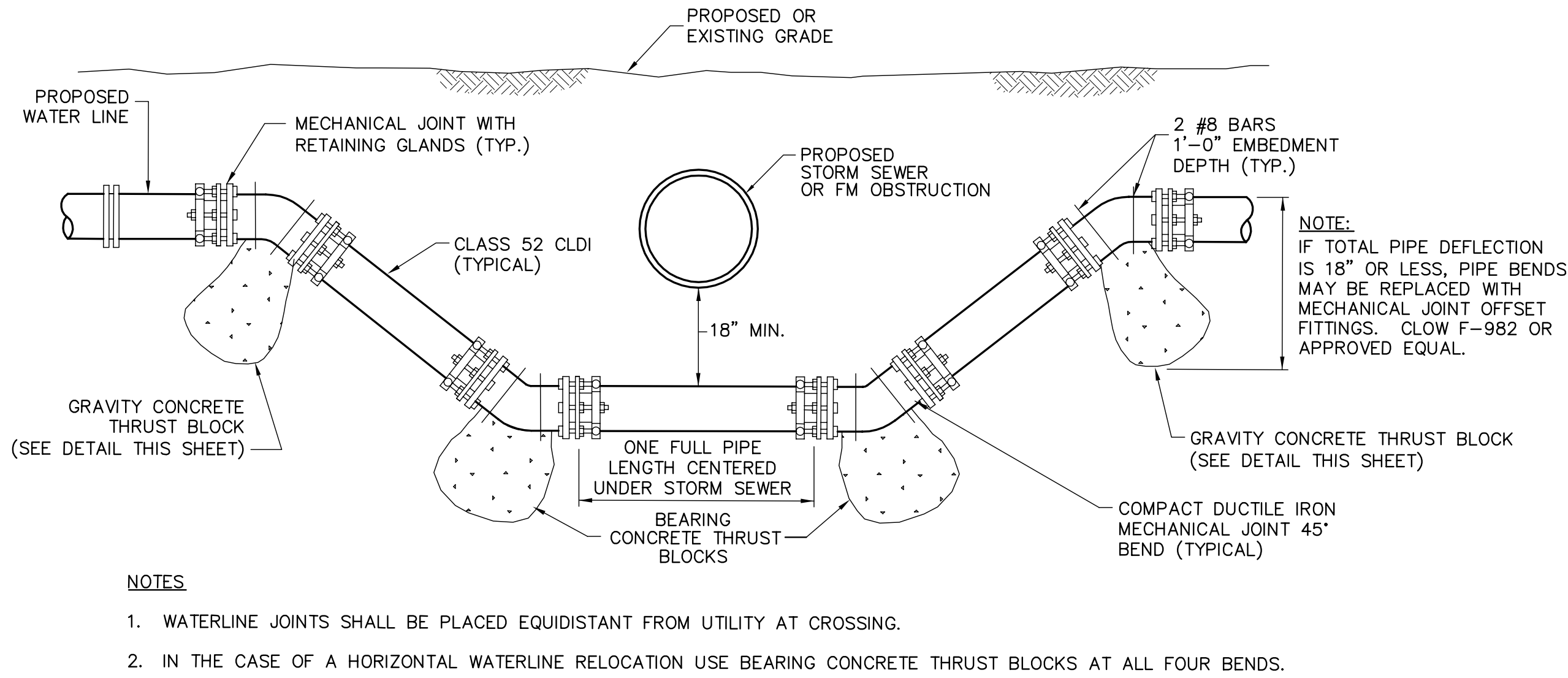
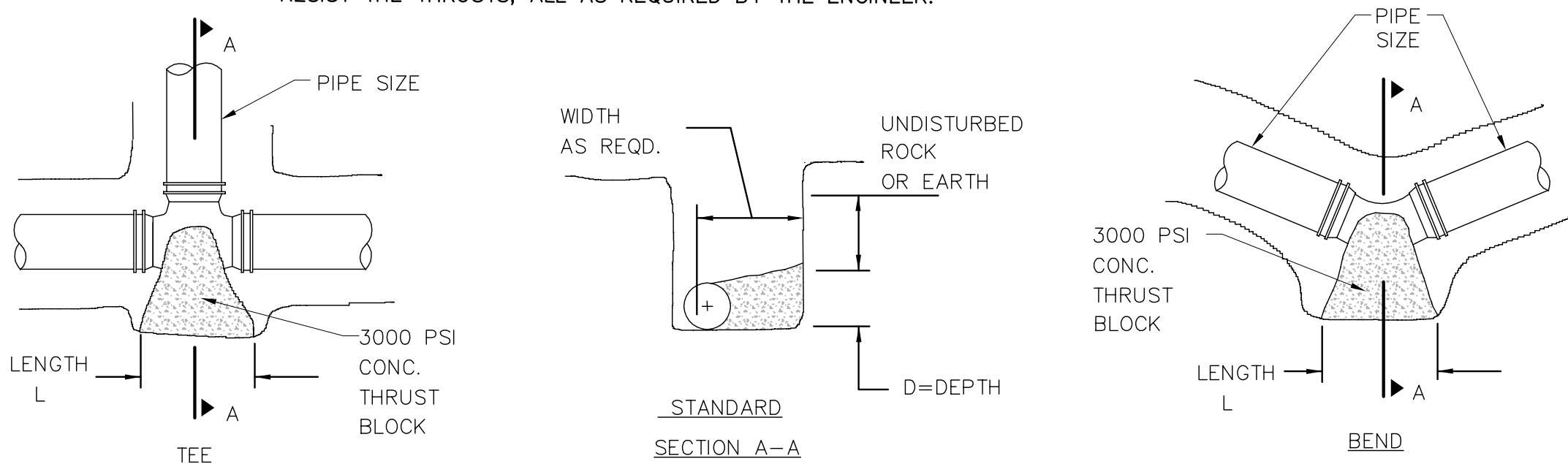
REQUIRED BEARING AREAS & DIMENSIONS										
FOR CONCRETE THRUST BLOCKS										
PIPE SIZE (IN.)	TEE(See Note 5)		90°(1/4)BEND		45°(1/8)BEND		22-1/2°(1/16)BEND		11-1/4°(1/32)BEND	
	Sq.Ft. AREA	Dimen. D x L	Sq.Ft. AREA	Dimen. D x L	Sq.Ft. AREA	Dimen. D x L	Sq.Ft. AREA	Dimen. D x L	Sq.Ft. AREA	Dimen. D x L
3 & 4	1.4	1.0 x 1.5	2.0	1.0 x 2.0	1.1	1.0 x 1.5	0.6	0.5 x 1.5	0.3	0.5 x 1.0
6	3.2	1.5 x 2.5	4.5	2.0 x 2.5	2.4	1.5 x 2.0	1.2	1.0 x 1.5	0.6	1.5 x 1.5
8	5.7	2.0 x 3.0	8.0	2.0 x 4.0	4.3	2.0 x 2.5	2.2	1.5 x 1.5	1.1	1.0 x 1.5
12	12.7	3.5 x 3.5	18.0	4.0 x 4.5	9.7	2.5 x 4.0	5.0	2.0 x 2.5	2.5	1.5 x 2.0
16	50.0	6.0 x 8.5	50.0	6.0 x 8.5	27.0	5.0 x 5.5	13.8	3.5 x 4.0	6.9	2.5 x 3.0

THRUST BLOCK NOTES

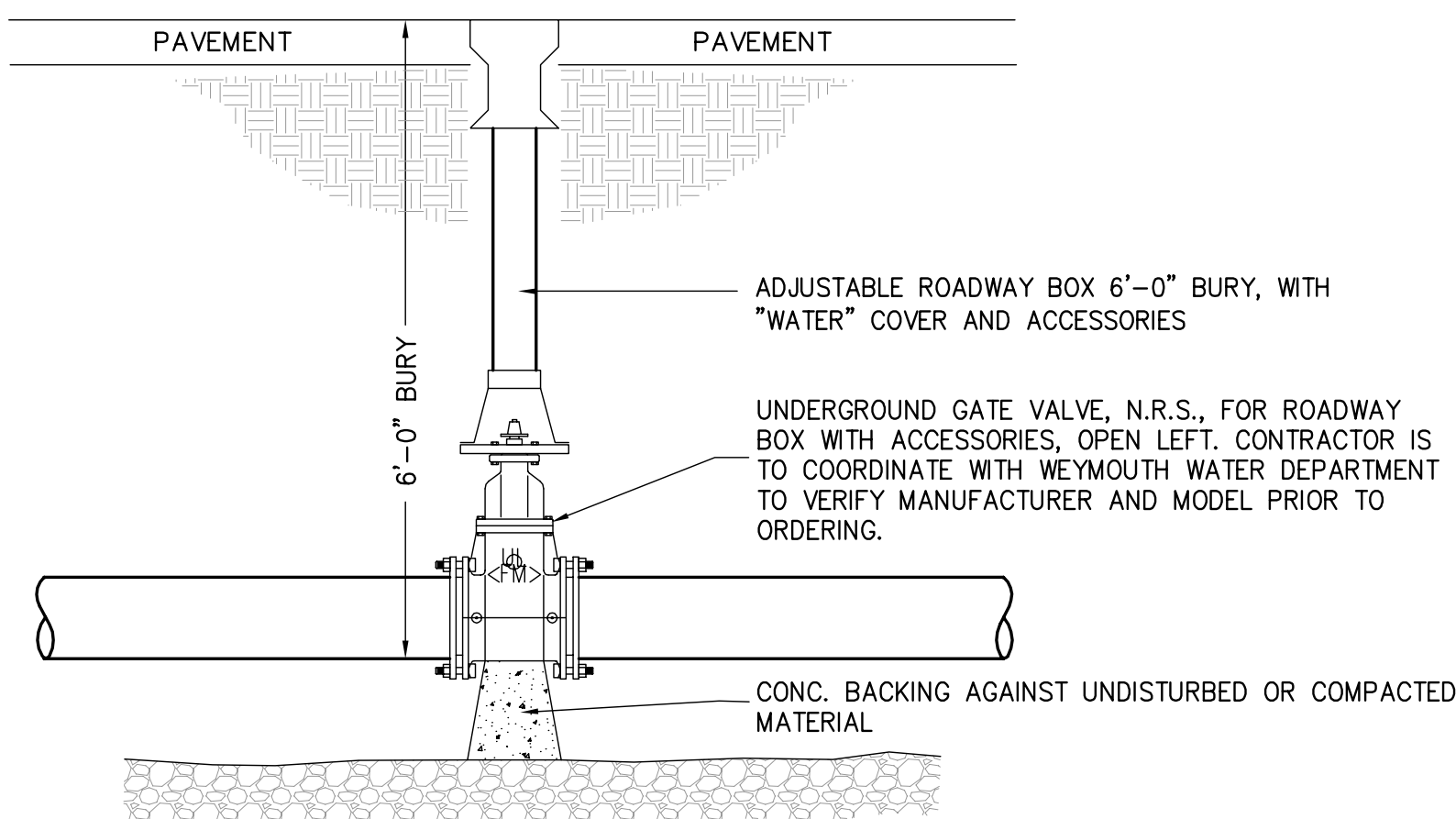
- FOR REQUIRED BEARING AREA DIMENSIONS D & L SEE TABLE DIMENSIONS OF D & L OTHER THAN THOSE SHOWN IN THE TABLE MAY BE USED PROVIDED THEY YIELD A BEARING AREA EQUAL TO OR LARGER THAN THAT REQUIRED.
- CONCRETE NOT TO OVERLAP ANY JOINT.
- CONCRETE TO BE PLACED SO AS NOT TO INTERFERE WITH REMOVING OR INSTALLING ANY OF THE JOINTING HARDWARE.
- APPROXIMATE VOLUME OF CONCRETE THRUST BLOCK:
 $V = \frac{LD(W+D)}{81}$
WHERE:
V = VOLUME IN CUBIC YARDS
L = LENGTH OF BLOCK IN FEET
D = DEPTH OF BLOCK IN FEET
W = WIDTH OF BLOCK IN FEET
ID = INSIDE DIAMETER OF PIPE IN FEET
- VALUES FOR TEE ALSO APPLY TO END PLUGS, CAPS, AND TAPPING SLEEVES.
- REQUIRED BEARING AREAS ARE DUE TO THRUSTS CAUSED BY 150 PSI WORKING PRESSURE PLUS 50K(75 PSI) SURGE ALLOWANCE RESULTING IN 225 PSI TOTAL INTERNAL PRESSURE. NORMAL PIPE DIAMETER USED.
- REQUIRED BEARING AREAS ARE BASED ON ALLOWABLE SOIL BEARING CAPACITY OF 2000 LBS. PER SQUARE FOOT FOR SAND. DUE TO OTHER SOIL CONDITIONS ENCOUNTERED, BEARING AREAS MAY BE MODIFIED BY THE ENGINEER.
- IN MUCK, PEAT, OR RECENTLY PLACED FILL ALL THRUST SHALL BE RESISTED BY PILES OR THE RODS TO SOLID FOUNDATIONS, OR BY REMOVAL OF SUCH UNSTABLE MATERIAL AND REPLACEMENT WITH BALLAST OF SUFFICIENT STABILITY TO RESIST THE THRUSTS, ALL AS REQUIRED BY THE ENGINEER.



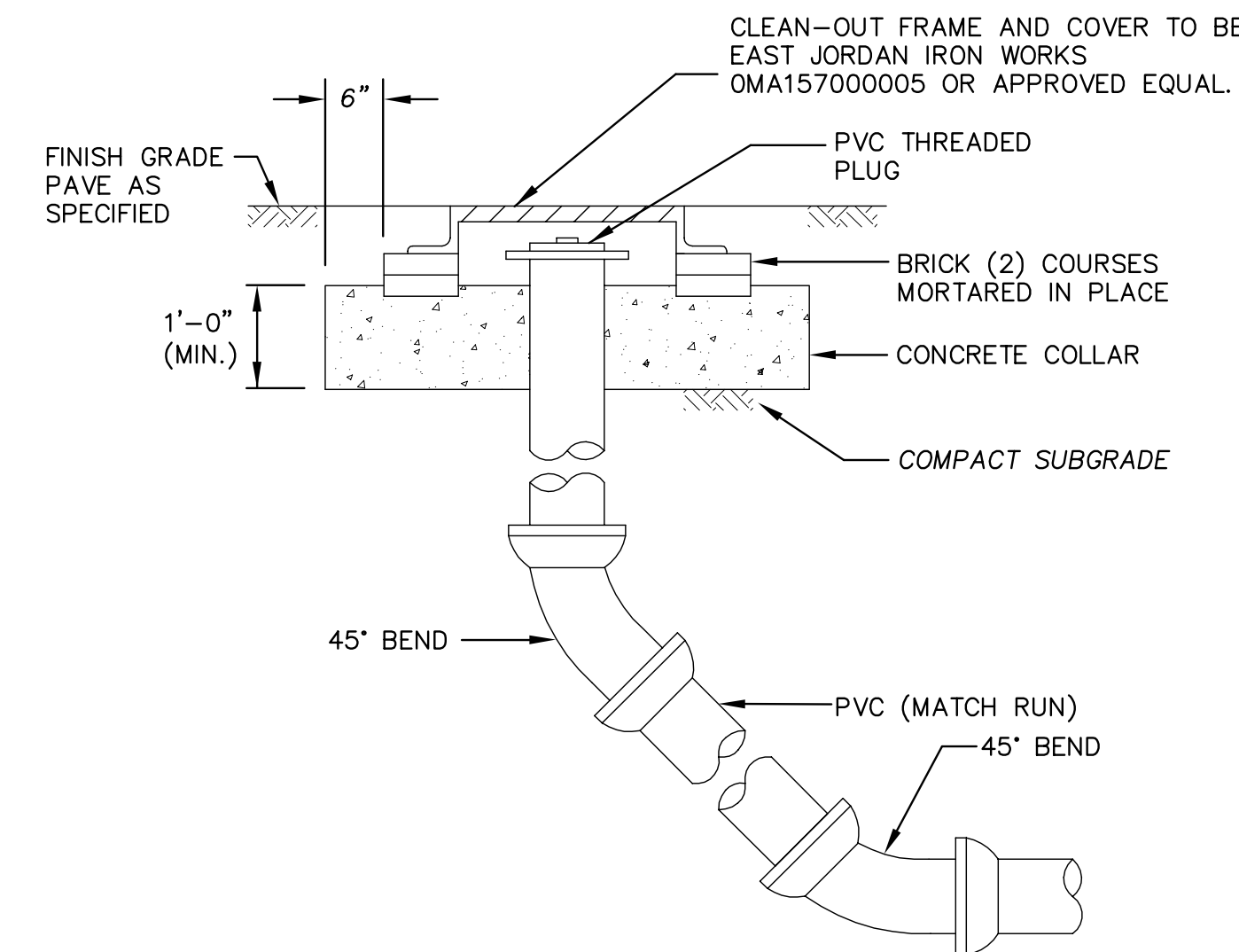
2 THRUST BLOCK DETAILS
SCALE: NO SCALE



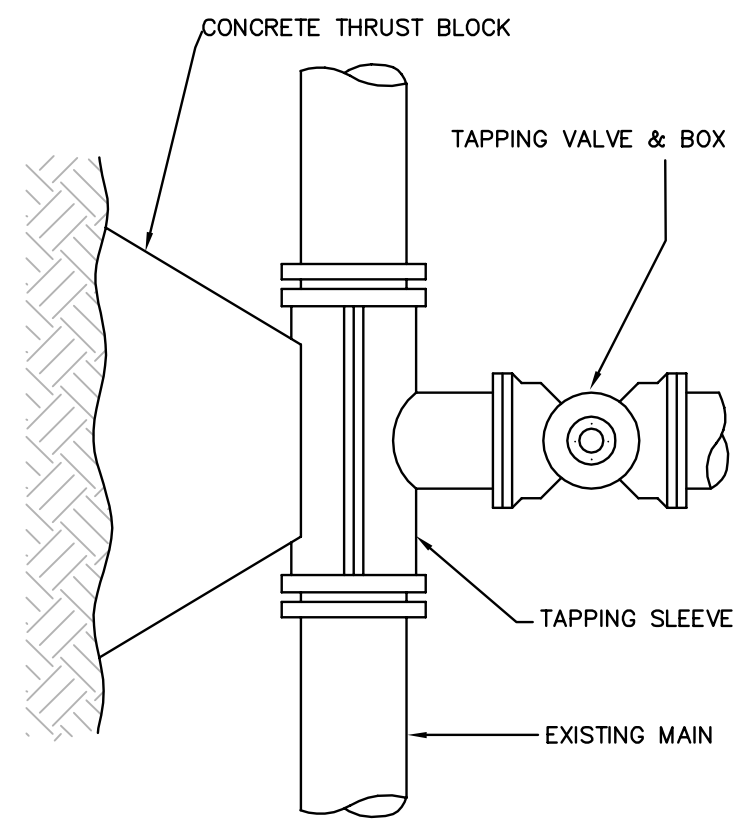
5 WATERLINE OBSTRUCTION DETAIL
SCALE: NO SCALE



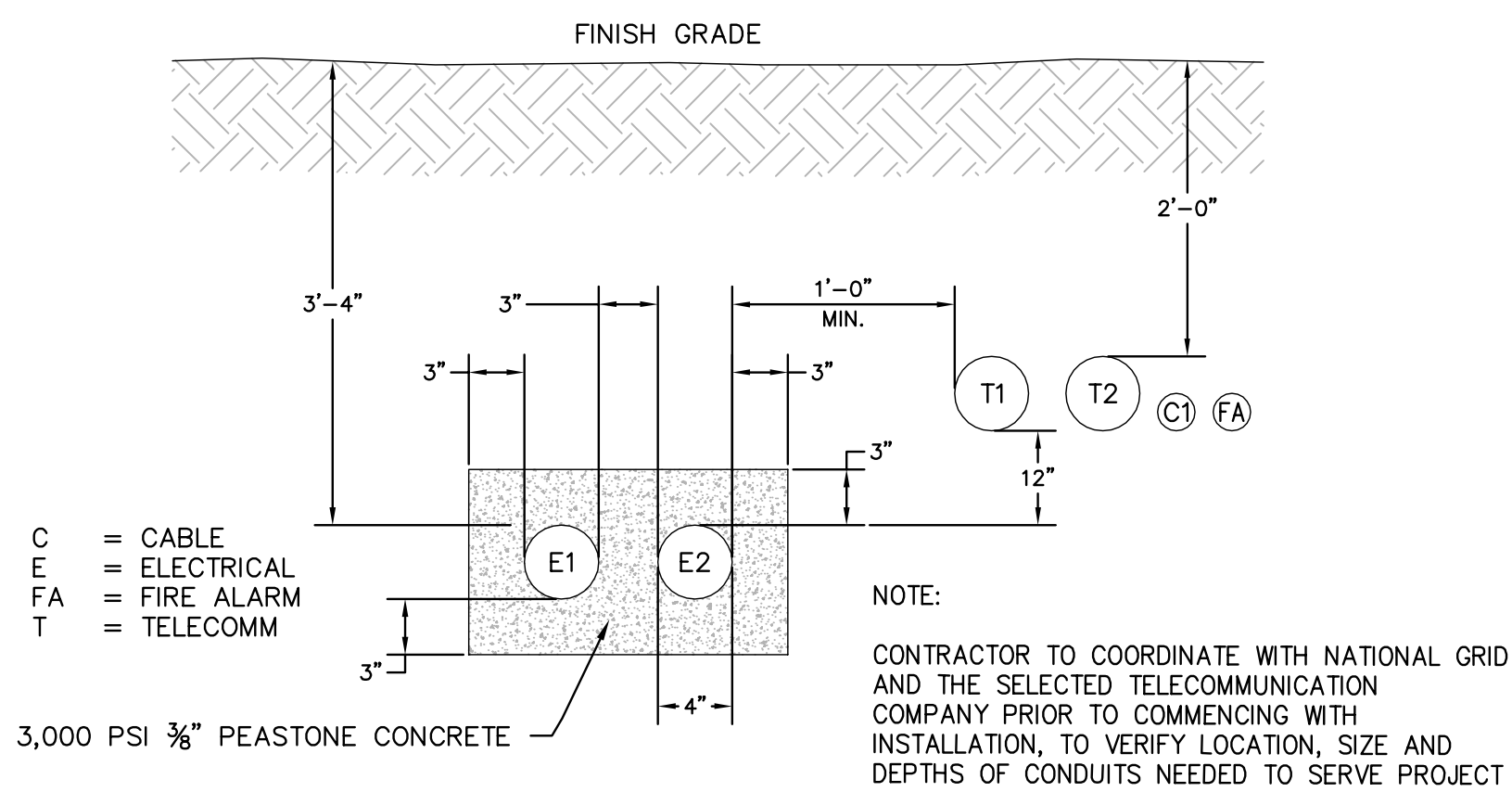
3 ROADWAY BOX VALVE DETAIL
SCALE: NO SCALE



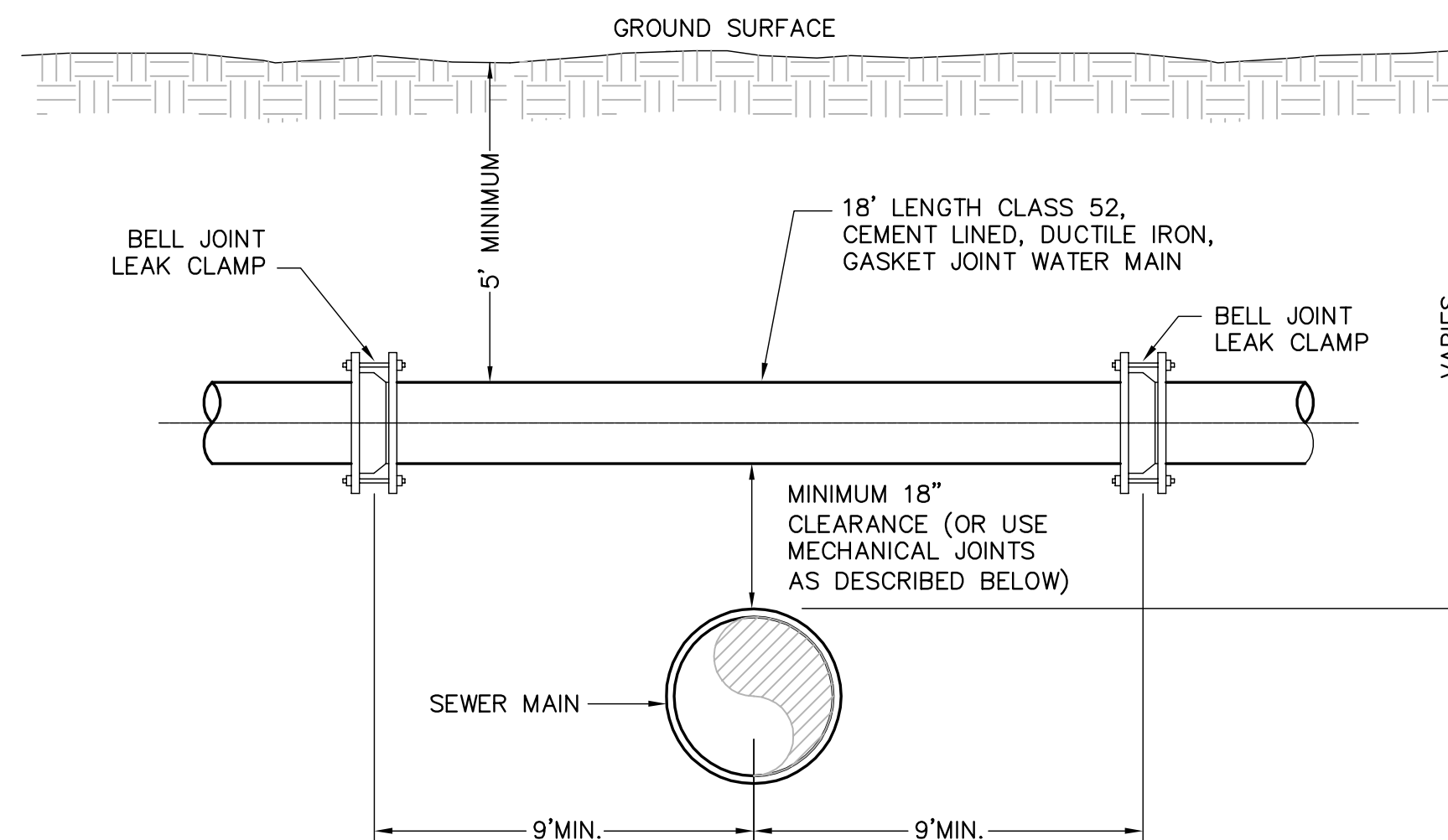
4 TYPICAL SEWER SERVICE CLEANOUT
SCALE: NO SCALE



6 TAPPING SLEEVE & VALVE DETAIL
SCALE: NO SCALE



7 TYPICAL UTILITY DUCTBANK CROSS SECTION
SCALE: NO SCALE



8 TYPICAL WATER CROSSING
SCALE: NO SCALE

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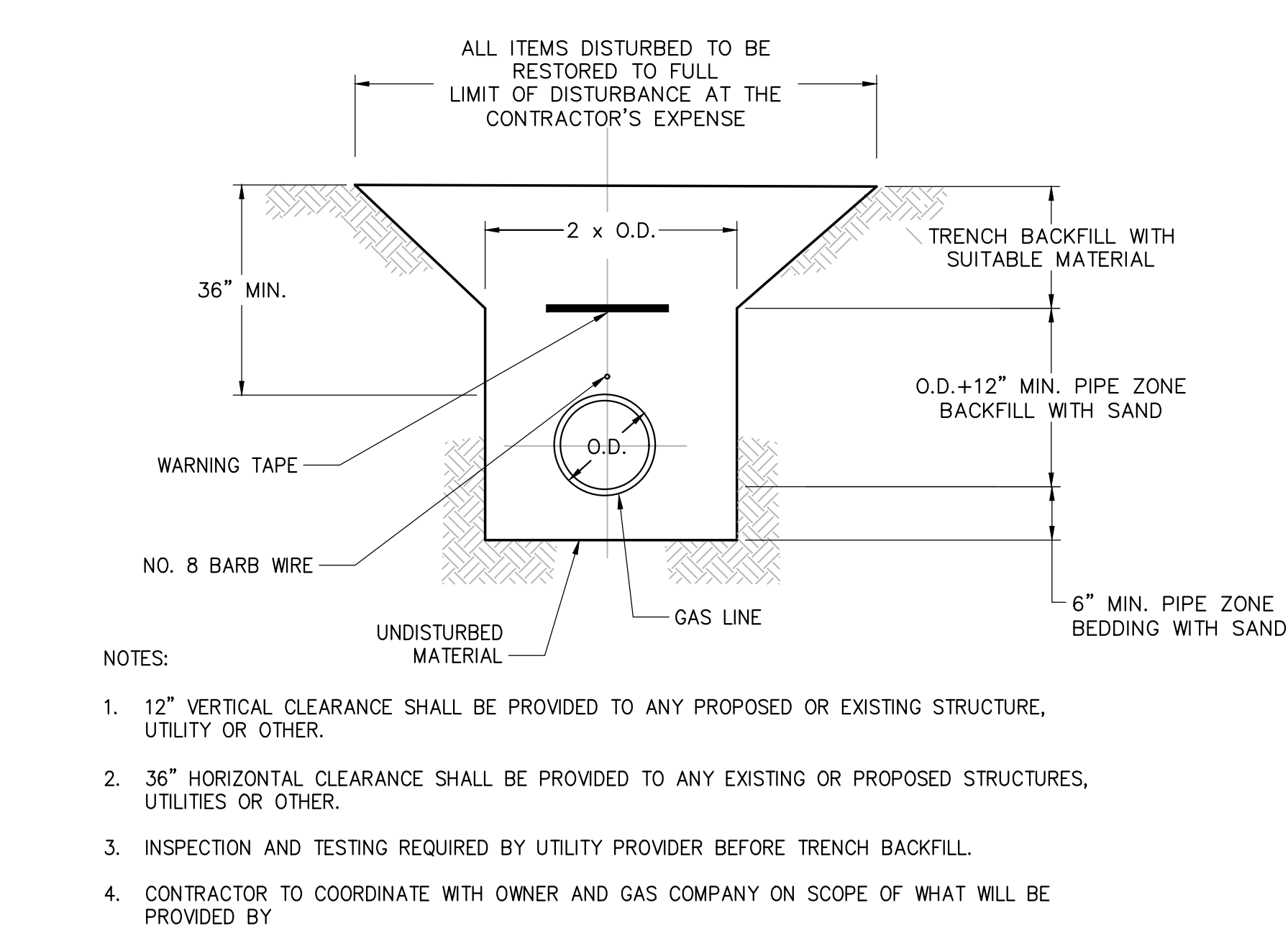
DATE: NOVEMBER 5, 2018
REVISION: BZA SUBMISSION

REVISIONS ON SHEET

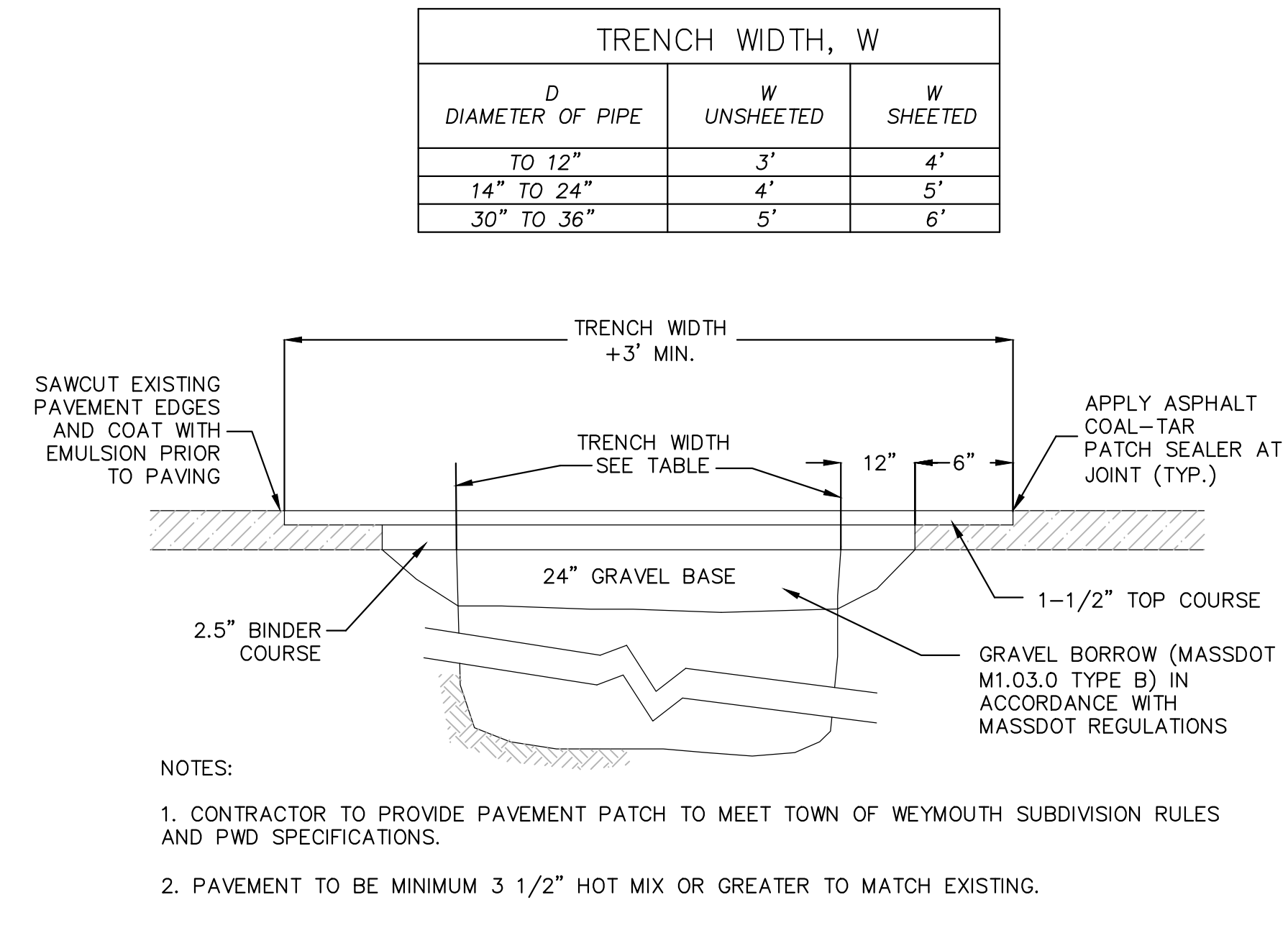
SCALE: UTILITE PROJECT NUMBER 1839

DETAILS - 6

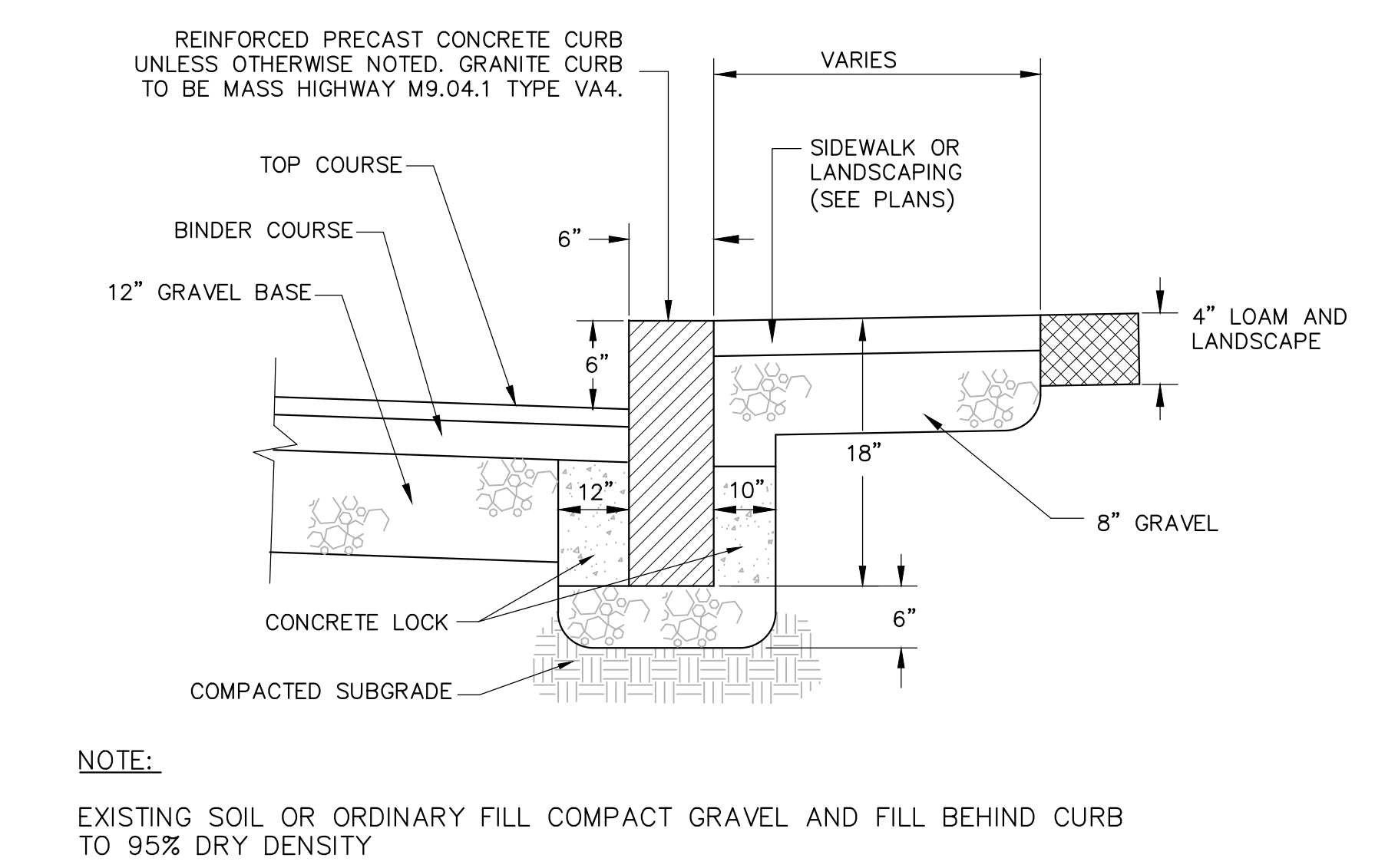
C-606



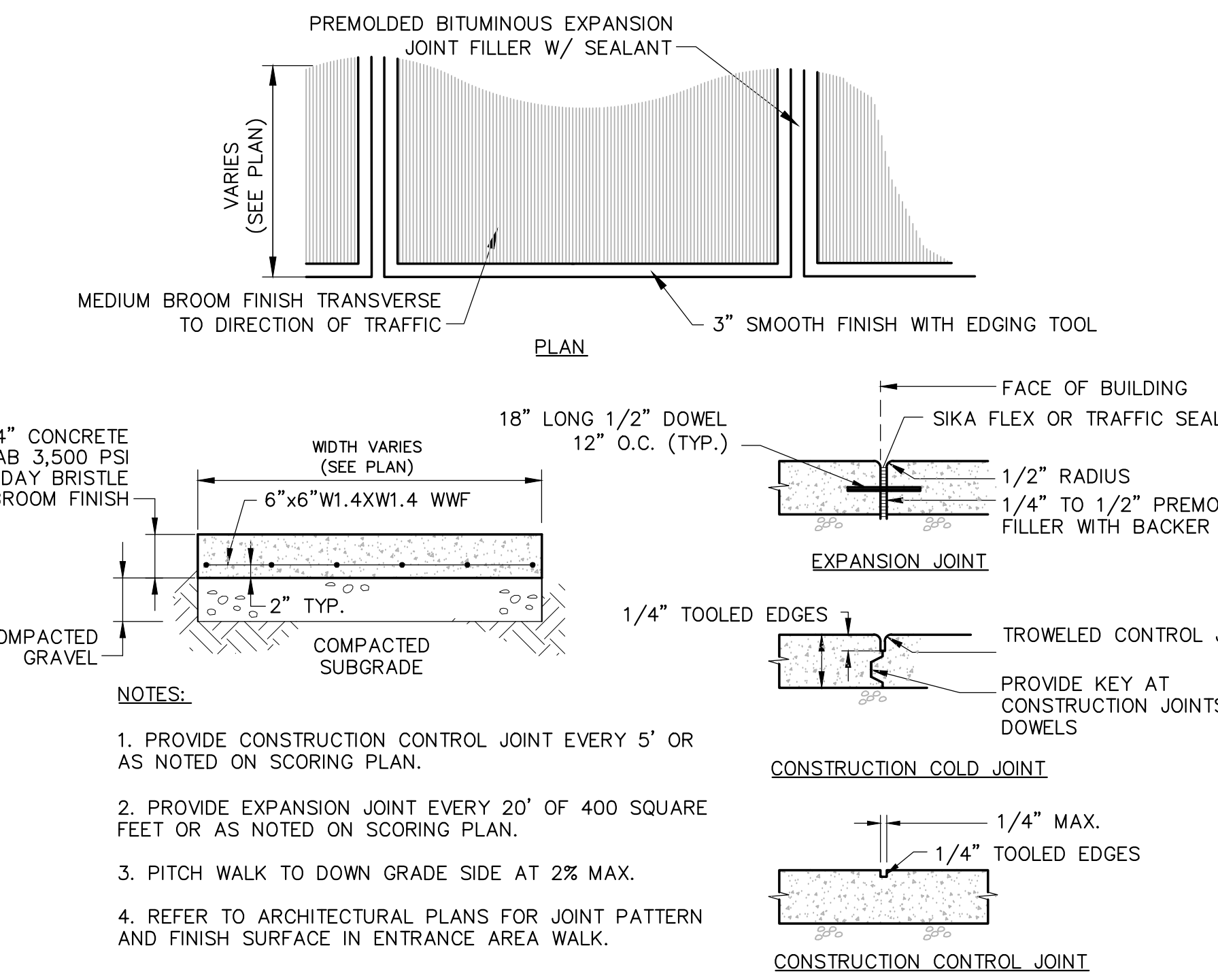
1 TYPICAL GAS UTILITY TRENCH DETAIL
SCALE: N.T.S.



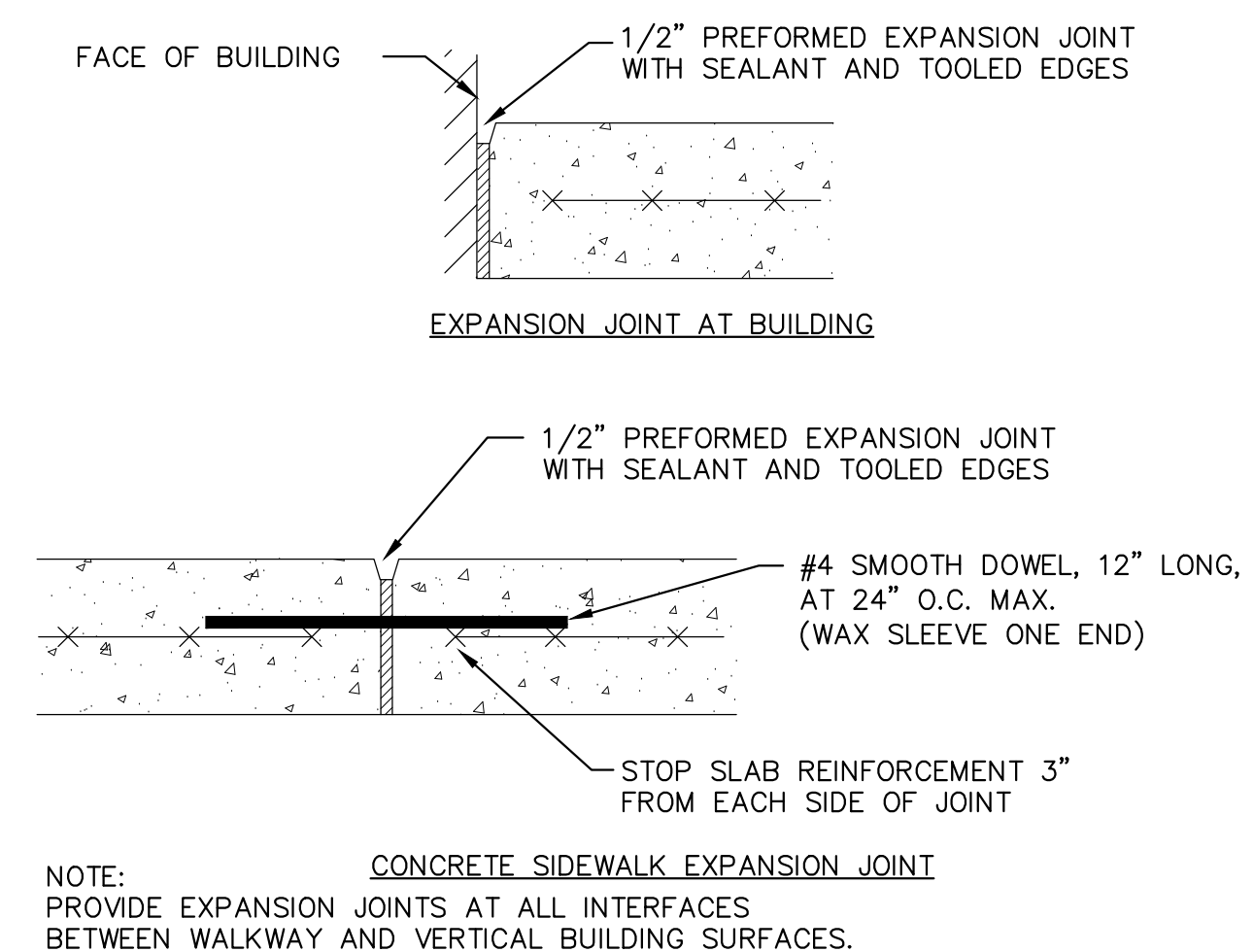
2 PAVEMENT PATCH DETAIL FOR TRENCH SECTIONS
SCALE: NO SCALE



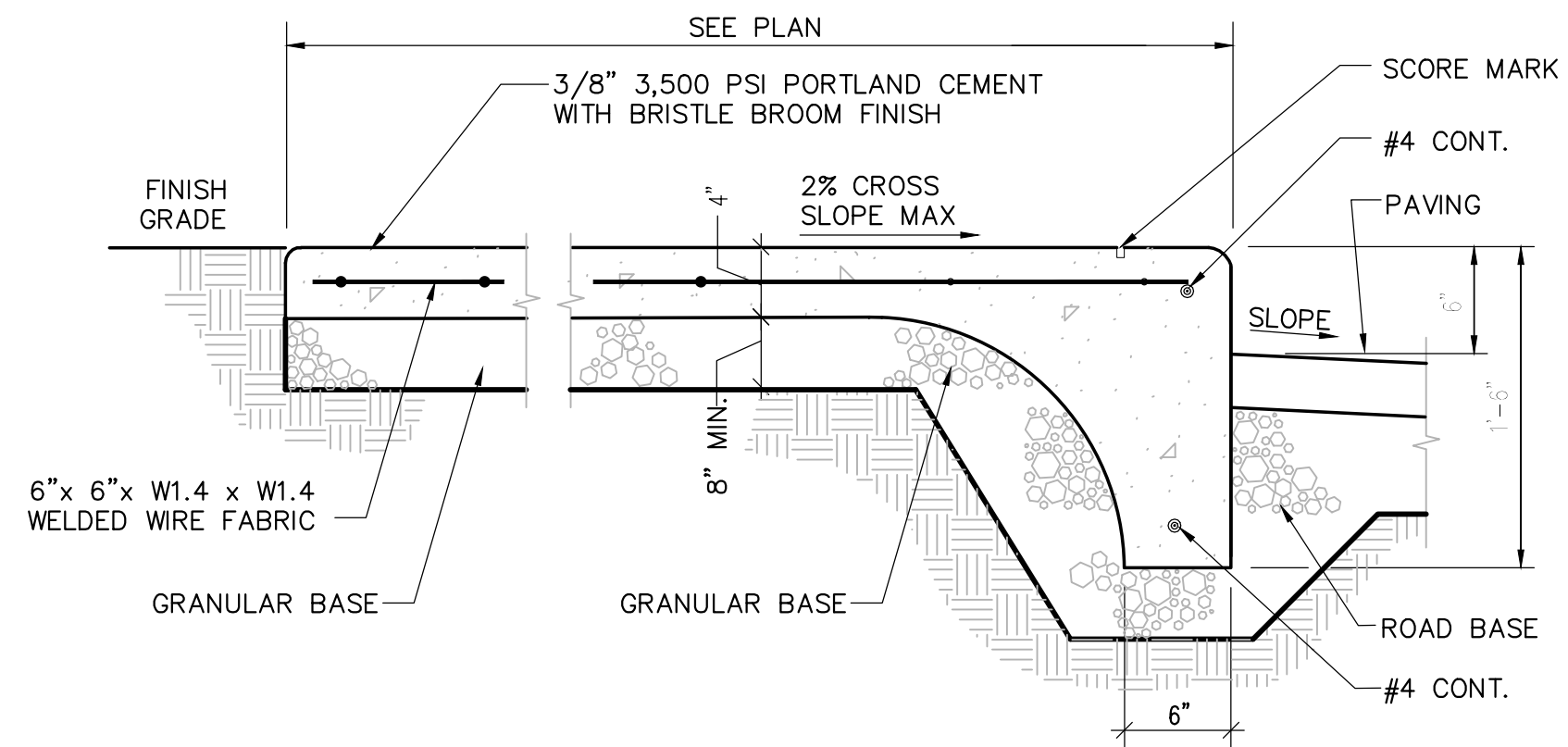
4 PRECAST CONCRETE OR VERTICAL GRANITE CURB DETAIL
SCALE: NO SCALE



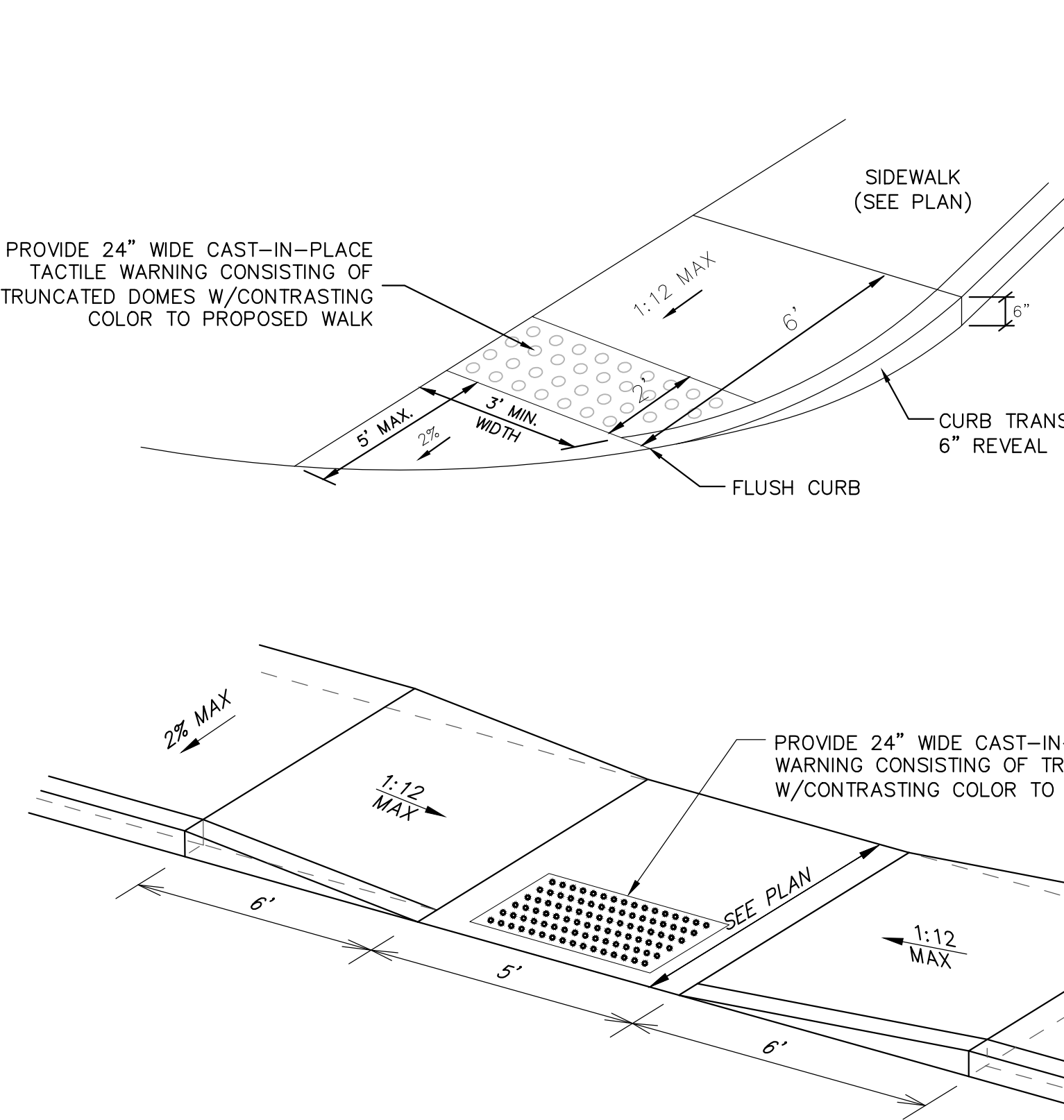
5 TYPICAL CONCRETE SIDEWALK DETAIL
SCALE: NO SCALE



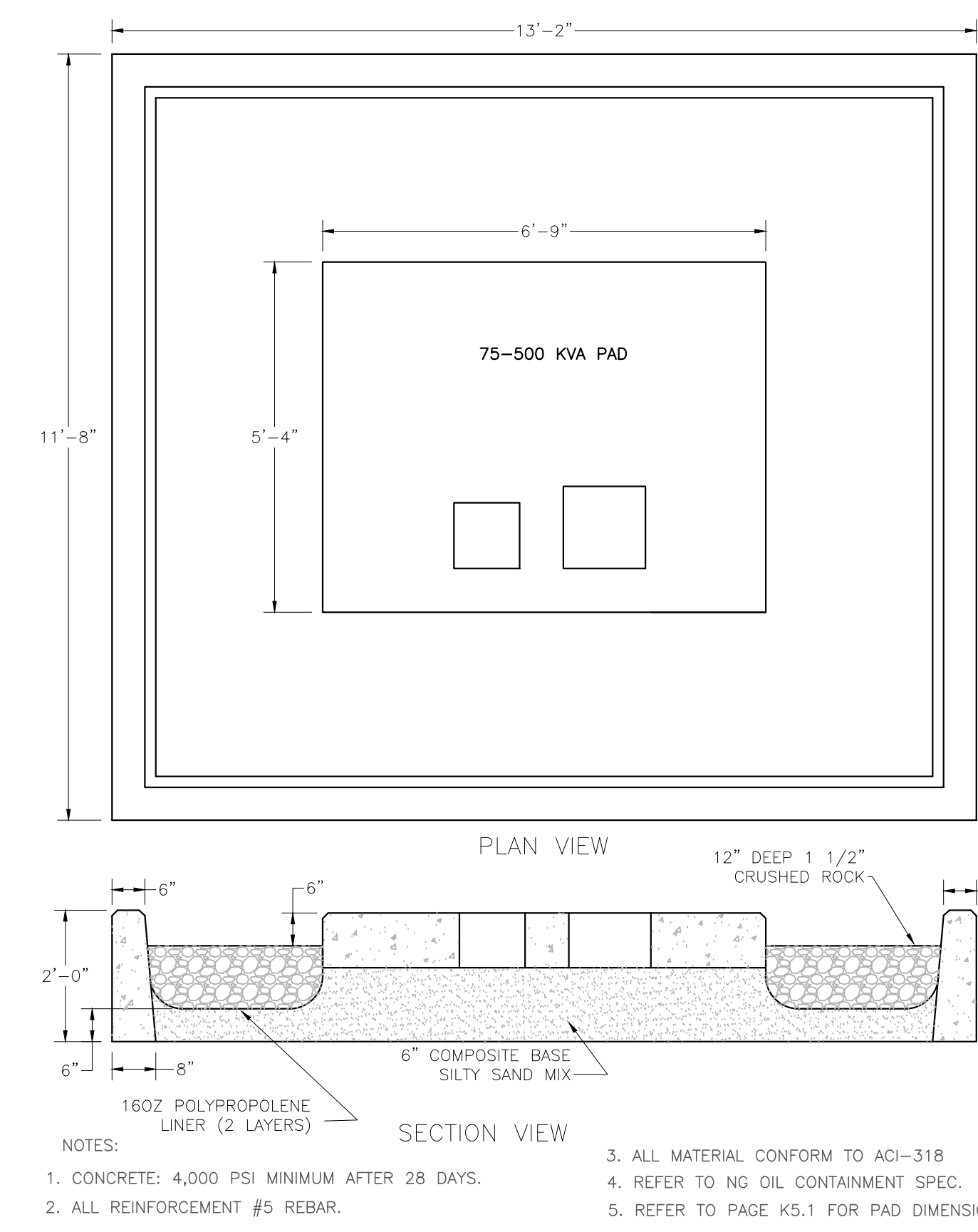
7 EXPANSION JOINT DETAIL
SCALE: NO SCALE



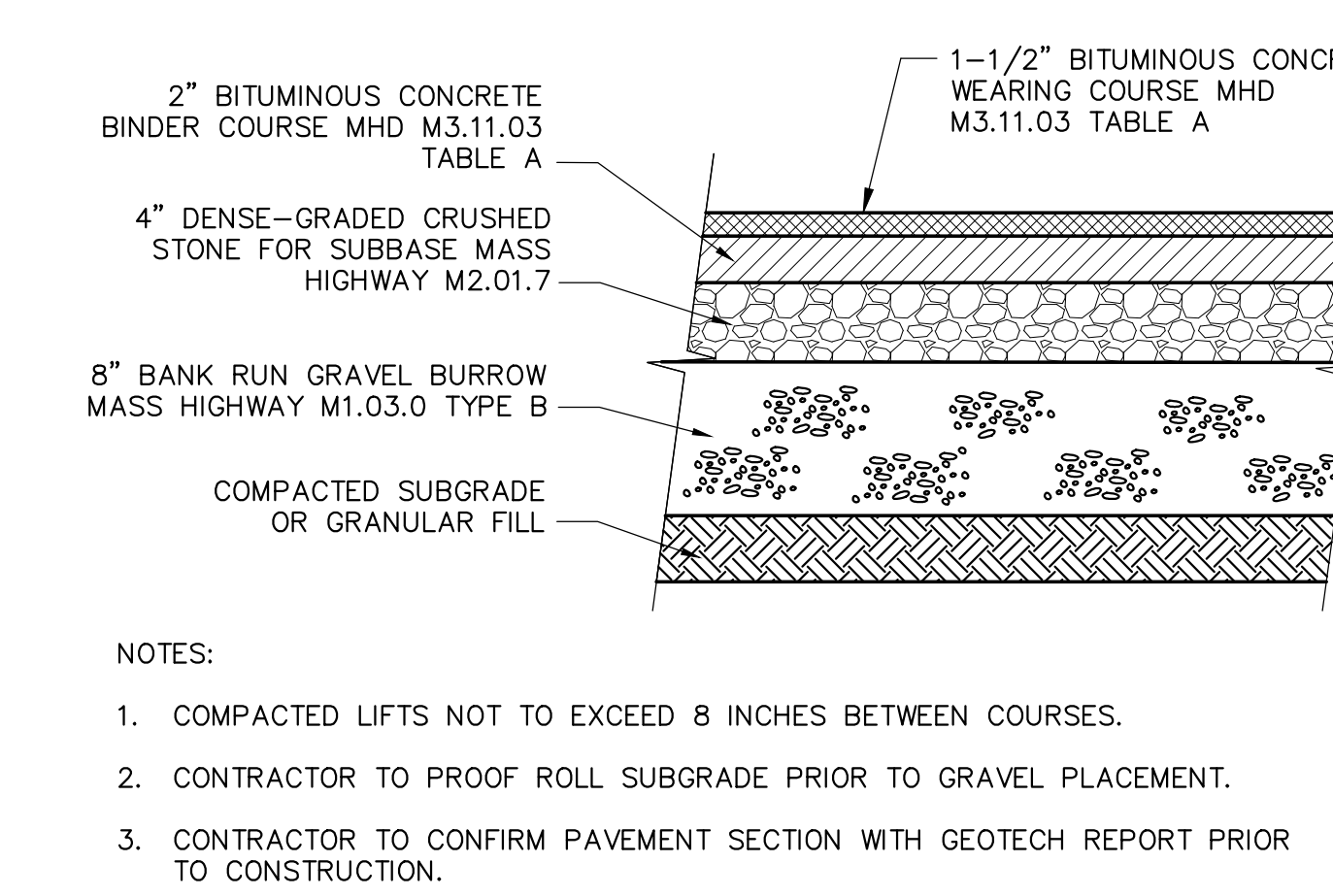
8 INTEGRAL WALK & CURB DETAIL (MONOLITHIC)
SCALE: NO SCALE



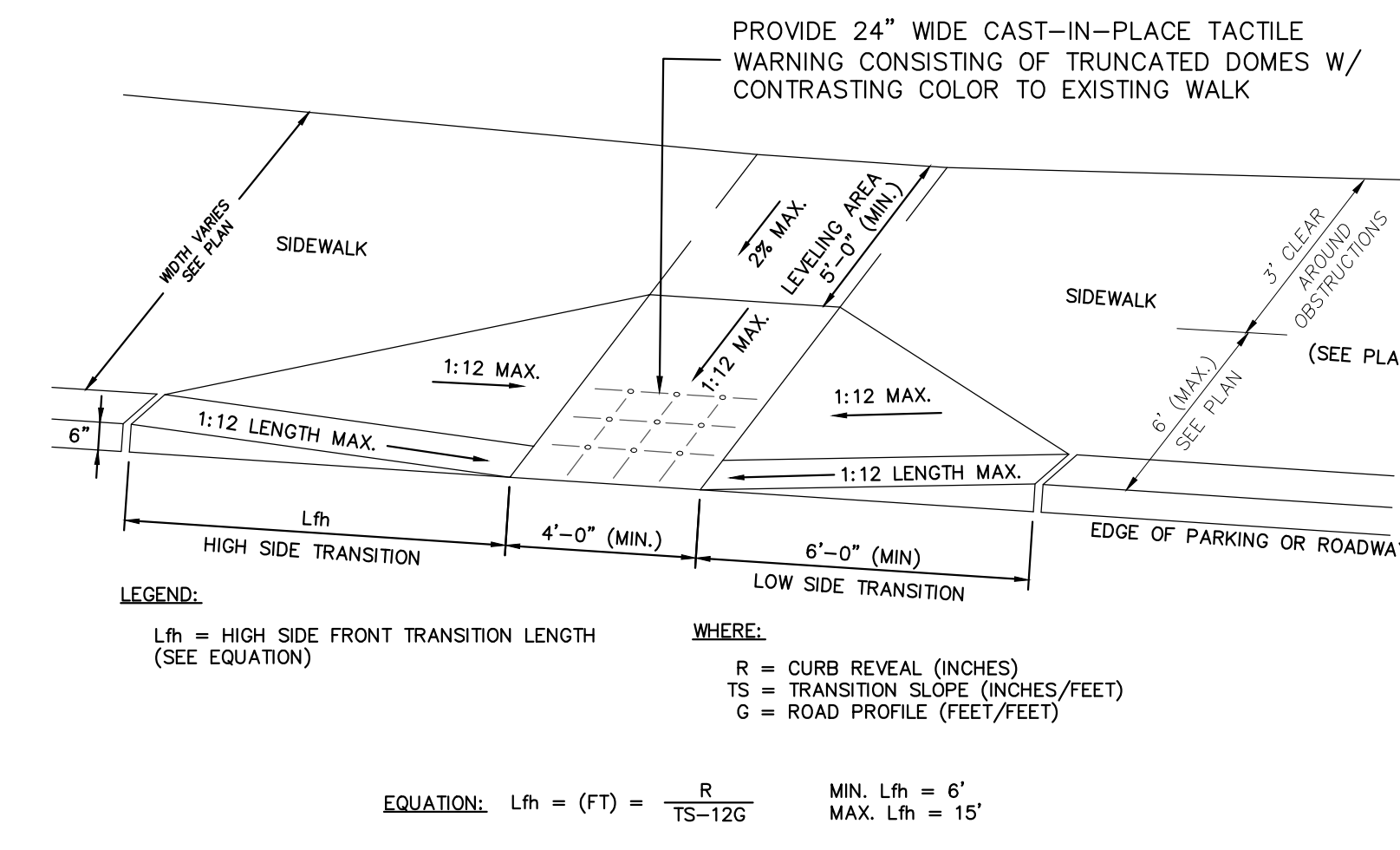
9 ADA CURB RAMP DETAILS
SCALE: NO SCALE

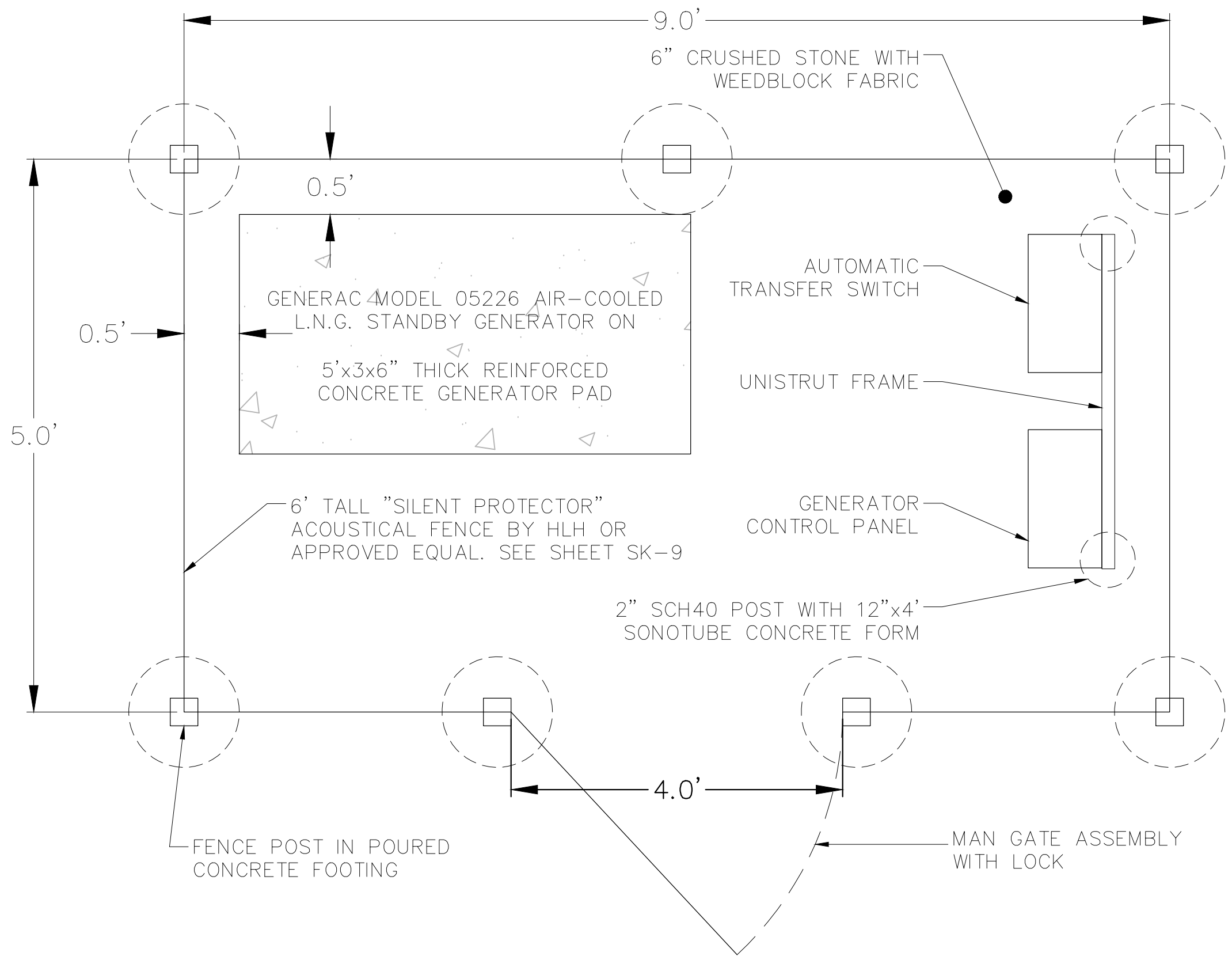


3 NGRID TRANSFORMER PAD DETAIL
SCALE: NO SCALE

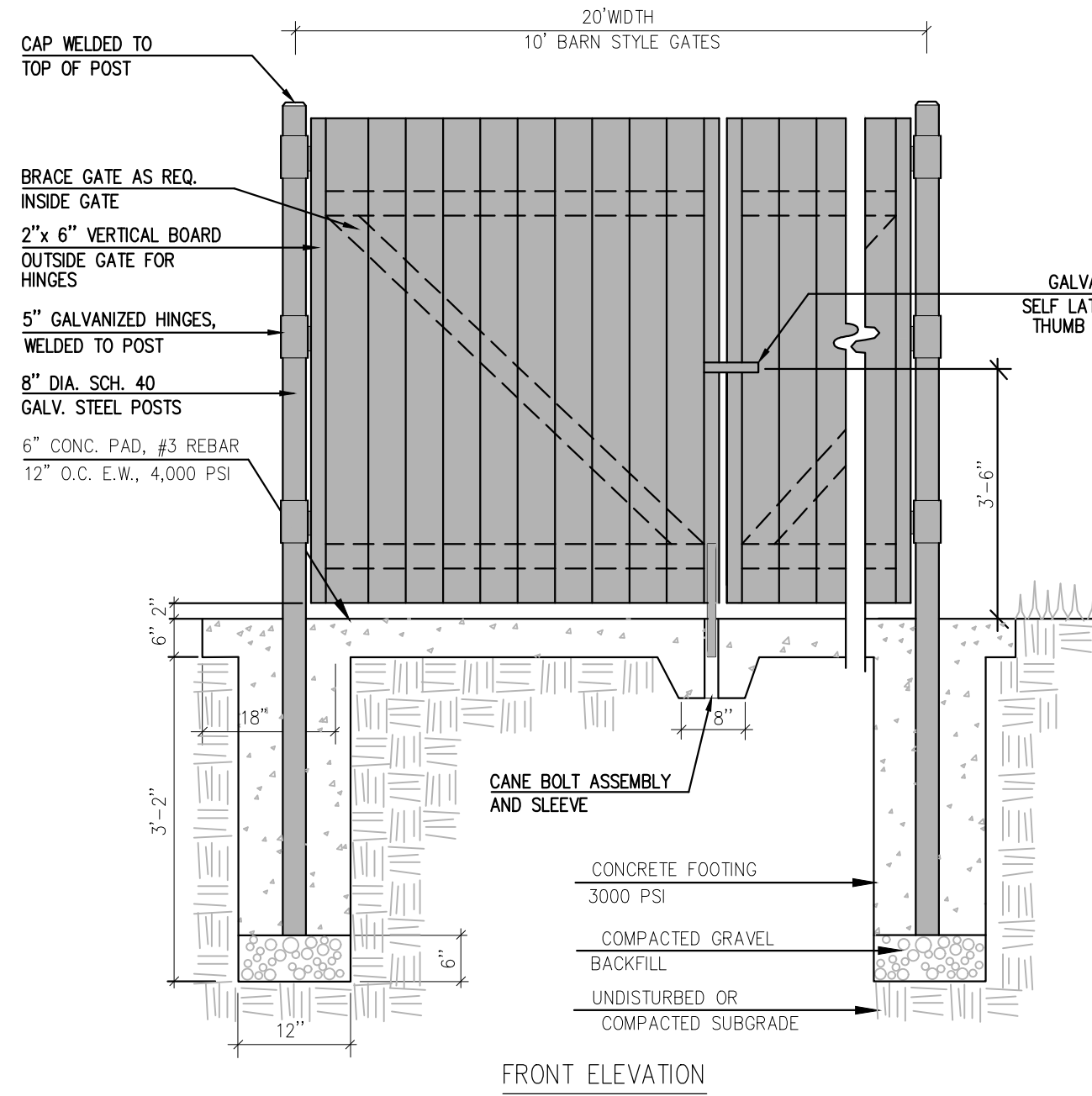


6 BITUMINOUS PAVEMENT SECTION DETAIL
SCALE: NO SCALE



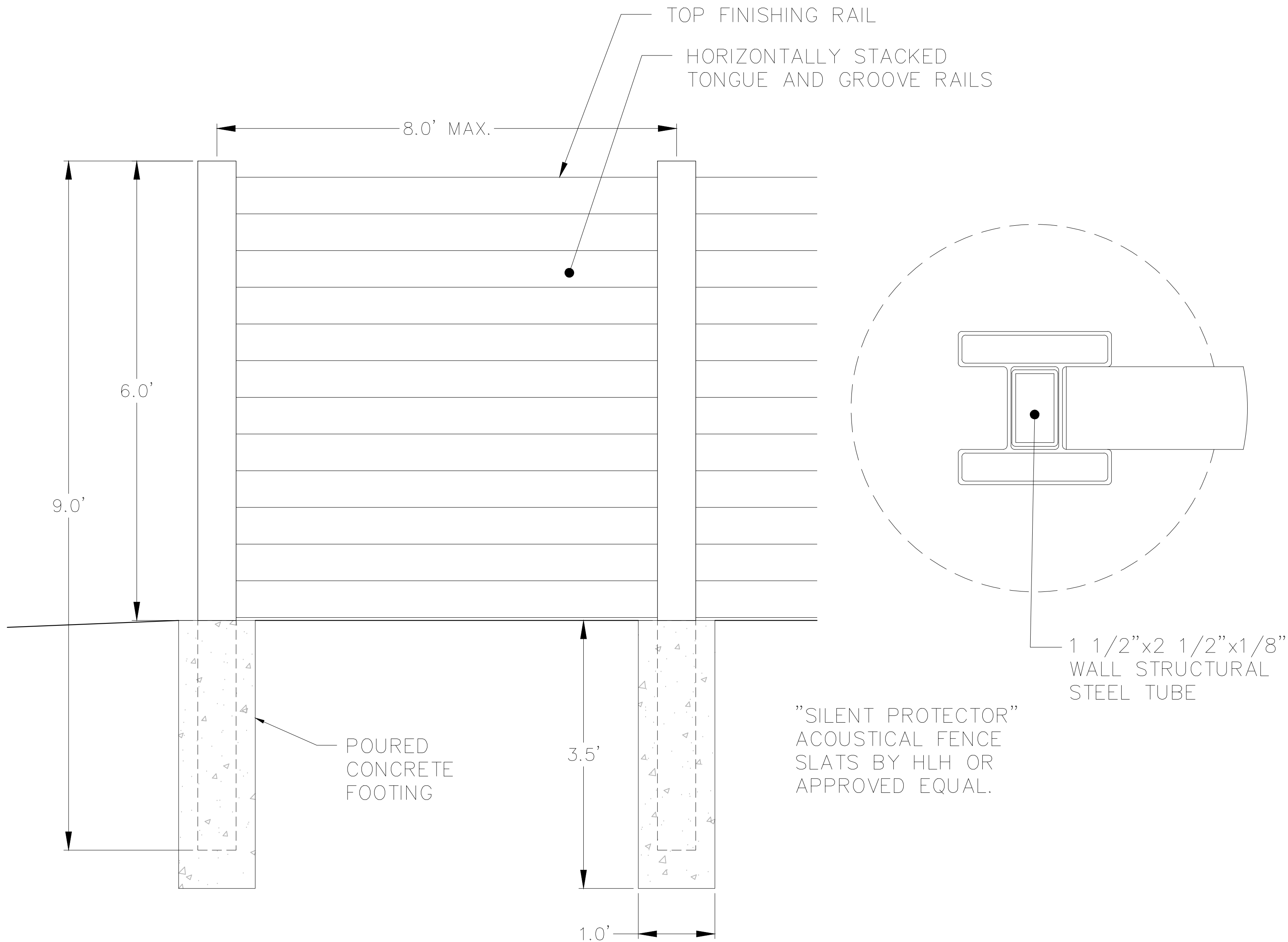


1 GENERATOR PAD ENCLOSURE DETAIL
SCALE: NO SCALE

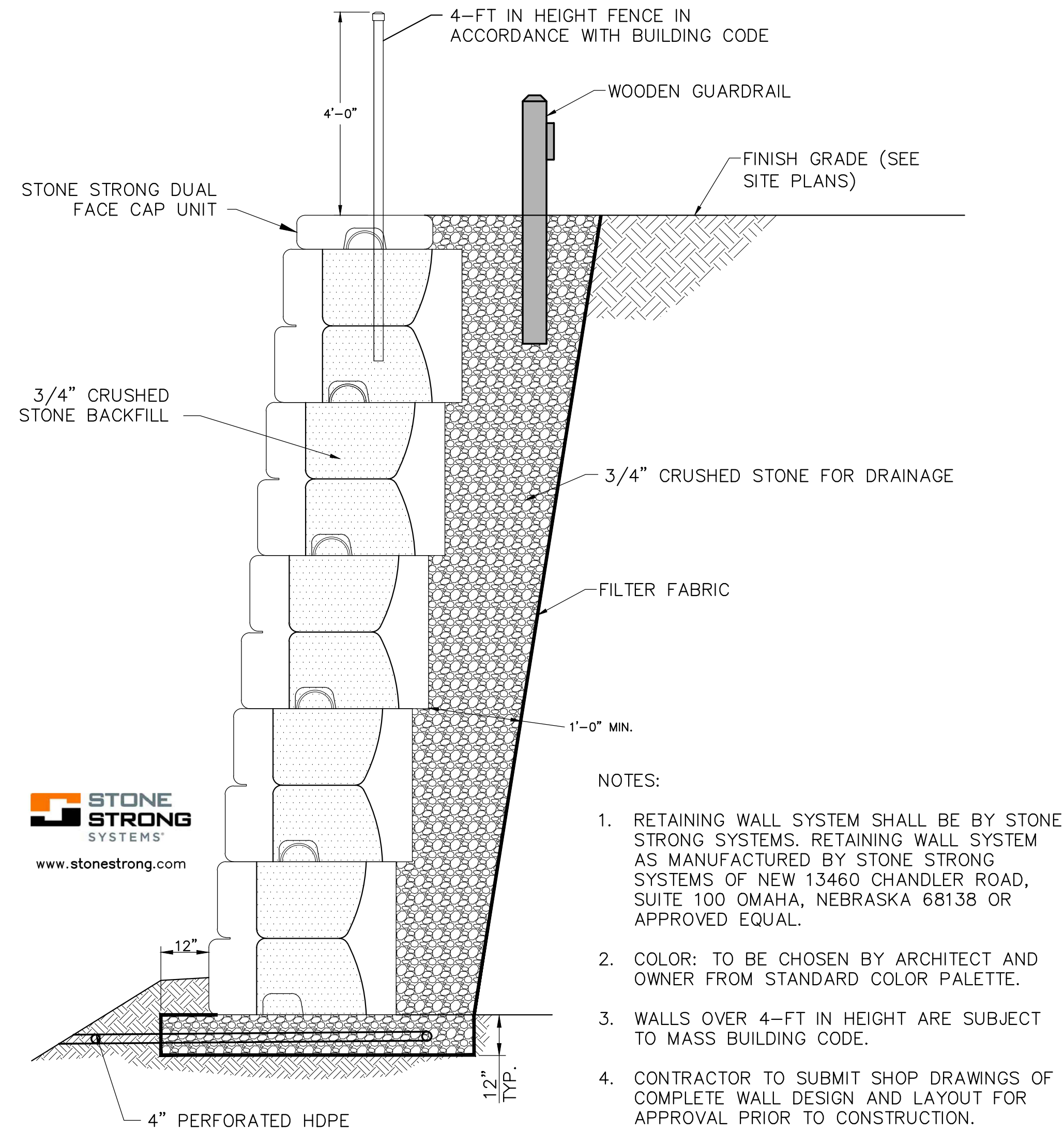


NOTE:
CONTRACTOR TO COORDINATE FENCE CONSTRUCTION WITH MANUFACTURER'S RECOMMENDATION AND TO WORK WITH CUSTOM DIMENSIONS IDENTIFIED ON SITE PLANS.

2 DUMPSTER PAD ENCLOSURE
SCALE: NO SCALE



3 GENERATOR PAD ENCLOSURE ACOUSTICAL FENCE DETAIL
SCALE: NO SCALE



4 SEGMENTAL BLOCK RETAINING WALL BY STRONG STONE SYSTEMS
SCALE: NO SCALE

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KEY PLAN



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SCALE: UTILE PROJECT NUMBER 1839

DETAILS - 8

C-608

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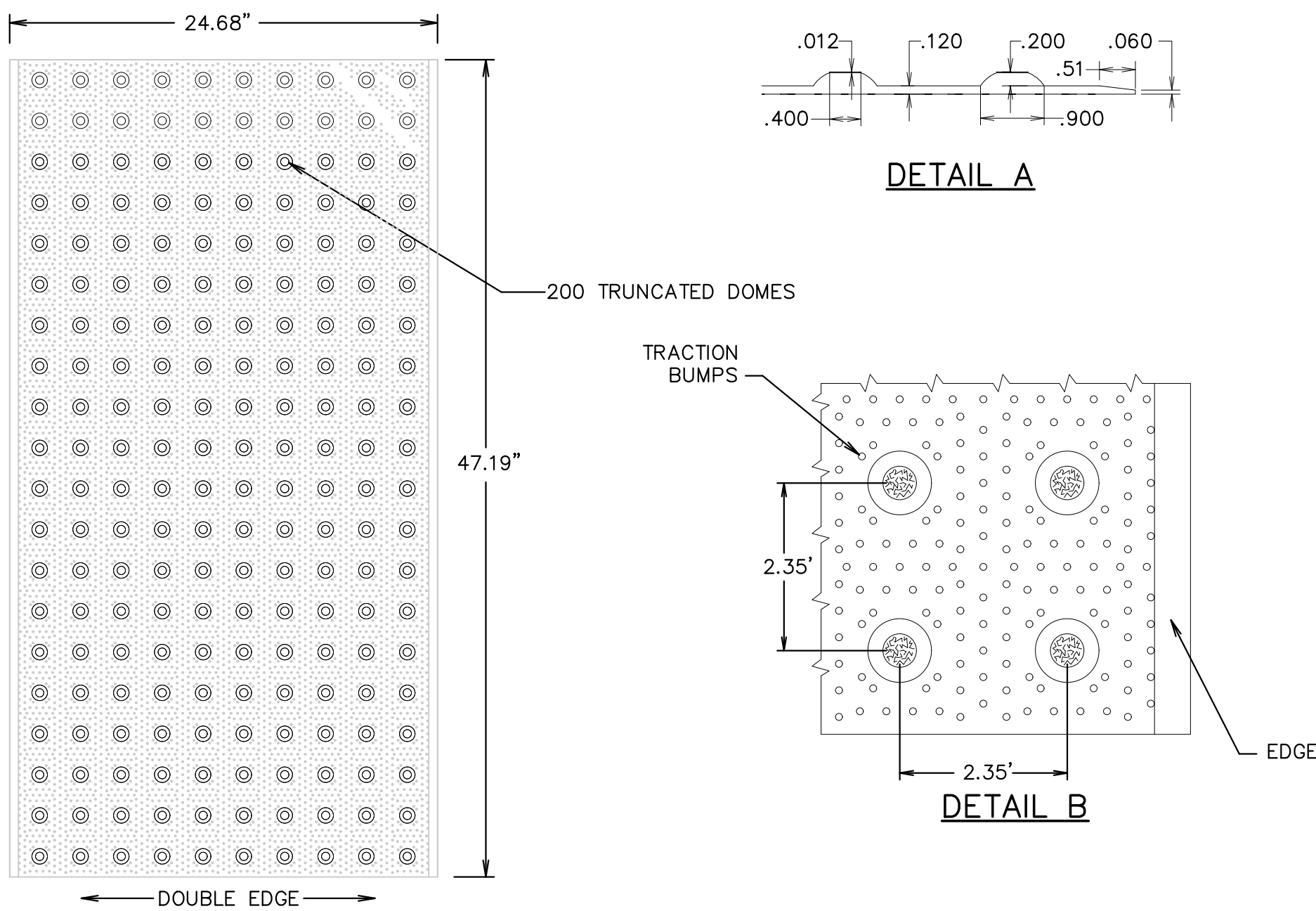
DATE REVISION
NOVEMBER 5, 2018 BZA SUBMISSION

REVISIONS ON SHEET

SCALE UTILE PROJECT NUMBER
1839

DETAILS - 9

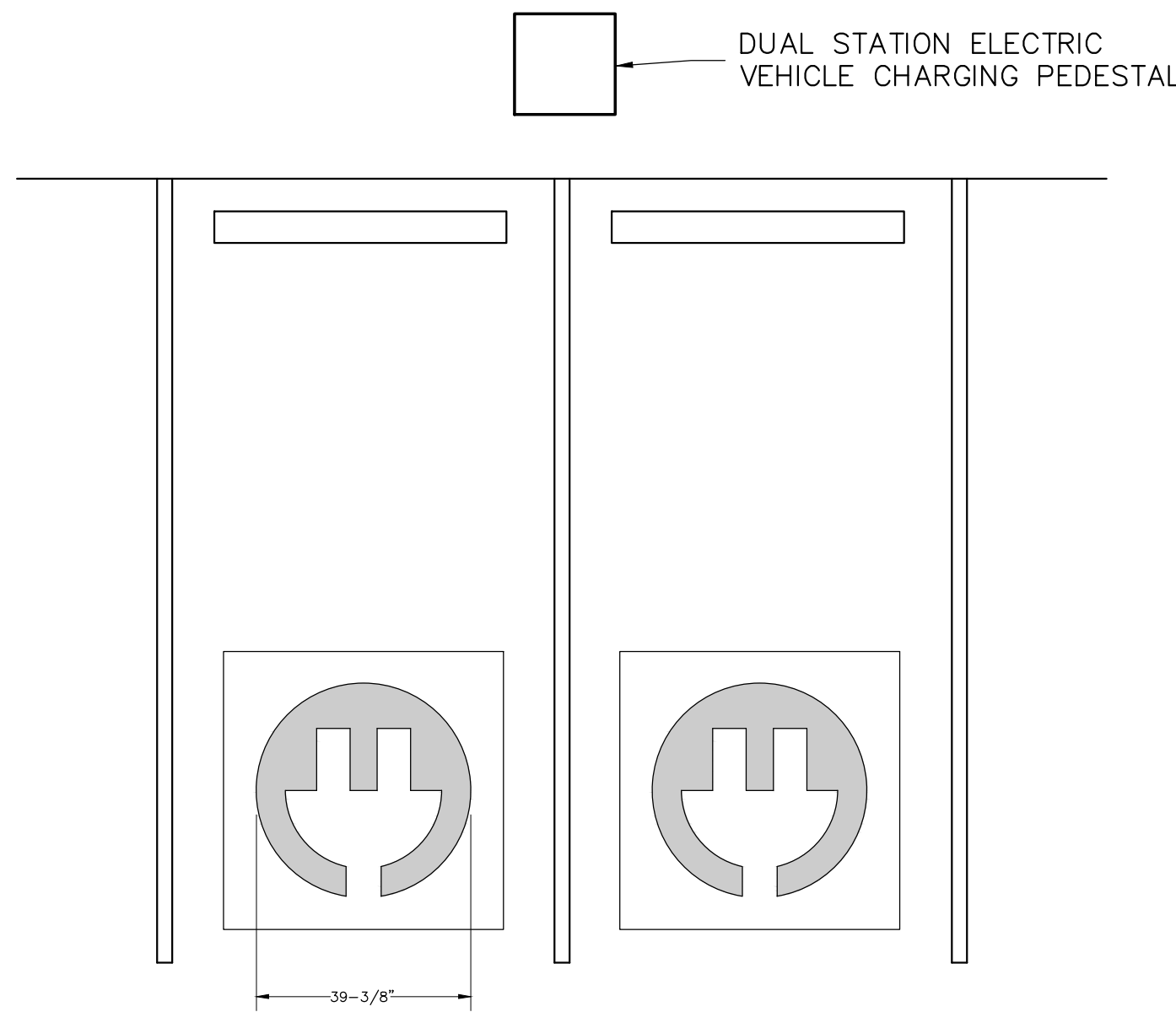
C-609



- NOTES:
1. DETECTABLE WARNING MATS SHALL BE PROVIDED BY 'DETECTABLE WARNING SYSTEMS, INC.' (866) 999-7452 WWW.DETECTABLE-WARNING.COM OR EQUAL. COLORS AVAILABLE YELLOW, BRICK RED, BLACK. CONTRACTOR TO COORDINATE COLOR WITH ARCHITECT.
 2. DETECTABLE WARNINGS ON CURB RAMPS SHALL CONFORM TO ADAAG SECTION 4.29.2 DETECTABLE WARNINGS ON WALKWAY SURFACES.
 3. DETECTABLE WARNINGS SHALL CONSIST OF TRUNCATED DOMES AND SHALL CONTRAST VISUALLY WITH ADJOINING SURFACES, EITHER LIGHT-ON-DARK OR DARK-ON-LIGHT.

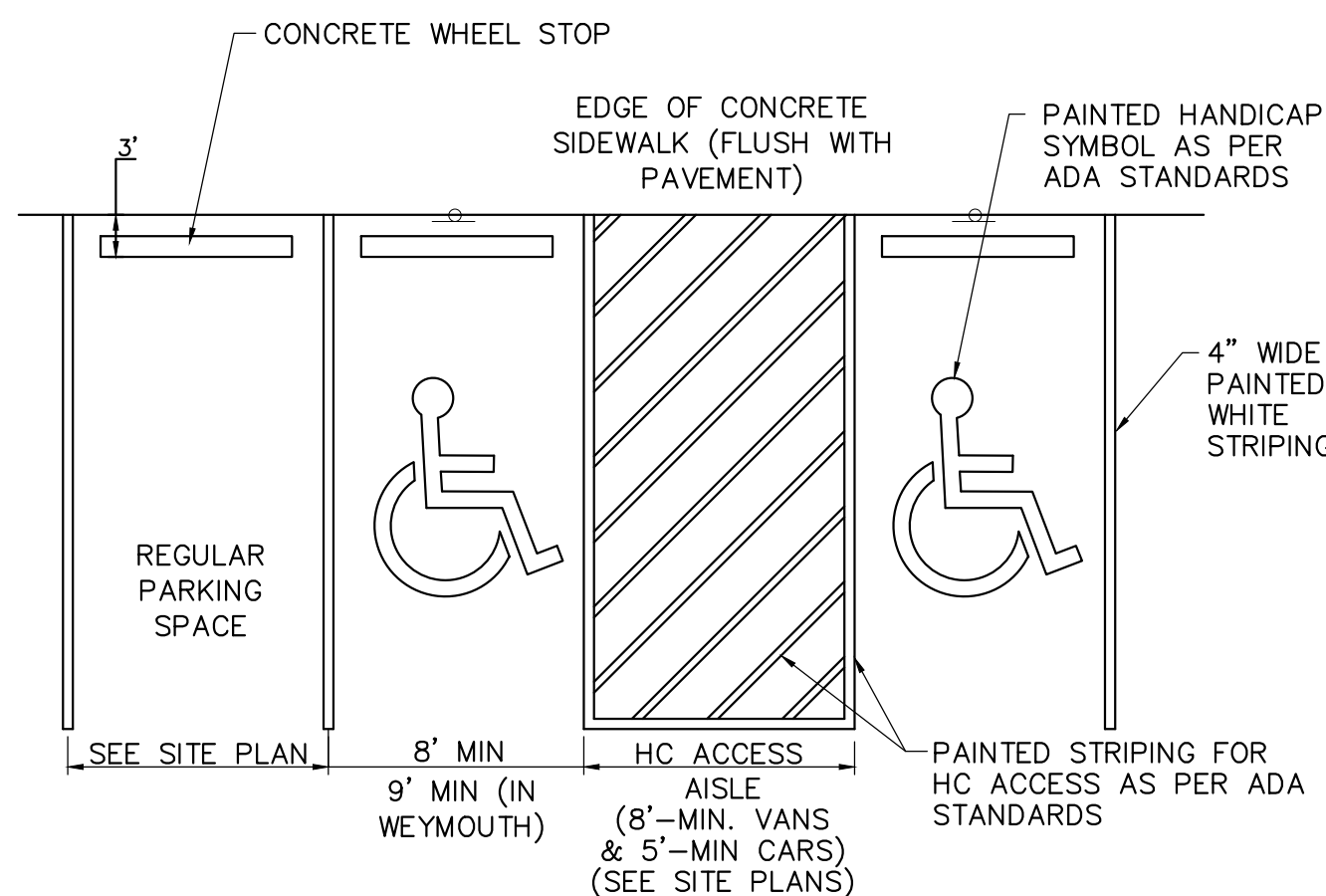
1 DETECTABLE WARNINGS AT RAMPS

SCALE: NO SCALE



4 EV PARKING STALL PAVEMENT MARKING DETAIL

SCALE: NO SCALE



PLAN

2 ACCESSIBLE STRIPING DETAIL

SCALE: NO SCALE



STOP SIGN
R1-1

5 SIGN DETAIL

SCALE: NO SCALE

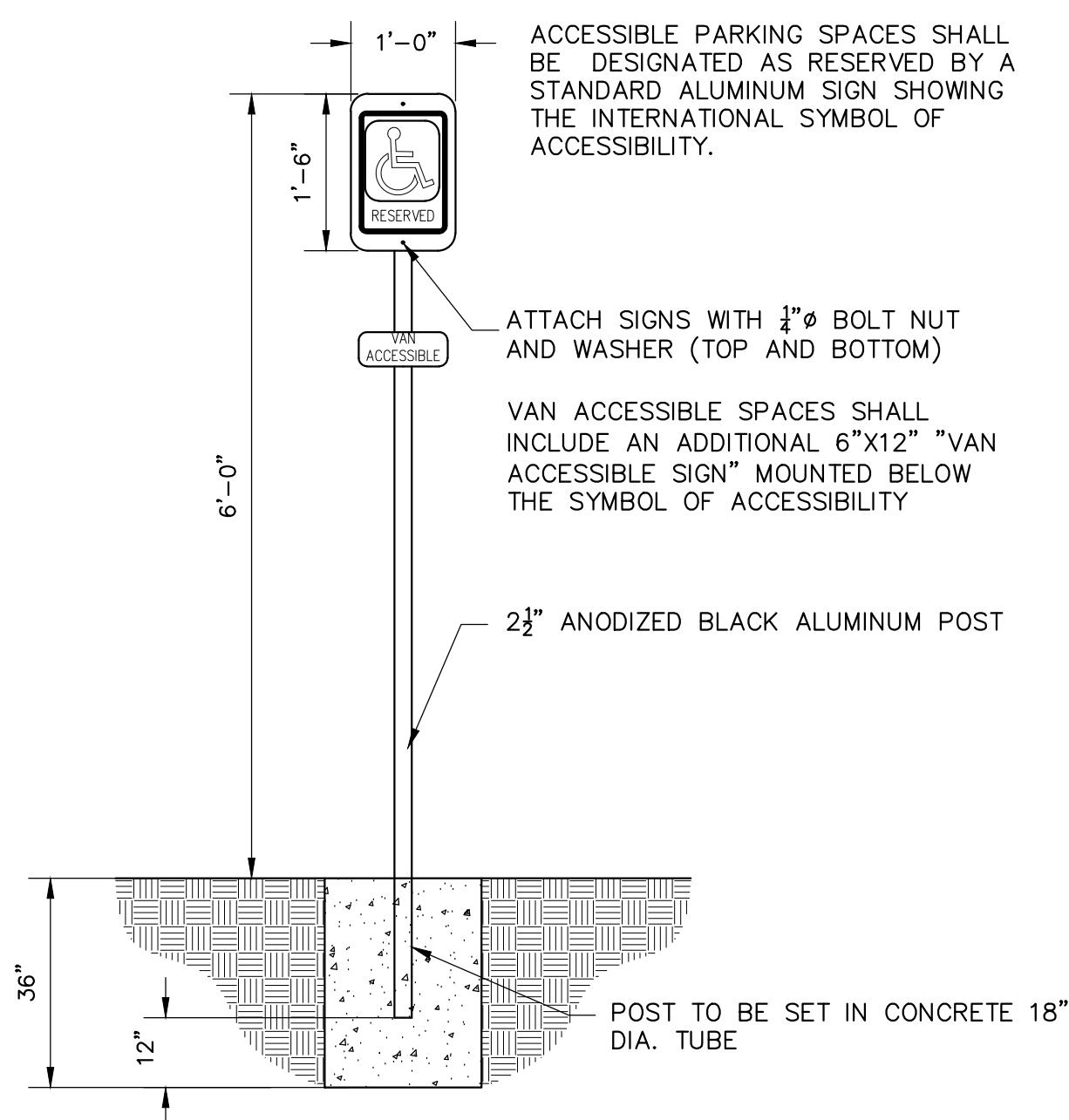


STOP BAR

- NOTES:
1. ALL PAINT TO BE THERMOPLASTIC.
 2. WORDS CAN BE ARRANGED IN OTHER COMBINATIONS THAN THOSE ILLUSTRATED HERE TO ACHIEVE DESIRED RESULTS.
 3. PAINTING OF WORDS TO BE AT THE DISCRETION OF THE OWNER. CONTRACTOR SHALL VERIFY PRIOR TO CONSTRUCTION.

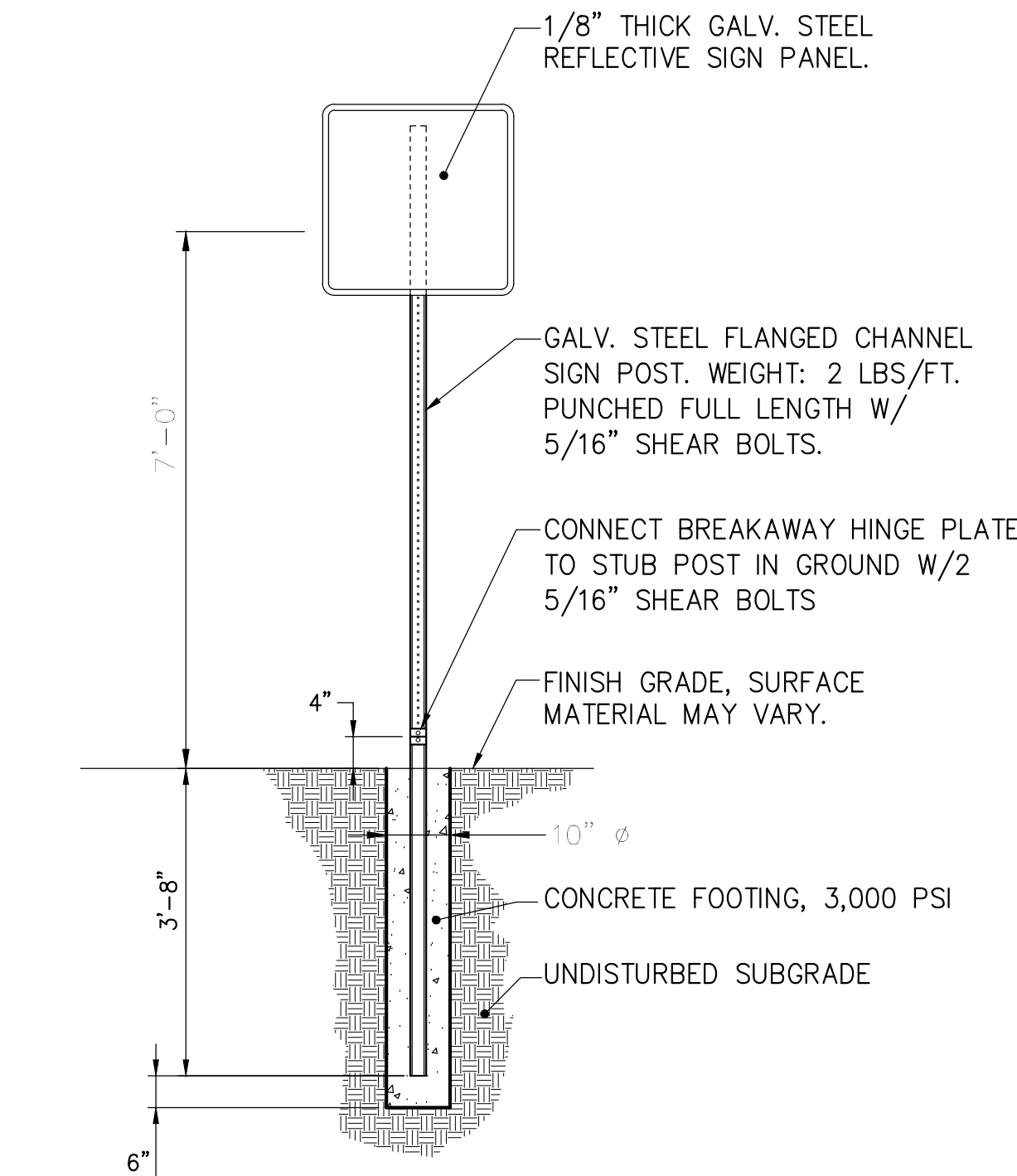
6 PAINTED MARKING DETAIL

SCALE: NO SCALE



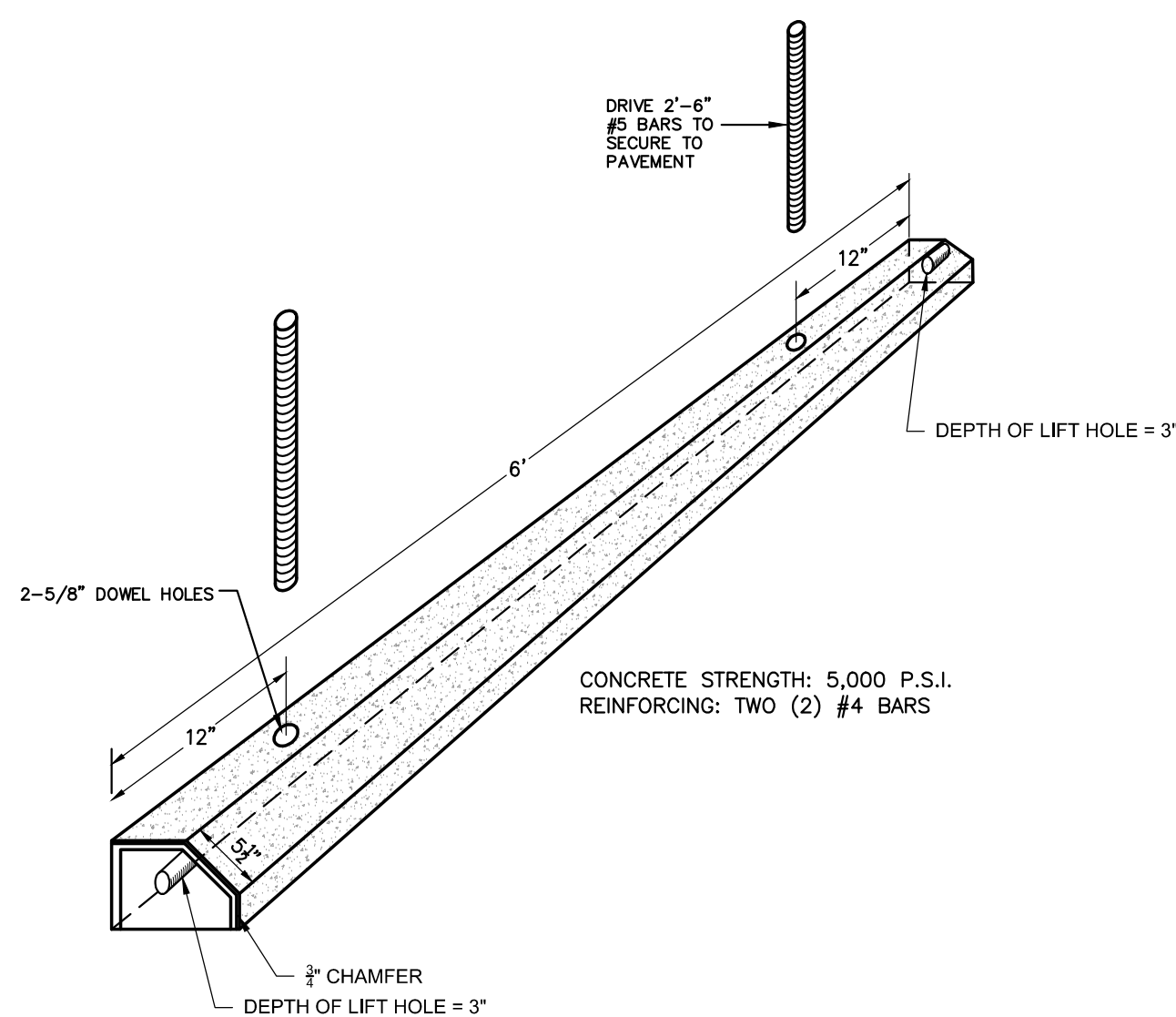
7 ACCESSIBLE PARKING SIGN DETAIL

SCALE: NO SCALE



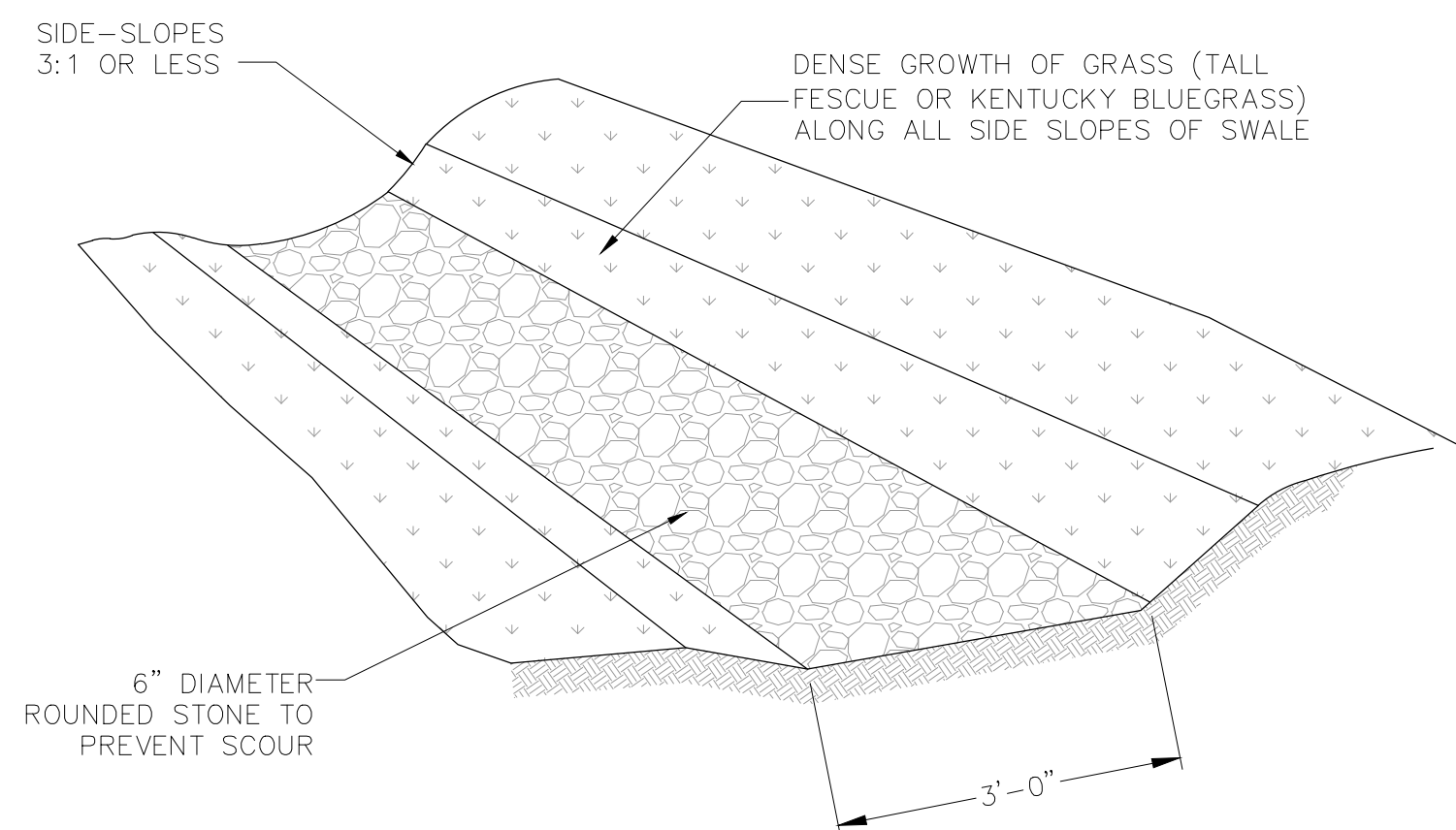
8 ROADWAY SIGNAGE w/BREAKWAY SUPPORT

SCALE: NO SCALE



9 CONCRETE WHEEL STOP DETAIL

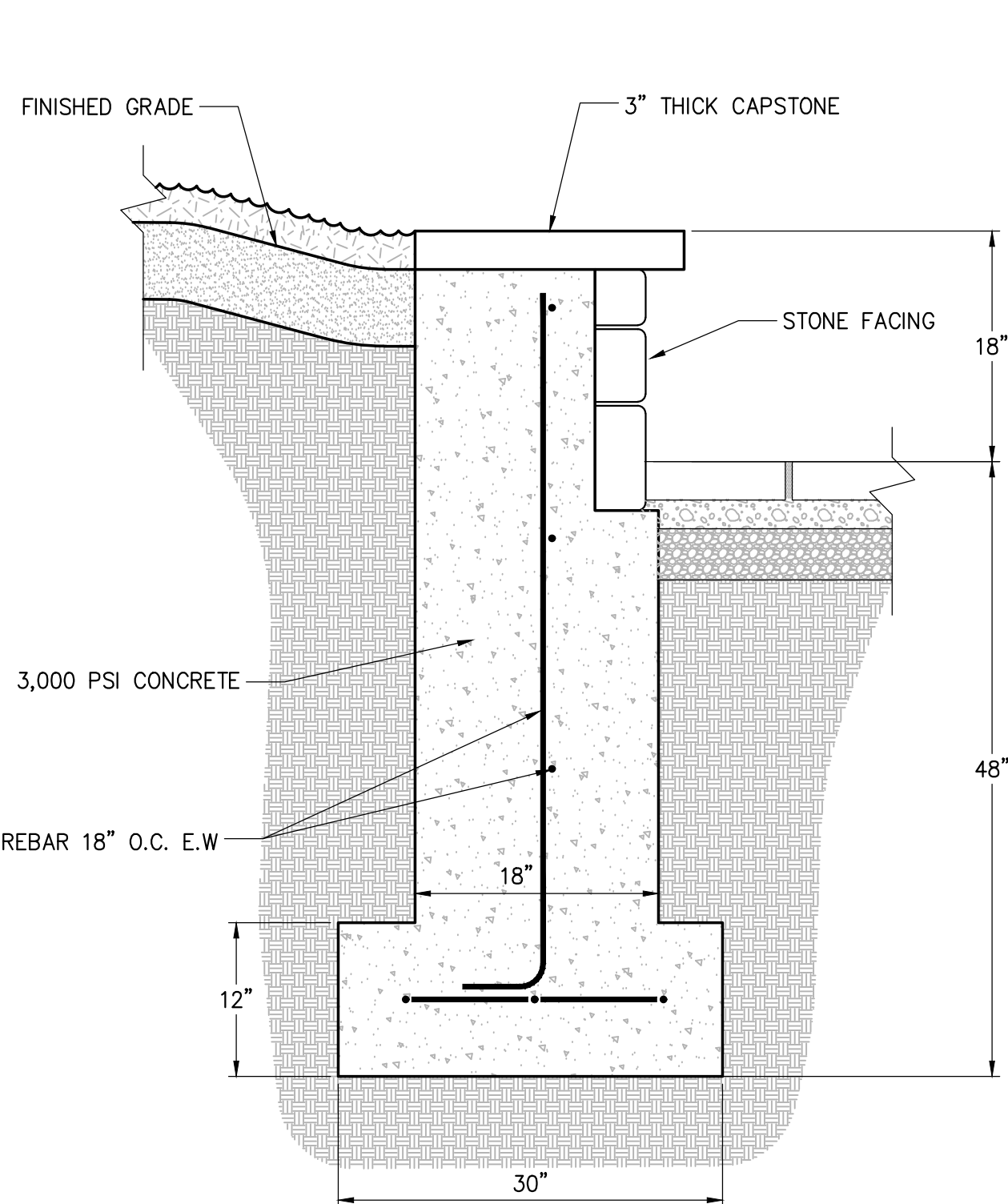
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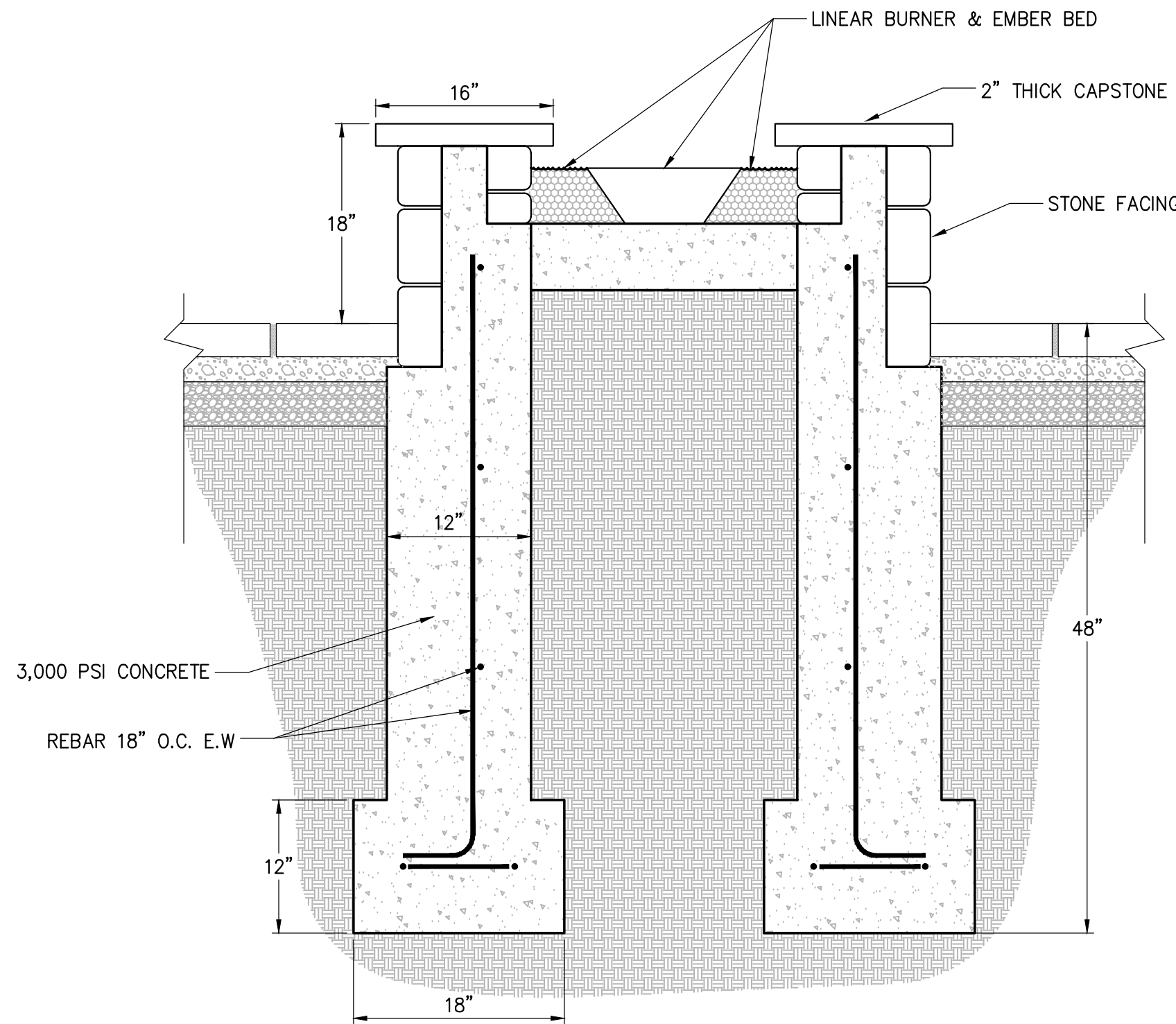
- NOTES:
1. INSTALL ROUNDED BOTTOM STONE TO GIVE A NATURAL STREAM APPEARANCE.
 2. JUTE NETTING WILL BE REQUIRED TO PREVENT EROSION OF GRASSED SIDE SLOPES.

10 GRASS SWALE WITH 3' WIDE STONE BOTTOM DETAIL

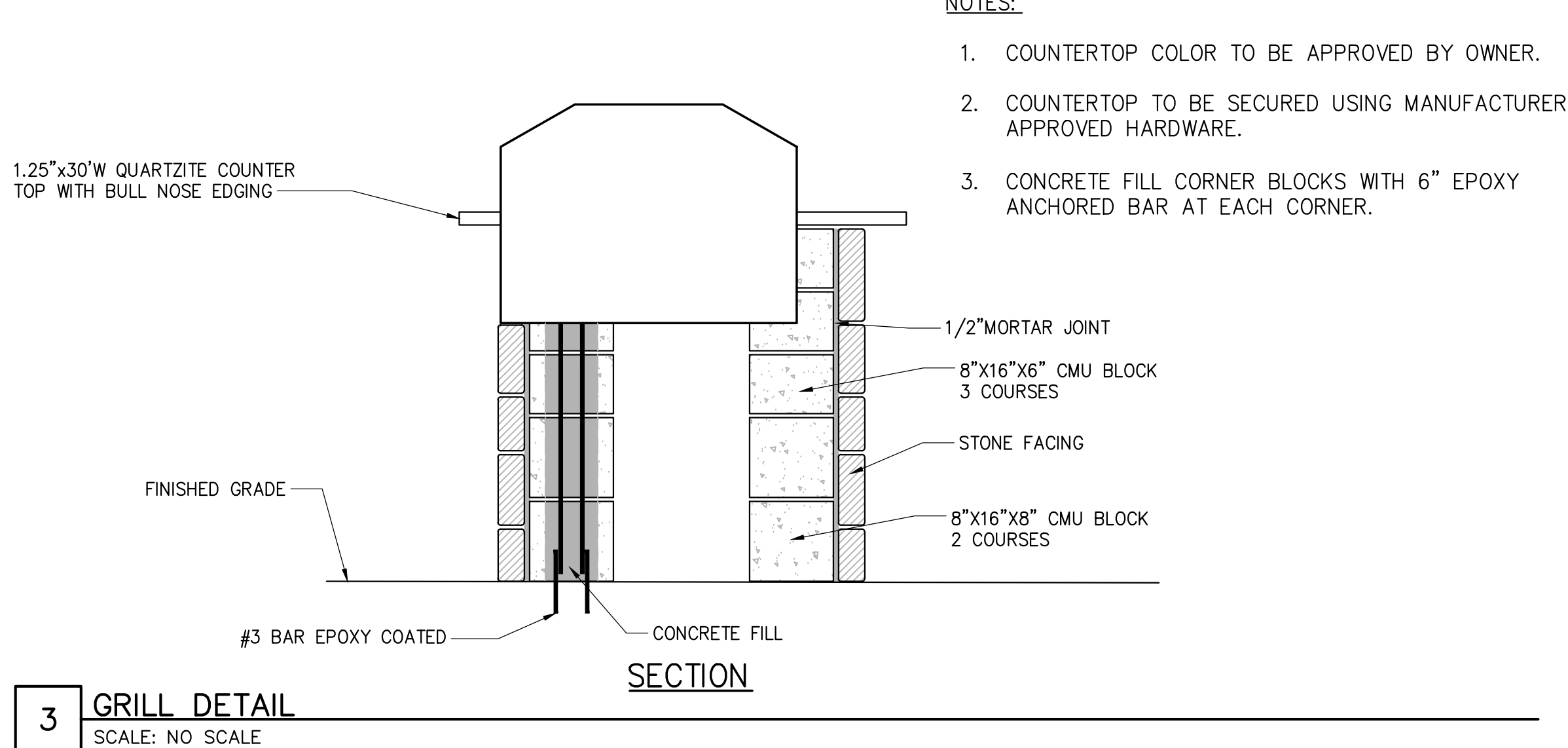
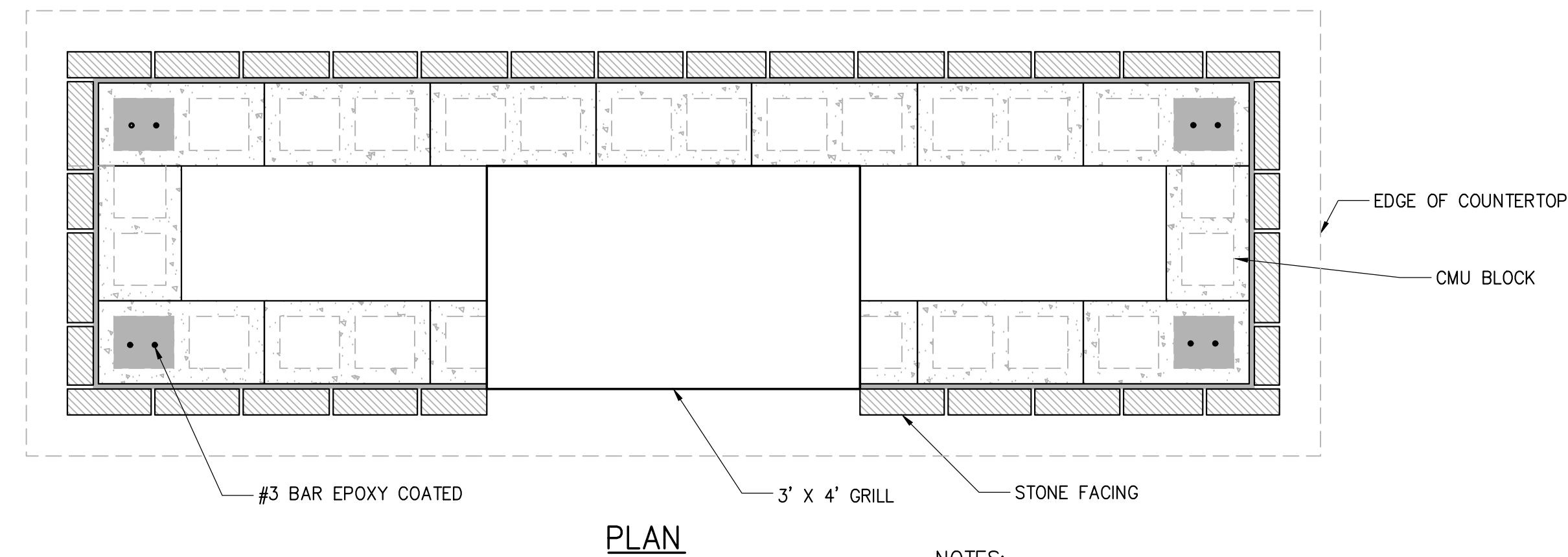
SCALE: NO SCALE



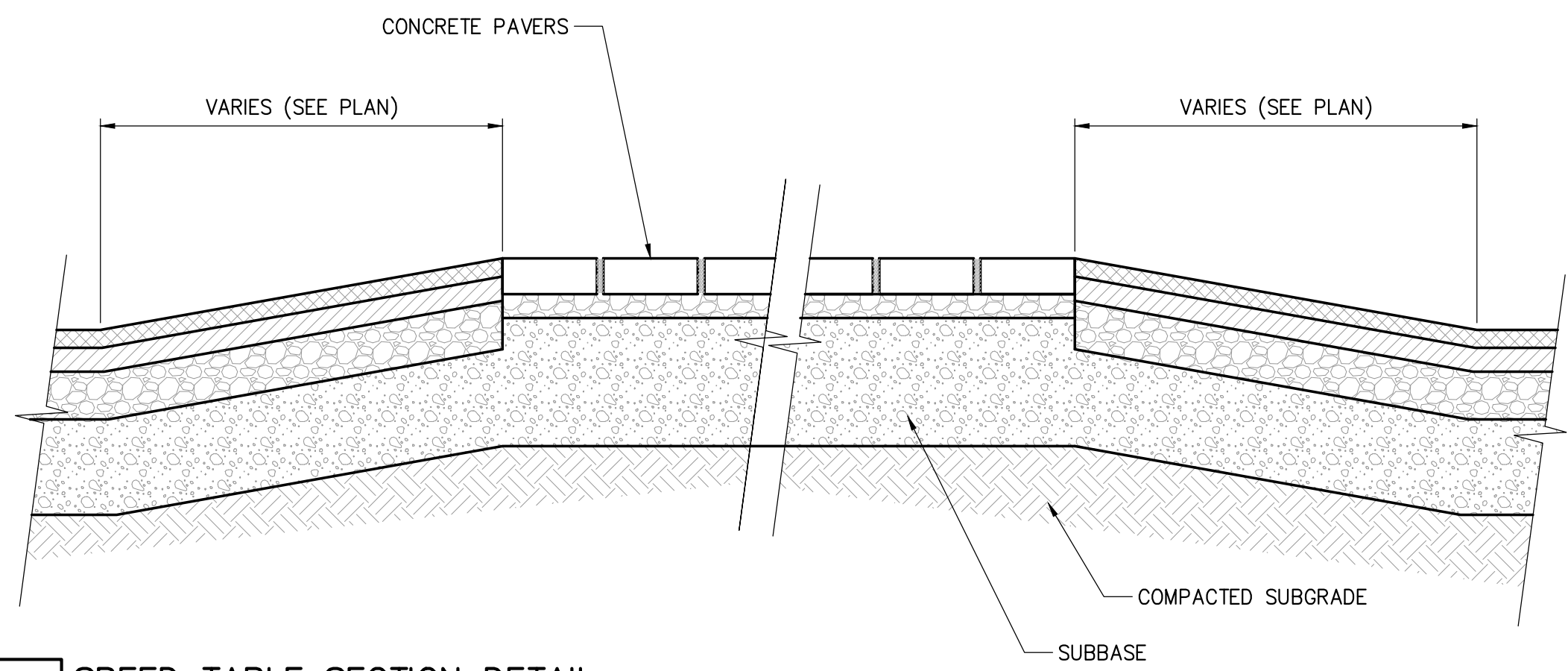
1 SEAT WALL SECTION DETAIL
SCALE: NO SCALE



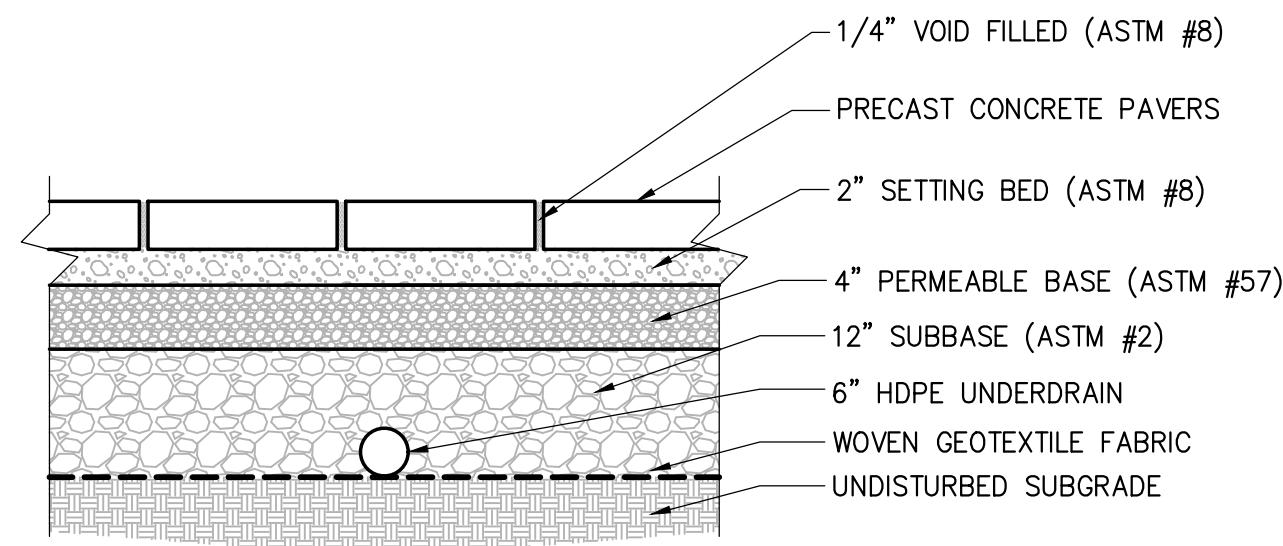
2 FIRE PIT SECTION DETAIL
SCALE: NO SCALE



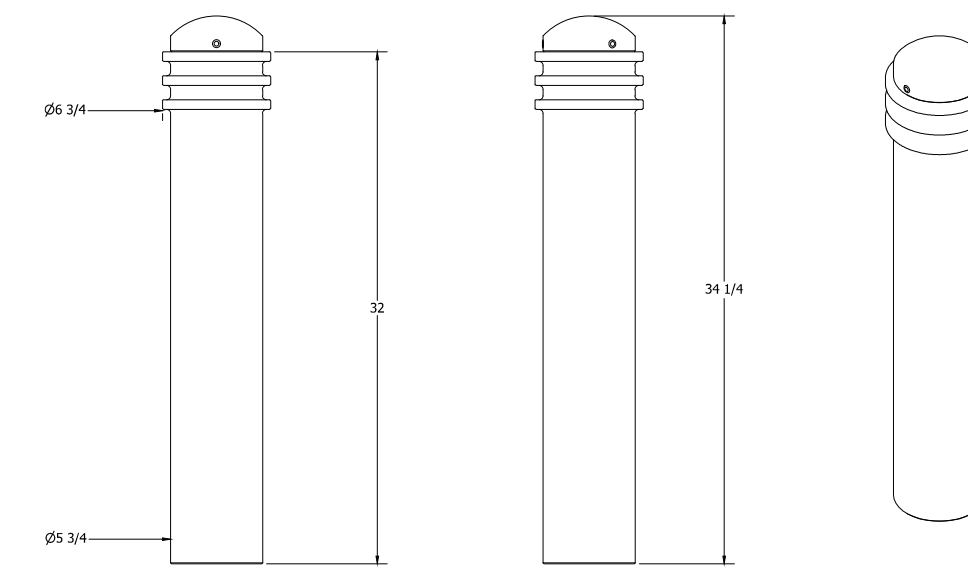
3 GRILL DETAIL
SCALE: NO SCALE



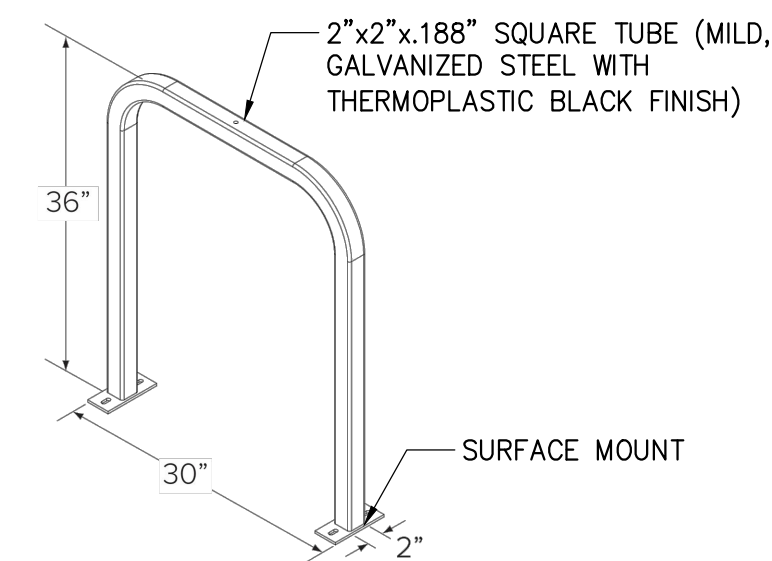
4 SPEED TABLE SECTION DETAIL
NOT TO SCALE



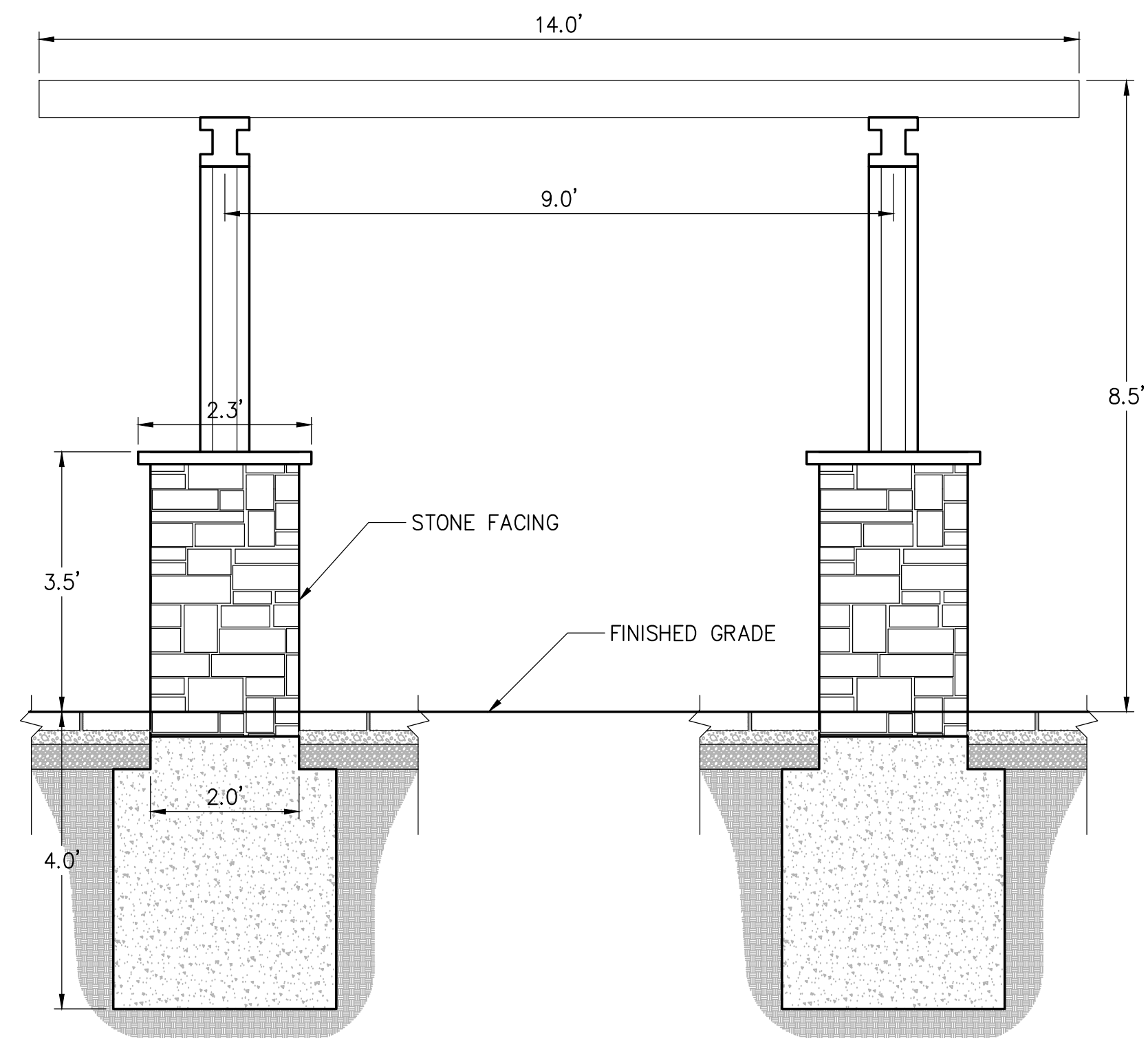
5 PERMEABLE PAVER SECTION DETAIL
SCALE: NO SCALE



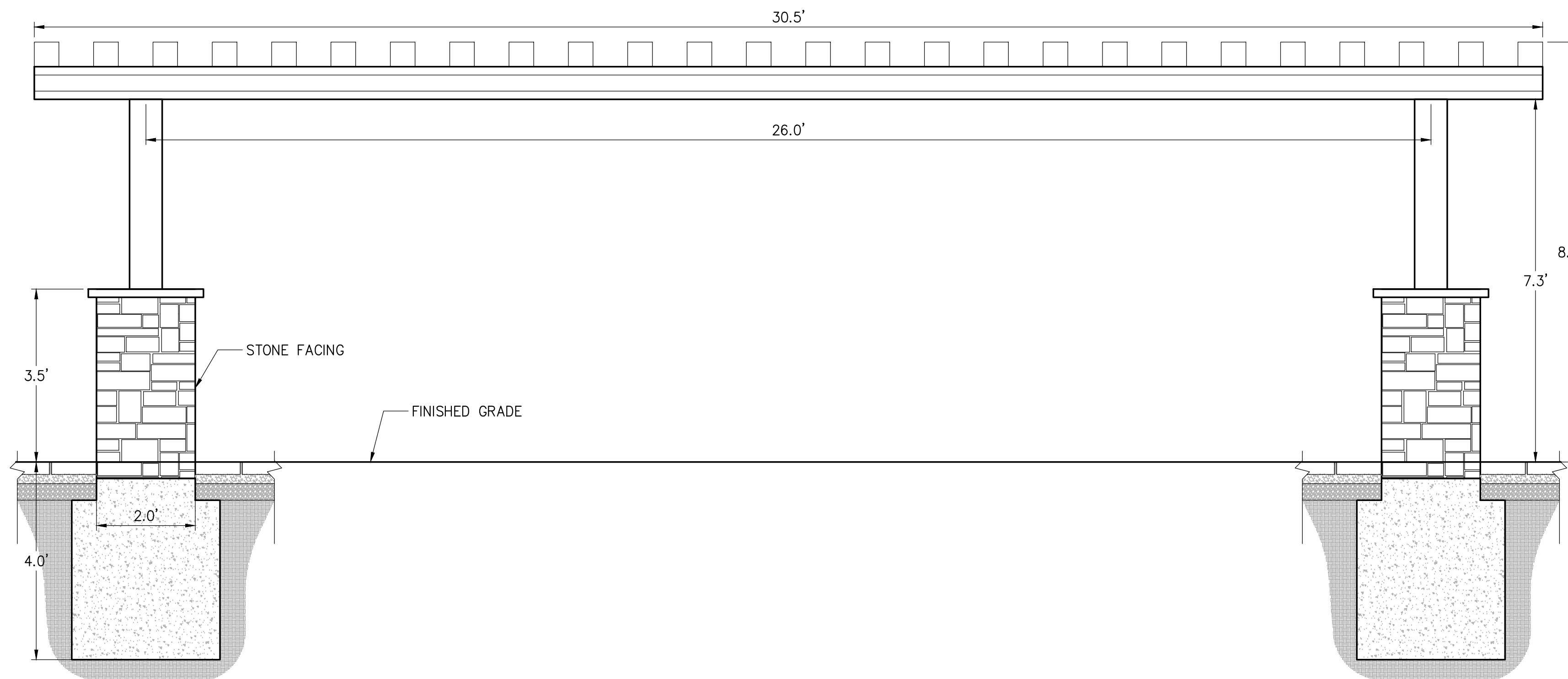
6 DECORATIVE BOLLARD DETAIL
NOT TO SCALE



7 U-SHAPED BIKE RACK
NOT TO SCALE



8 PERGOLA ELEVATION VIEW
SCALE: NO SCALE



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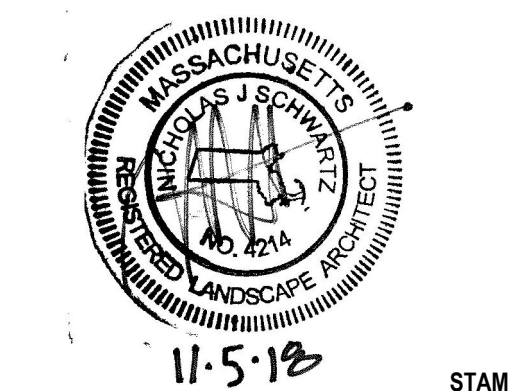
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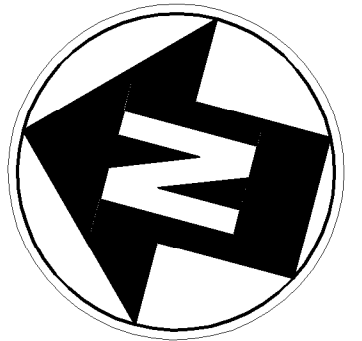
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REVISIONS ON SHEET

SCALE: UTILE PROJECT NUMBER 1839

DETAILS - 10

C-610



Scale in feet
0 20 40

MBTA COMMUTER RAIL (KINGSTON / PLYMOUTH LINE)

FEMA FLOOD LINE (ZONE A)

100' WETLAND BUFFER
50' WETLAND BUFFER

TROTTER ROAD
(ACCESS EASEMENT)

LEGEND

- 25'-FT BUFFER TO IWV
- 50'-FT BUFFER TO IWV
- 100'-FT BUFFER TO IWV
- IWV BOUNDARY AND FLAG
- IWV RESOURCES AREA WITHIN PROPERTY LIMITS
- FEMA FLOOD PLAIN ZONE A LIMITS PER FEMA FIRM PANEL NUMBER 25021002.99E DATED JULY 17, 2012
- BUFFER ZONE (0-25 FT)
- BUFFER ZONE (25-50 FT)
- BUFFER ZONE (50-100 FT)

PROPOSED DISTURBANCE				
	AREA DESCRIPTION	DISTURBANCE AREA (S.F.)	PROPOSED IMPERVIOUS AREA (S.F.)	PROPOSED PERVIOUS AREA (S.F.)
RESOURCE AREA	FEMA FLOOD PLAIN	74,090±	11,580±	62,510±
RESOURCE AREA	ISOLATED VEGETATED WETLAND (IWV)	NONE	-	-
BUFFER ZONES	ISOLATED BUFFER ZONE 0'-25'	NONE	-	-
	ISOLATED BUFFER ZONE 25'-50'	4,770±	1,600±	3,170±
	ISOLATED BUFFER ZONE 50'-100'	19,300±	9,970±	9,330±

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CIVIL & LANDSCAPE

BLW ENGINEERS

311 GREAT ROAD
LITTLETON, MA 01460
P 978 486.4301
MEPPF

VEITAS AND VEITAS

639 GRANITE ST
BRAINTREE, MA 02184
P 781 849.2065
STRUCTURAL

VANASSE & ASSOCIATES

35 NEW ENGLAND BUS CENTER DR.
SUITE 140
ANDOVER, MA 01810
P 978 474.8800
TRAFFIC

McPHAIL ASSOCIATES

2269 MASSACHUSETTS AVE.
CAMBRIDGE, MA 02140
P 617 868.1420
GEOTECH

KEY PLAN



STAMP

DATE REVISION
NOVEMBER 30, 2018 NOI SUBMISSION

REVISIONS ON SHEET

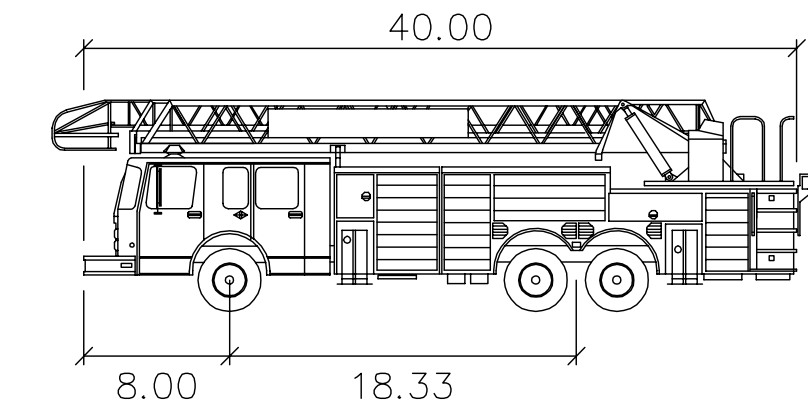
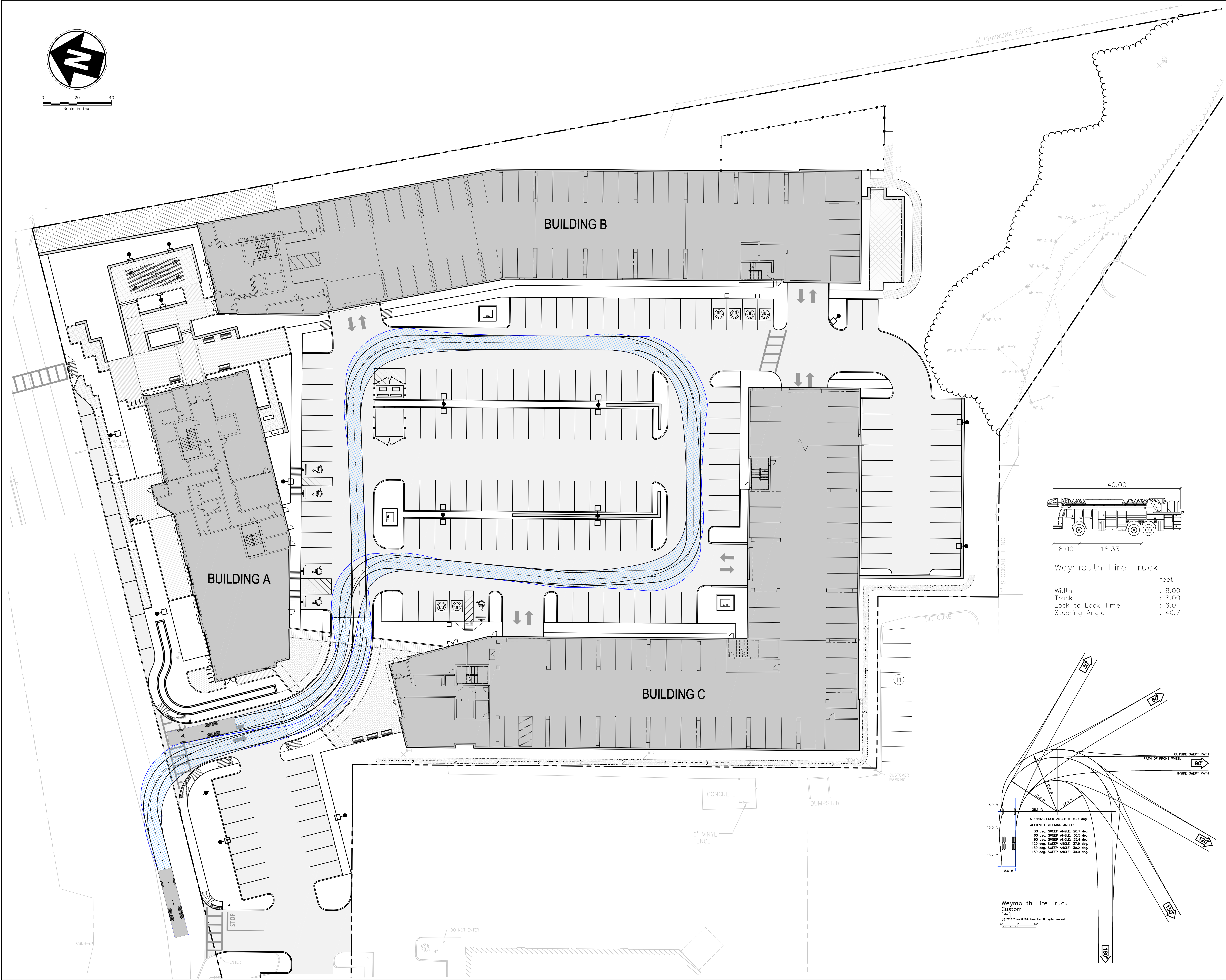
SCALE UTILE PROJECT NUMBER
1839

RESOURCE AREA
PLAN

C-801

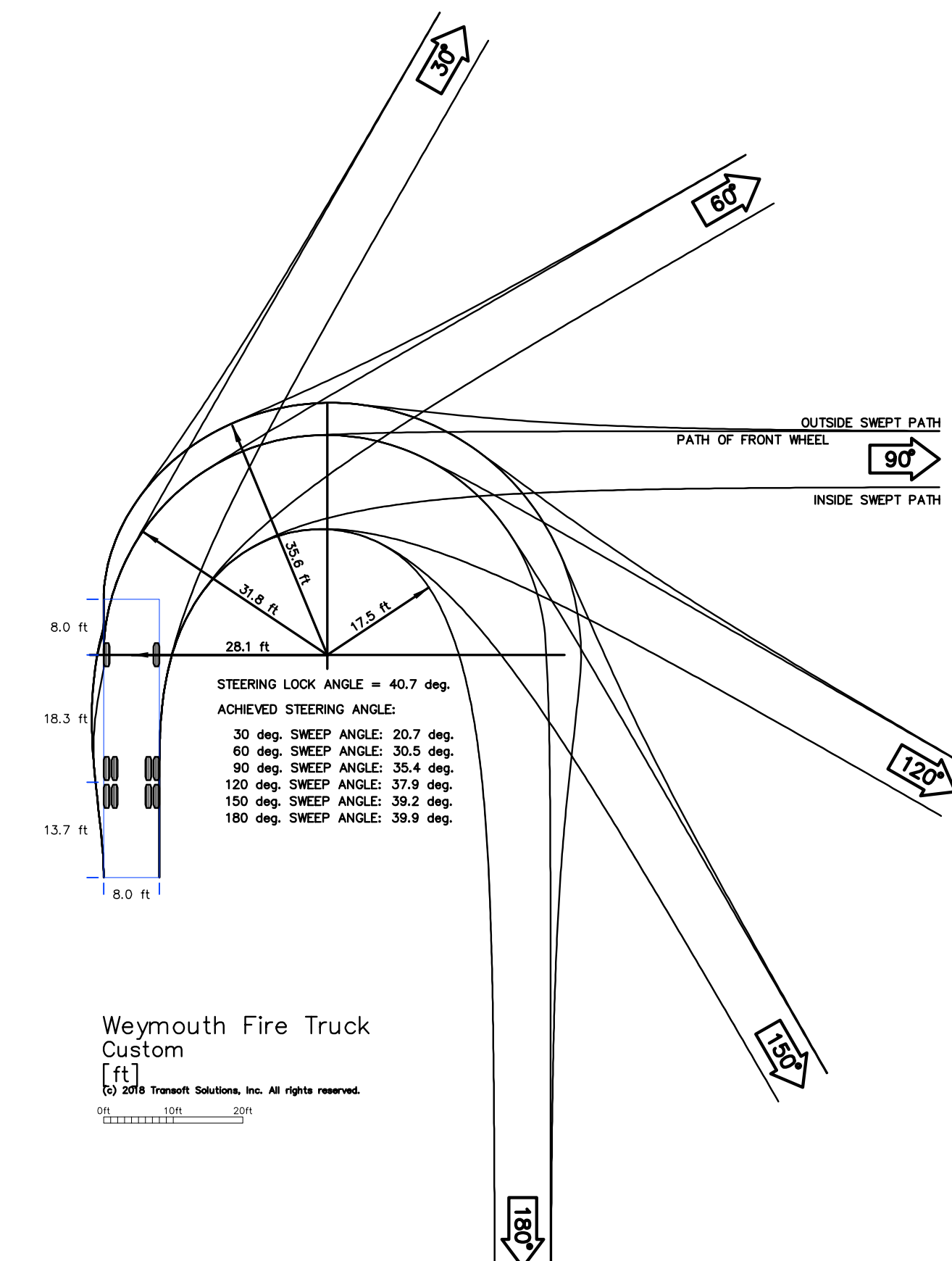


0 20 40
Scale in feet



Weymouth Fire Truck

	feet
Width	: 8.00
Track	: 8.00
Lock to Lock Time	: 6.0
Steering Angle	: 40.7



THE RESIDENCES AT 1500 MAIN

1500 MAIN STREET
WEYMOUTH, MA 02190

PROJECT

JOHN M. CORCORAN &
CO. LLC

100 GRANDVIEW ROAD, SUITE 203
BRAINTREE, MA 02184
P 781 849.0011

OWNER

utile

ARCHITECTURE + URBAN DESIGN

115 KINGSTON ST
BOSTON, MA 02111
P 617 423.7200 F 617 423.1414
utile.design.com

ARCHITECT

CHA COMPANIES

141 LONGWATER DRIVE, SUITE 104
NORWELL, MA 02061
P 781 982.5400

CIVIL & LANDSCAPE

BLW ENGINEERS

311 GREAT ROAD
LITTLETON, MA 01460
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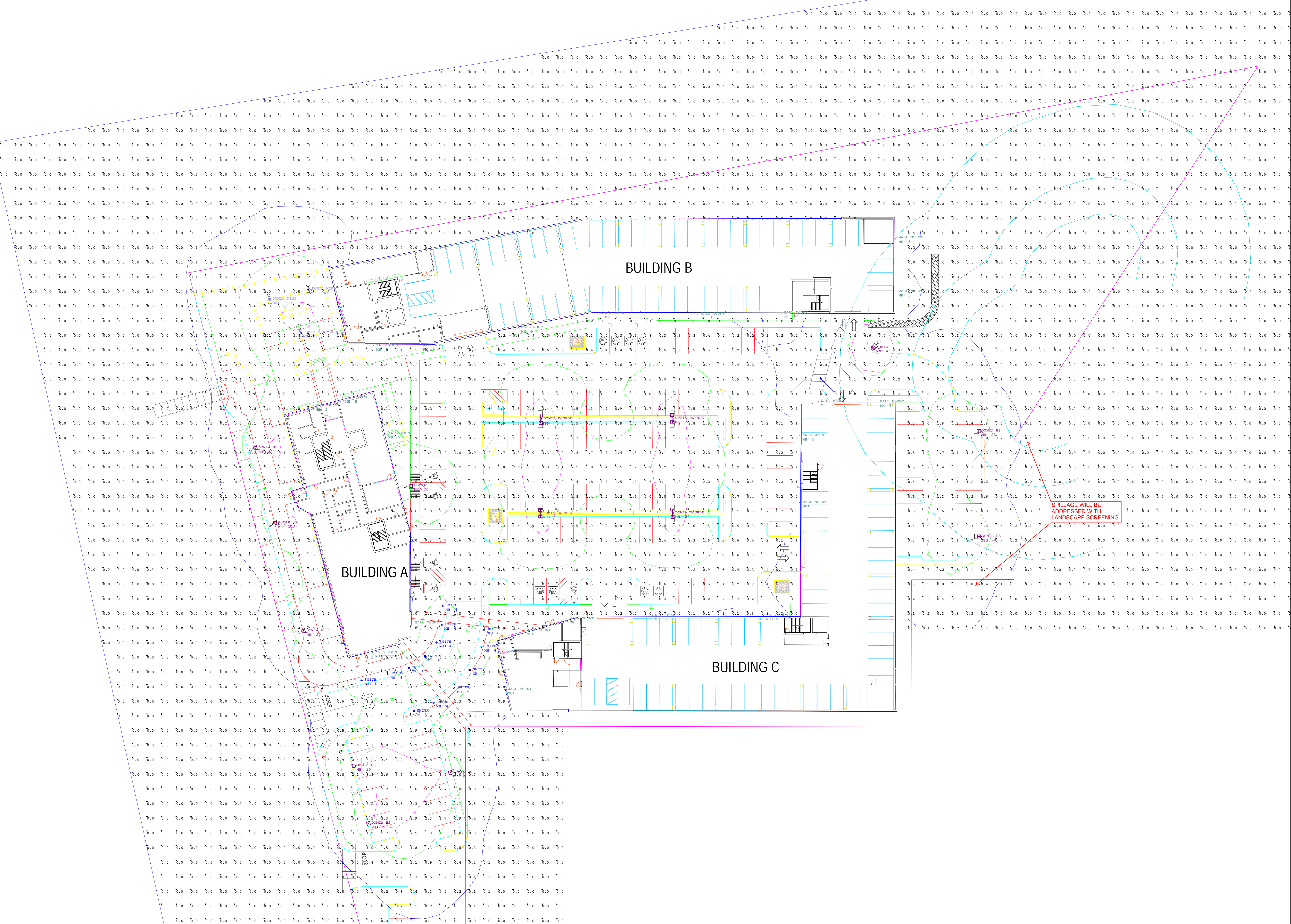
DATE REVISION
NOVEMBER 5, 2018 BZA SUBMISSION

REVISIONS ON SHEET

SCALE UTILE PROJECT NUMBER
1839


FIRE TRUCK
TURNING EXHIBIT

C-901



Luminaire Schedule					
Symbol	Qty	Label	Arrangement	Description	
	2	DOMIA	SINGLE	CYCLONE: CY55P1B-FGC-5-80W-3K	
	13	SMITH	SINGLE	LIGMAN: SM-10693-W30-XX-120/277	
	4	DOMIA DOUBLE	BACK-BACK	CYCLONE: CY55P1B-FGC-4HS-80W-3K	
	2	DOMIA MINI	SINGLE	CYCLONE: SY21P1-FGC-5-40W-3K	
	1	DOMIA MINI DOUBLE	BACK-BACK	CYCLONE: SY21P1-FGC-5-40W-3K	
	8	DOMIA HS	SINGLE	CYCLONE: CY55P1B-FGC-4HS-80W-3K	
	22	WALL MOUNT	SINGLE	WAC: WS-W53610	

Calculation Summary					
Label	CalcType	Units	Avg	Max	Min
GROUND_Planar	Illuminance	Fc	0.36	21.5	0.0
					Avg/Min
					Max/Min



REFLEXLIGHTING
7 Tide St., Boston, MA 02210

Southfield

Weymouth, MA

Drawn By: JIM

Checked By:

Date: 10.29.18

Specifier:

Scale: NTS

#

Date

Comments

XXXXXX

XX/XX

Revisions